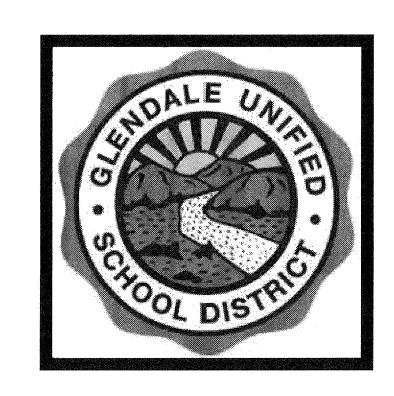
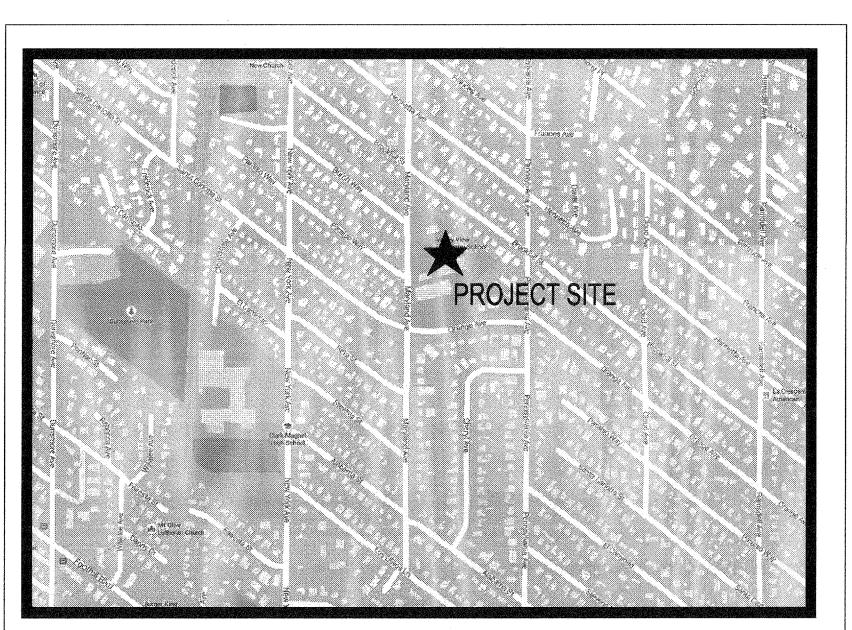
NEW SHADE STRUCTURE VALLEY VIEW ELEMENTARY SCHOOL



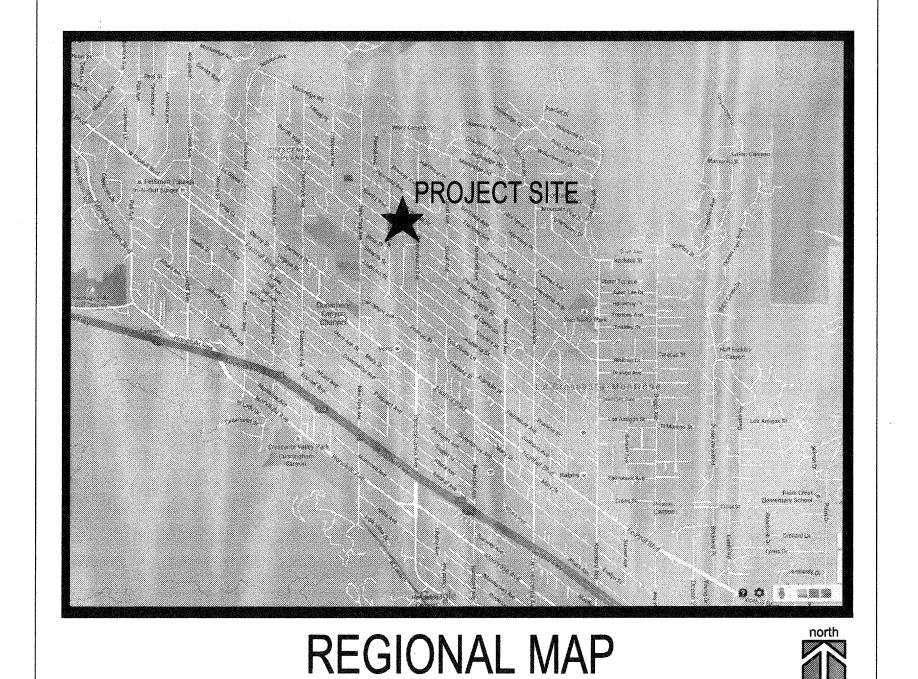
4900 MARYLAND AVE., LA CRESCENTA, CALIFORNIA

GLENDALE UNIFIED SCHOOL DISTRICT



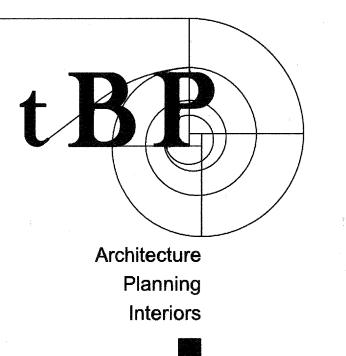


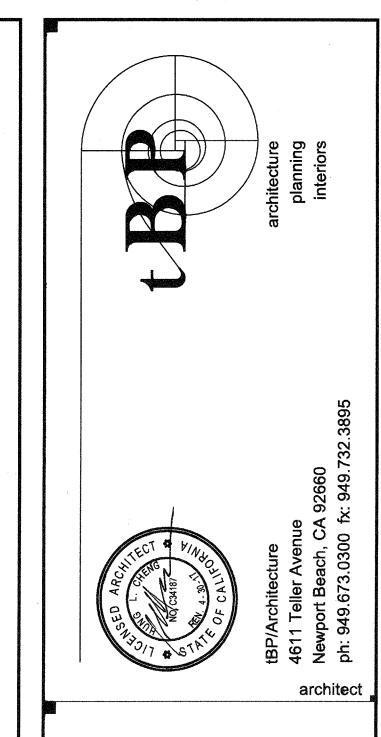
VICINITY MAP



tBP /Architecture

4611 Teller Avenue - Newport Beach - California - 92660 http://www.tbparchitecture.com ph: 949.673.0300 - fx: 949.732.3895





IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT

O 3 117045

ACW FLSTCSS 2016

DSA Los Angeles Regional Office
700 N. Alameda Street, Suite 5-500
Los Angeles, California 90012
ph: (213)897-3995 fx: (213)897-3159/0726

Y VIEW ELEMENTARY SCHOOL
SHADE STRUCTURE

4900 MARYLAND AVENUE
LA CRESCENTA, CALIFORNIA 91214

tBP project number: 20908.00
file name:
drawn by: checked by:
date: FEB. 09, 2016

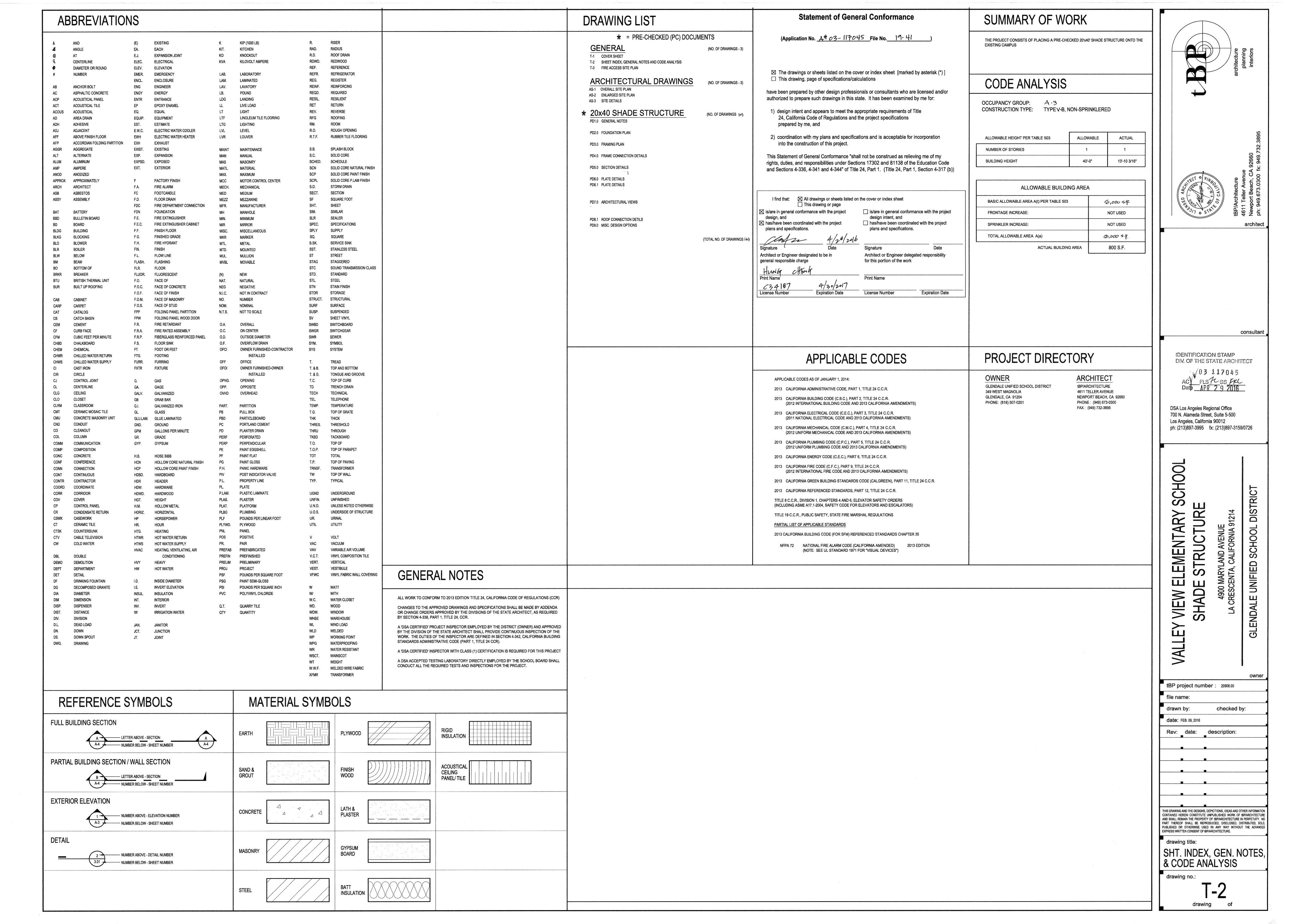
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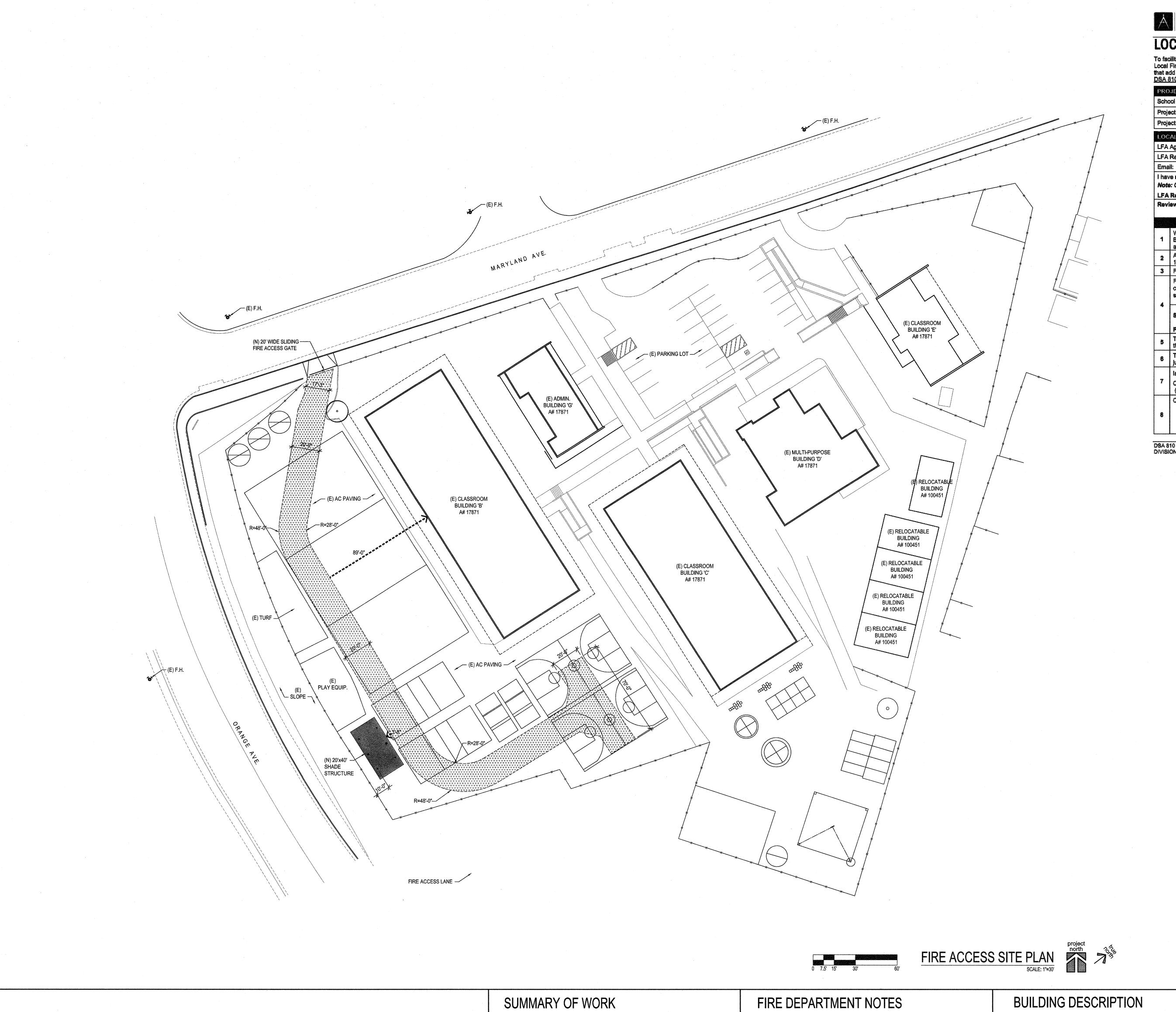
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COVER SHEET

drawing no

T-1





MDSA

LOCAL FIRE AUTHORITY REVIEW

To facilitate the Division of the State Architect's (DSA) approval of the Fire/Life Safety portion of a project, DSA requires Local Fire Authority (LFA) review of certain elements as identified in this form. Use of this form is mandatory for projects that add square footage to a campus or if any item on this form is relevant to the project. For additional information, see DSA 810 instructions and DSA Policy 09-01.

<u>DSA </u>	<u>810 Instructi</u>	ons and DSA P	olicy 09-01.						
PRO	DJECT INFO	RMATION							
Scho	ool District/C	wner: Glenda	le Unified School Distri	ct					
Proje	ect Name/S	chool: Valley	View Elementary School	ol Sha	ade Structure				
Proje	ect Address	4900 Maryla	nd Ave., La Crescenta,	CA 9	91214				
Loc	SAL FIRE A	JTHORITY (LF	A)						
LFA	Agency Na	me: County o	Los Angelese Films (Dep	anime	e nt	ring to province the second of the second or second or se			
LFA	. Reviewer N	ame:	APPRALE			Title:			
Ema	all:		APPKUVI		Telephone Numbe	it'.			
Note	e: Only sign		to the applicable items for it is imaged onto the site p		• •	cceptable to D	SA.	1-2	9-
Rev	lew Key:	•	es with FA requirements pplicable to the project		"N" = Not approve "NR" = LFA elec			8)	
			Description	1			Υ	N	NA
1		ode (CBC), use	ot meet medical emergen of stairways for emergen						
	1					•	1 1	- 1	

4	Where an elevator does not meet medical emergency service cab size, per the California Building Code (CBC), use of stairways for emergency rescue and patient transport is acceptable.			X			
2	Access roads, fire lane markings, pavers and gate entrances are in accordance with Title 19, California Code of Regulations and the California Fire Code, Chapter 5.	X					
3	Fire hydrant location and distribution complies with the California Fire Code (or see # 4).						
	Fire hydrant location and distribution complies with NFPA 1142, "Alternate Means." If "NR" is checked, DSA can only approve on-site water storage as an alternate. The signature of the school district official is required to acknowledge the use of alternate means.			X			
*	Signature of School District Official:						
5	The location(s) of the proposed post indicator valve and fire department connection meet the requirements of this jurisdiction.			X			
6	The location(s) of the detector check valve assembly meet the requirements of this jurisdiction.			X			

Is the project located in a hazard severity zone area? (CBC, Chapter 7A, Section 701A.) Yes No
Check type if "Yes": Moderate High Very High WIFA
(If one of these boxes is checked, the project design must meet the requirements of Chapter 7A.) COMMENTS (note deficiencies):

DSA 810 (rev 05-12-14) DIVISION OF THE STATE ARCHITECT

DEPARTMENT OF GENERAL SERVICES

Page 1 of 1 STATE OF CALIFORNIA

810

consultant

IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT 03 117045 ACAN FLS 72SS FCC Date APR 29 2016

DSA Los Angeles Regional Office 700 N. Alameda Street, Suite 5-500 Los Angeles, California 90012 ph: (213)897-3995 fx: (213)897-3159/0726

SHADE

VIEW ELEMENTARY SCHOOL

tBP project number: 20908.00

file name: checked by:

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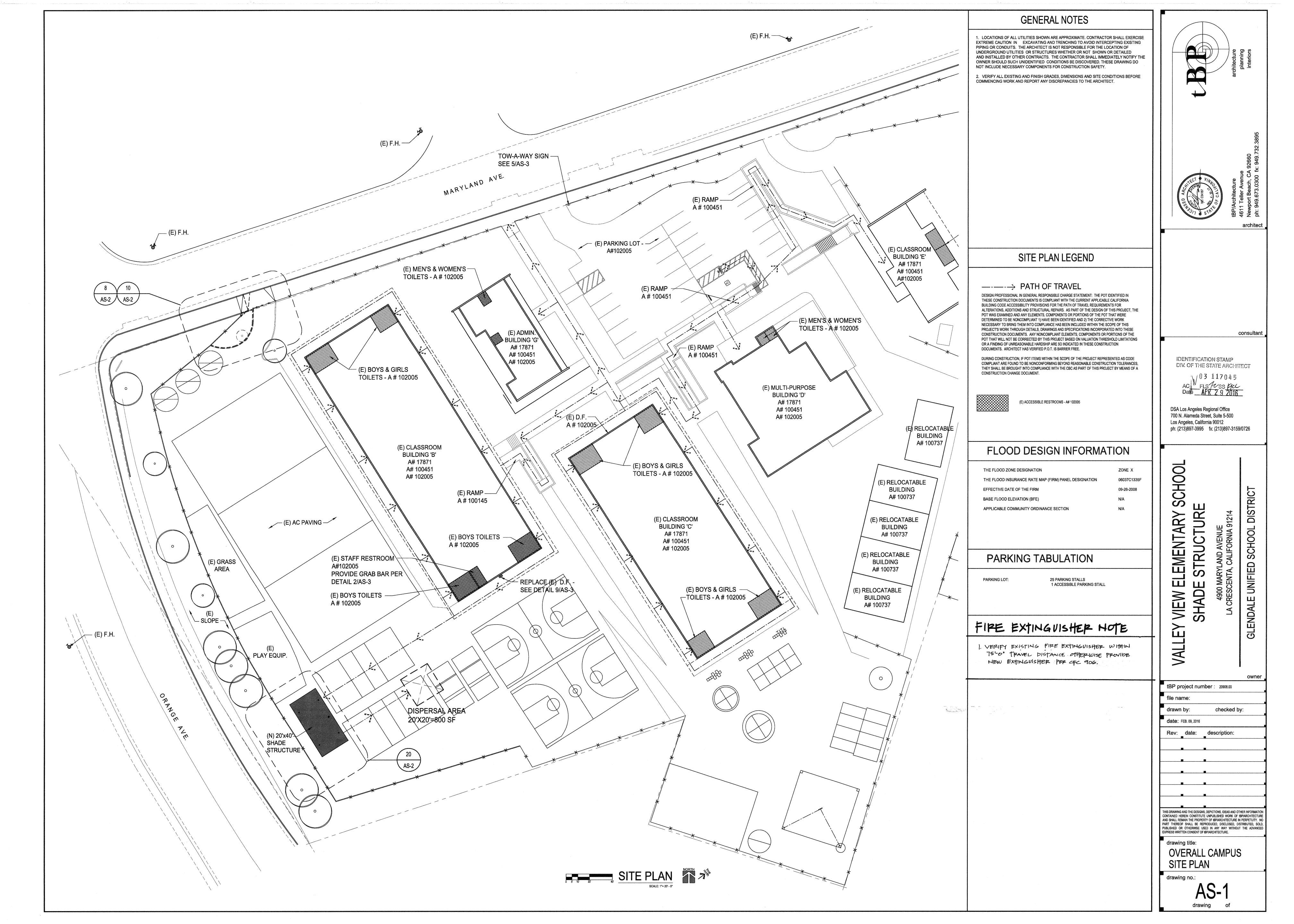
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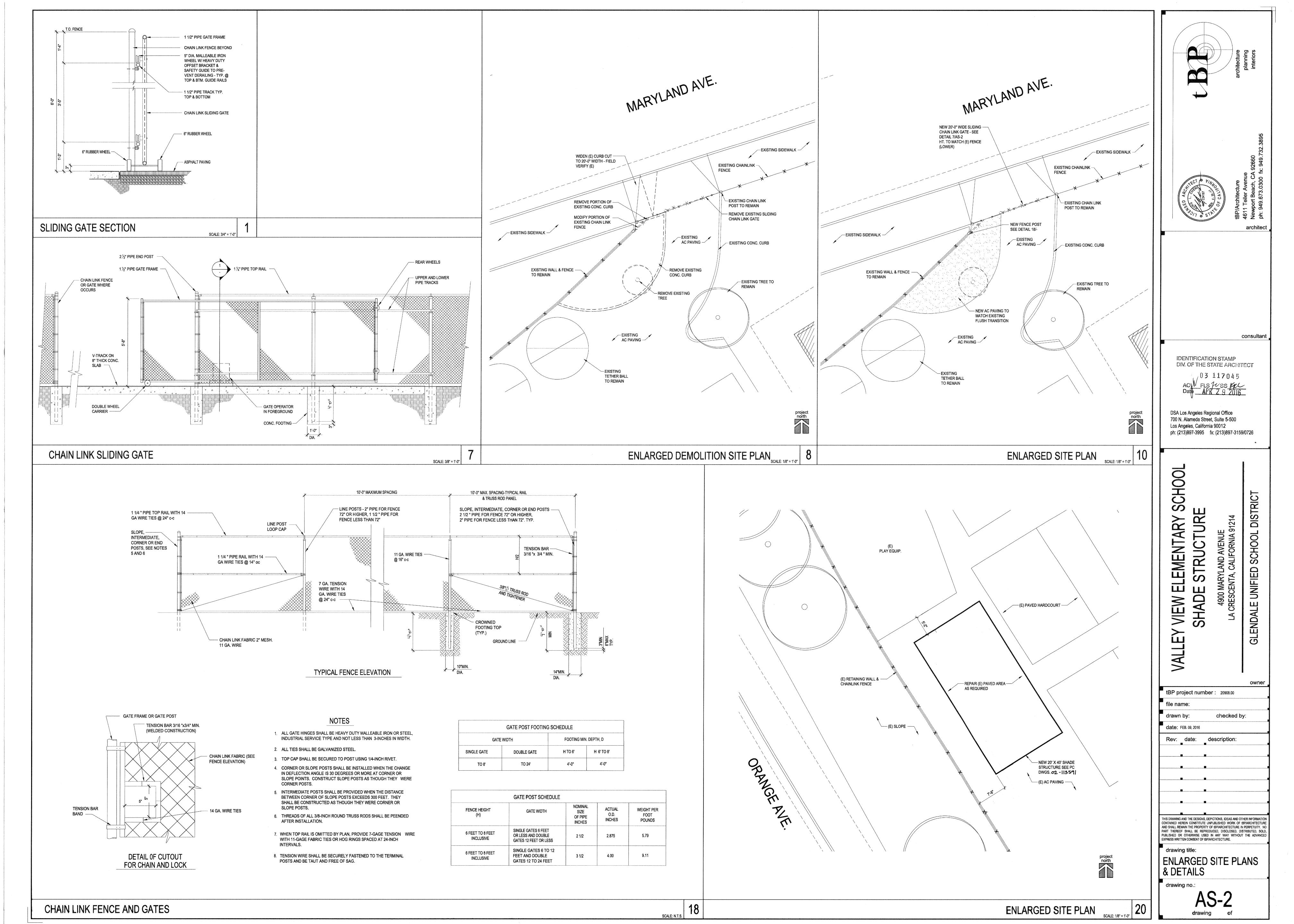
FIRE ACCESS SITE PLAN

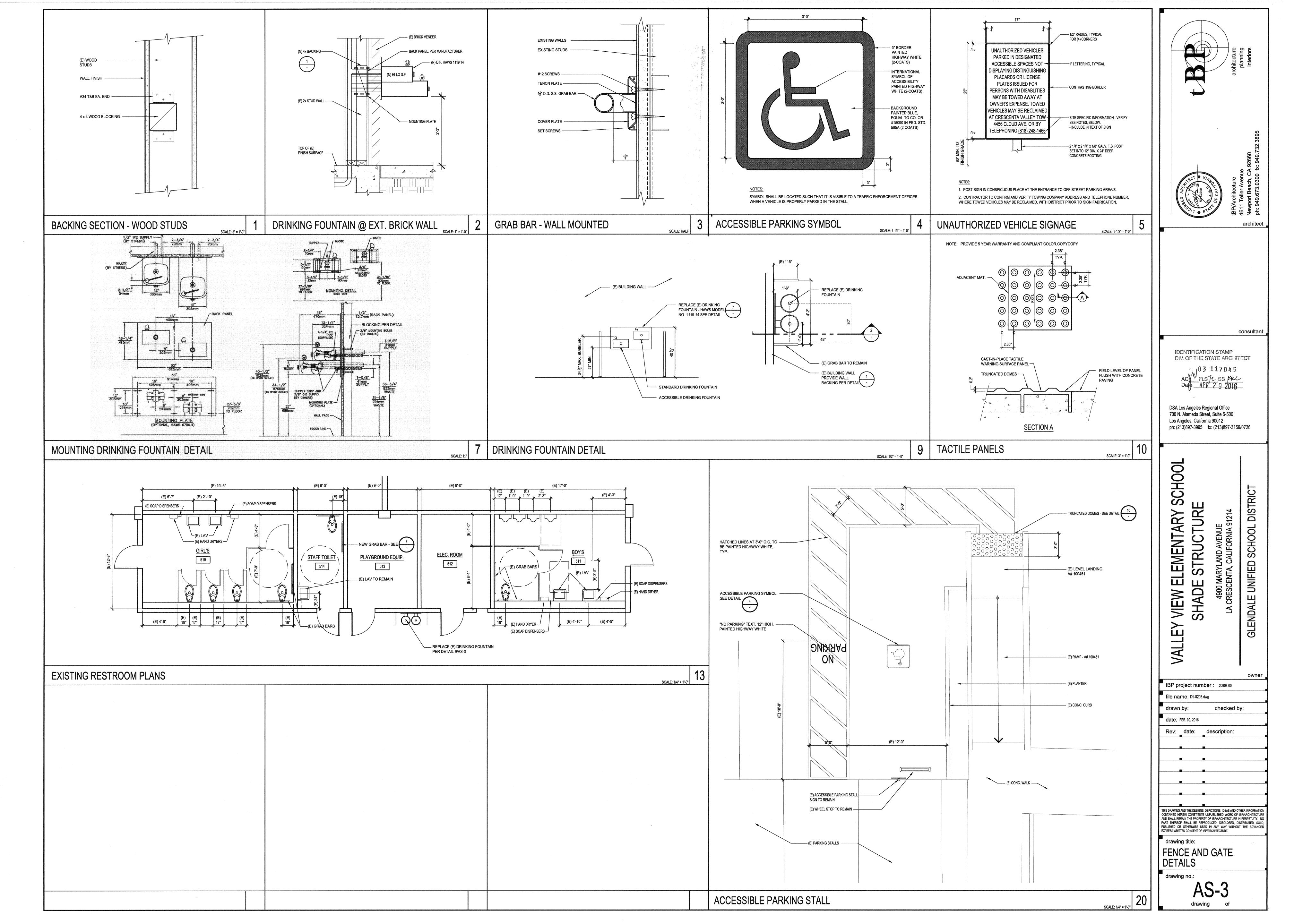
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SUMMARY OF WORK	FIRE DEPARTMENT NOTES	BUILDING DESCRIPTION SITE P	LAN LEGEND
THE PROJECT CONSISTS OF THE PLACEMENT OF A PRE-CHECKED 20'x40' SHADE STRUCTURE.	1. FIRE DEPARTMENT VEHICULAR ACCESS ROADS MUST BE INSTALLED AND MAINTAINED IN A SERVICEABLE MANNER PRIOR TO AND DURING THE TIME OF CONSTRUCTION. (FIRE CODE 501.4) 2. FIRE DEPARTMENT VEHICULAR ACCESS ROADS SHALL BE HARD SCAPE ALL WEATHER ACCESS IN ACCORDANCE WITH THE DEPARTMENT'S ALL WEATHER ACCESS	BUILDING AREA: 800 SF BUILDING HEIGHT: 16'-0", NO PARAPET SINGLE STORY BUILDING USE: A分OCCUPANCY CONSTRUCTION TYPE: TYPE Y-B, NON-SPRINKLERED	EXISTING FIRE DEPARTMENT VEHICULAR ACCESS LANE 4.4.4.4.4.4.4.4.4 4.4.4.4.4.4.4 4.4.4.4.4.4.4 20'-0" MINIMUM UNOBSTRUCTED WIDTH 4.4.4.4.4.4.4 FIREFIGHTER ACCESS WALKWAY
	REQUIREMENTS. (LOS ANGELES COUNTY FIRE CODE 503.2.3) 3. ALL FIRE HYDRANTS SHALL MEASURE 6" x 4" x 2 1/2", BRASS OR BRONZE, CONFORMING TO AMERICAN WATER WORKS ASSOCIATION STANDARD C503, OR APPROVED EQUAL, AND SHALL BE INSTALLED IN COMPLIANCE WITH THE COUNTY OF LOS ANGELES FIRE DEPARTMENT REGULATION 8. (FIRE CODE 507.5 AND REGULATION 8)	FIRE FLOW ANALYSIS	5'-0" MINIMUM CLEAR WIDTH TO A P. (E) FHO. EXISTING FIRE HYDRANT E
		2013 CALIFORNIA FIRE CODE TABLE BB105.1 - MIN. REQUIRED FIRE FLOW AND FLOW DURATION FOR BUILDINGS FIRE AREA FIRE FLOW FLOW DURATION 50% REDUCTION DUE TO REQUIRED TYPE V-B (GPM) (HOURS) FIRE SPRINKLERS (BB105.1) FIRE FLOW	
		800 S.F. 1,500 2 0 1,500	







AKCHIIECIUKAL KEQUIKEMENIS.				
DESCRIPTION	DESIGN VALUES			
TYPE OF CONSTRUCTION	∨B			
OCCUPANCY CLASSIFICATION	A3			
NUMBER OF STORIES	1			
FIRE HAZARD SEVERITY ZONE	VERY HIGH			
FIRE SPRINKLER SYSTEM	NOT BY POLIGON			

POLIGON ASSUMES ANY OCCUPANT LOAD CALCULATIONS ARE BASED ON 15 SQ FT/ PERSON. PROJECT ARCHITECT MAY ADJUST OCCUPANT LOAD AS PERMITTLED BY THE BUILDING CDE.

RELATED BUILDING CODES AND STANDARDS:

TITLE 24 CODES:

2013 California Building Code (CBC), Volumes 1, and 2 (Part 2, Title 24, CCR) (2012 International Building Code with 2013 California amendments)(Part 3, Title 24, CCR) 2013 California Electrical Code 2013 California Mechanical Code (CMC) .. (2012 Uniform Mechanical Code with 2013 California amendments) ...(Part 5, Title 24, CCR) 2013 California Plumbing Code (CPC). ..(Part 6, Title 24, CCR) (Effective July 1, 2014) 2013 California Fire Code (CFC)(Part 9, Title 24, CCR) (2012 International Fire Code with 2010 California Amendments) (Effective January 1, 2014) 2013 California Referenced Standards Code (Part 12, Title 24, CCR)

NFPA 13 - 2013 NFPA 72 - 2013

REFERENCE CODE SECTIONS FOR APPLICABLE STANDARDS:

2013 California Administrative Code (CAC)..........

2013 CBC, CHAPTER 35 2013 CFC, CHAPTER 45

SCOPE OF WORK NARRATIVE:

THESE DRAWINGS ILLUSTRATE THE FABRICATION AND INSTALLATION REQUIREMENTS FOR A FREE-STANDING PREFABRICATED TEEL SHADE STRUCTURE. THE ENTIRE STRUCTURAL SYSTEM IS COMPRISED OF TUBULAR STEEL MEMBERS SUPPORTED ON CONCRETE FOUNDATIONS. THE FLEXIBILITY INCLUDED HEREIN ALLOWS THIS STRUCTURE TO COMPLY WITH A WIDE VARIETY OF PROJECT SITES AND LOADING REQUIREMENTS.

- GENERAL NOTES AND TYPICAL DETAILS SHALL APPLY TO ALL PARTS OF THE JOB EXCEPT WHERE THEY MAY CONFLICT WITH DETAILS AND NOTES ON OTHER SHEETS. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER FOR THIS PROJECT.
- WORK SHALL CONFORM TO THE REQUIREMENTS, AS AMENDED TO DATE, OF THE LATEST ADOPTED EDITION OF THE CBC, C.A.C. TITLE 24, AND ALL OTHER LOCAL, STATE AND FEDERAL REGULATIONS.
- OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT PRIOR TO
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHALL CHECK ALL DIMENSIONS. ALL DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT AND BE RESOLVED BEFORE PROCEEDING WITH THE WORK.
- THESE CONSTRUCTION DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO, BRACING, TEMPORARY SUPPORTS, AND SHORING. OBSERVATION VISITS TO THE SITE BY FIELD REPRESENTATIVES OF THE ARCHITECT/ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. ANY SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER DURING THE CONSTRUCTION SHALL BE DISTINGUISHED FROM CONSTRUCTION AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER, WHETHER OF MATERIAL OR WORK, ARE FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DOCUMENTS, BUT DO NOT GUARANTEE
- ASTM DESIGNATIONS AND ALL STANDARDS REFER TO THE LATEST AMENDMENTS.
- CONFORM TO APPLICABLE CAL/OSHA CONSTRUCTION SAFETY REGULATIONS FOR ALL WORK PERFORMED DURING CONSTRUCTION. JOB SITE SAFETY IS STRICTLY THE RESPONSIBILITY OF THE CONTRACTOR AND NOT THE ARCHITECT/ENGINEER OR OWNER.
- THE ENGINEER AND THEIR CONSULTANTS SHALL HAVE NO RESPONSIBILITY FOR THE DISCOVERY, HANDLING, REMOVAL OR DISPOSAL OF HAZARDOUS MATERIALS AT THE PROJECT SITE, INCLUDING BUT NOT LIMITED, TO ASBESTOS, ASBESTOS PRODUCTS, POLYCHLORINATED BIPHENYL (PCB) OR OTHER TOXIC SUBSTANCES.
- SHOULD ANY CONDITIONS DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS, OR IF A CHANGE IN THE SCOPE OF WORK IS PROPOSED, A CONSTRUCTION CHANGE DOCUMENT DETAILING AND SPECIFYING THE
- REQUIRED CHANGE(S) SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK. THE SCHOOL DISTRICT'S INSPECTOR OF RECORD SHALL INSPECT AND APPROVE THE ERECTED FRAME PRIOR TO
- ROOF INSTALLATION. SEE REQUIREMENTS FOR LOCATION IN ANY FIRE HAZARD SEVERITY ZONE FOR WILDLAND URBAN INTERFACE AREAS (WUI) AS SPECIFIED IN THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. PROVIDE PROTECTION AND DETAILS OF ALL AREAS COMPLYING WITH THE WUI REQUIRMENTS.
- LOCATING THIS STRUCTURE CLOSER THAN 20 FEET TO OTHER STRUCTURES MAY AFFECT THE ALLOWABLE AREA FOR
- THE EXISTING CONSTRUCTION PER THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE.
- VIEWS AND DETAILS ARE NOT DRAWN TO SCALE (UNLESS NOTED OTHERWISE). DO NOT SCALE THESE DRAWINGS. OTHER SITE SPECIFIC ITEMS MAY BE REQUIRED.

STRUCTURAL AND MISCELLANEOUS STEEL:

- ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) SPECIFIATION MANUAL REFERENCED BY THE LATEST EDITION OF THE CALIFORNIA BUILDING CODE.
- PIPE SECTIONS SHALL CONFORM TO ASTM A53, Fy = 35 ksi, GRADE B OR A501 UNLESS NOTED OTHERWISE.
- STRUCTURAL TUBING (HSS SHAPES) SHALL CONFORM TO ASTM A500, GRADE B (OR HIGHER), Fy = 46 KSI. IF MATERIAL AVAILABILITY IS LIMITED, MEMBER THICKNESSES CAN BE INCREASED BEYOND WHAT IS SHOWN IN THESE DRAWINGS (MAXIMUM INCREASE OF 1/8").
- ALL CHANNELS, ANGLES, AND MISC. STEEL SHALL CONFORM TO ASTM A36, Fy = 36 KSI.

THE POLIGON ENGINEERING DEPARTMENT IS AVAILABLE TO HELP YOU COMPLETE THESE STEPS (616-399-1963).

- FRAME WIDTHS AND LENGTHS ASSUME 2' OVERHANGS (UNO BY ARCHITECT - 2' MAX DIMENSION)

- SS VALUE DEPENDS ON THE PROJECTS GEOGRAPHICAL LOCATION (VALUES RANGE FROM 0.00 TO 3.73)

- THE SS REGION DICTATES THE MAXIMUM DEAD LOAD PERMITTED ON THE FRAME (SEE TABLE TO THE RIGHT)

- E.G. A PROJECT IN THE WHITE SS REGION WITH A 4 PSF ROOF DEAD LOAD IS LOAD SCENARIO 2

- LOAD SCENARIOS HAVE NO IMPACT ON FRAME DESIGN OR COST (BUT DO AFFECT FOUNDATION SIZE)

- THE COLLATERAL LOAD REPRESENTS ADDITIONAL LOAD THAT CAN BE SUPPORTED BYTHE FRAME

- THE 20' AND 30' WIDTHS ARE SUGGESTED BECAUSE THEY ARE THE MOST ECONOMICAL

- "SS" REPRESENTS MCELROY METAL "MEDALLION-LOK" 16" STANDING SEAM ROOF DECK

MAXIMUM WIDTH IS 30'; (SEE 'ARCHITECTURAL VIEWS' SHEET FOR REFERENCE)

- ALL COLD FORM STEEL SHALL CONFORM TO ASTM A653, CS = TYPE B, Fy = 50 KSI.
- STRUCTURAL STEEL AND DECK SHALL BE IDENTIFIED FOR CONFORMITY PER CBC 2203A.1.
- ROOF DECK SHALL HAVE KYNAR 5000 METAL COATING. ROOF DECK SHALL CONFORM TO ATSM A792, Fy = 50 KSI.

TEP 1: SELECT FRAME DIMENSIONS FOR YOUR PROJECT

STEP 2: SELECT ROOF DECK FOR YOUR PROJECT

- STRUCTURES UP TO 20' WIDE USE THE "RAM 20" BASE FRAME

- STRUCTURES UP TO 30' WIDE USE THE "RAM 30" BASE FRAME

- "MR" REPRESENTS MCELROY METAL "MULTI-RIB" ROOF DECK

- SS VALUE DETERMINES THE REQUIRED SEISMIC DESIGN FORCES

- THE REGIONS ARE DEPENDANT ON THE SS VALUE DETERMINED IN STEP 3

- REFERENCE DSA BU 14-01 FOR A MAP OF VARIOUS SS REGIONS

STEP 3: IDENTIFY THE SS ACCELERATION (g) FOR YOUR PROJEC

STEP 4: IDENTIFY THE SS REGION FOR YOUR PROJECT

IDENTIFY A SINGLE LOAD SCENARIO

STEP 5: IDENTIFY THE ROOF DEAD LOAD FOR YOUR PROJECT

- THE ROOF DECK DEAD LOAD WILL ALWAYS BE INCLUDED

STEP 6: IDENTIFY THE FOUNDATION REQUIREMENTS FOR YOUR PROJECT

- SELECT EITHER SPREAD PAD OR DRILLED PIER FOUNDATION

- REFERENCE THE SS REGION (STEP 4) AND THE TOTAL ROOF DEAD (STEP 5)

MR ROOF SCREWS MEET ASTM A510 WITH A HEAD DIMENSION OF 0.31" (FLAT-TO-FLAT) AND INTEGRAL WASHER DIMENSION OF 0.58" (OUTSIDE DIAMETER).

INSTRUCTIONS FOR ARCHITECTS SUBMITTING THESE PRE-CHECKED DRAWINGS TO DSA:

BEFORE SUBMITTING THESE PRE-CHECKED DRAWINGS FOR YOUR PROJECT, FOLLOW THE STEPS BELOW TO PROPERLY DEFINE THE APPROVED OPTIONS:

- THE 44', 64', AND 84', LENGTHS ARE SUGGESTED BECAUSE THEY ARE THE MOST COMMON (20' BAYS ARE MOST ECONOMICAL)

- FIND SS VALUES FOR YOUR PROJECT ON THE USGS WEBSITE (SEARCH INTERNET FOR "USGS SEISMIC DESIGN MAPS")

- BE SURE THE TOTAL ROOF DEAD LOAD FOR YOUR PROJECT IS LESS THAN OR EQUAL TO THE MAX DEAD LOAD SHOWN IN STEP 4

- FOUNDATION TYPE IMPACTS STEEL FABRICATION (COLUMN LENGTH) AND CONSTRUCTION (TIMING, SEQUENCE, COST, ETC.)

- POLIGON CAN REVIEW THE SITE-SPECIFIC SOILS REPORT TO EVALUATE THE POSSIBILITY OF SMALLER FOUNDATIONS

- THIS PC IS NOT APPROVED FOR SS VALUES GREATER THAN 3.00 (CONTACT POLIGON FOR ADDITIONAL OPTIONS)

SS ROOF SCREWS MEET ASTM A510 WITH A HEAD DIMENSION OF 0.437" (OUTSIDE DIAMETER).

WELDING:

- ALL WELDING SHALL COMPLY WITH AWS D1.1 SPECIFICATIONS AND SHALL BE DONE BY AWS QUALIFIED WELDERS CERTIFIED FOR THE TYPE OF WELDING TO BE PERFORMED AS REQUIRED BY DSA.
- ALL WELDING SHALL BE DONE BY GAS METAL ARC PROCESS WITH E70XX ELECTRODES. FLUX CORE ARC WELD SHALL CONFORM TO CHARPY NOTCH TOUGHNESS RATING OF 20 ff-lb @ (O° F).
- ALL WELDING SHALL BE DONE IN THE SHOP WITH REQUIRED INSPECTION, PRE-APPROVED BY DSA, TO INSURE PROPER MATERIAL ID AND WELDING.
- WELD FILLER METAL MANUFACTURER SHALL PROVIDE WRITTEN CERTIFICATION OF COMPLIANCE WITH CODE AND SPECIFICATIONS.

- ALL BOLTS SHOWN ON THESE DRAWINGS ARE ASTM A325 HIGH STRENGTH BOLTS (UNO), TYPE 3.
- HIGH STRENGTH BOLTS SHALL BE SAMPLED AND TESTED IN COMPLANCE WITH CBC 2213A.1. BEFORE ERECTING THE FRAME, VERIFY ALL BOLTS AND NUTS ARE CLEAN OF DEBRIS AND BURRS - INCLUDING THE HARDWARE ALREADY FASTENED INSIDE THE MEMBERS. CHASING SOME OF THE BOLTS AND NUTS MAY BE REQUIRED.
- ANCHOR BOLTS (HEAVY HEX HEAD, ASTM F1554, GRADE 55) SHALL BE HOT DIPPED GALVANIZED PER ASTM F2329. ANCHOR BOLTS MAY BE HEADED OR THREADED WITH A NUT THAT IS PREVENTED FROM ROTATING.
- HIGH STRENGTH NUTS SHALL CONFORM TO ASTM A563.
- HIGH STRENGTH WASHERS SHALL CONFORM TO ASTM F436.
- THE BOLTING INSTALLATION REQUIREMENTS OUTLINED BELOW ARE CRITICAL TO THE STRUCTURE'S DESIGN AND PERFORMANCE. THE INSTALLER IS REQUIRED TO COORDINATE THIS PHASE OF CONSTRUCTION WITH THE SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD PRIOR TO THE ERECTION OF THE FRAME. ALL BOLTS SHALL BE INSTALLED AND INSPECTED PER THE APPLICABLE VERSION OF AISC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS", CBC 1705A.2.1; AISC 341-10 J7; AISC 360-10 N5.6.
 - PRETENSIONED JOINTS (IDENTIFIED ON THE FRAME CONNECTION DETAILS WITH A "PJ REQUIRED") MUST BE INSTALLED AND INSPECTED TO MEET ONE OF FOLLOWING REQUIREMENTS:
 - TURN-OF-NUT PRETENSIONING
 - 2. CALIBRATED WRENCH PRENTENSIONING
 - 3. DIRECT-TENSION-INDICATOR PRETENSIONING (CONTRACTOR RESPONSIBLE FOR PURCHASE OF REQUIRED WASHERS)
- B. ALL OTHER JOINTS MUST BE INSTALLED AND INSPECTED TO MEET THE REQUIREMENTS OF SNUG-TIGHTENED JOINTS. NOTE TO INSTALLER AND INSPECTOR(S): THE SNUG-TIGHT CONDITION EXISTS, IN PART, WHEN ALL THE BOLTS IN THE JOINT HAVE BEEN TIGHTENED SUFFICIENTLY TO PREVENT THE REMOVAL OF THE NUTS WITHOUT
- THE CONTRACTOR, SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD MUST ALL AGREE ON WHICH APPROACH WILL BE USED TO PRETENSION THE BOLTS. THE CONTRACTOR IS RESPONSIBLE FOR DOCUMENTING THE APPROACH AGREED TO BY ALL PARTIES LISTED ABOVE.

FOUNDATIONS:

- ALLOWABLE SOIL PRESSURES ASSUME CLASS 4 SOIL CLASSIFICATION PER CBC TABLE 1806A.
- A GEOTECHNICAL REPORT / LETTER IS REQUIRED AT THE OVER-THE-COUNTER APPOINTMENT FOR EACH PROJECT. FILL AND BACKFILL SHALL BE COMPACTED TO 95% OF MAX. DENSITY IN ACCORDANCE WITH ASTM TEST METHOD D1557-70. FLOODING NOT PERMITTED.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING, ETC. NECCESSARY TO SUPPORT CUT AND/OR FILL BANKS DURING EXCAVATION, AND FORMING AND PLACEMENT OF CONCRETE.

CONCRETE:

FRAME WIDTH 🔀 20'

ROOF DECK TYPE

DESCRIPTIO

EFERENCE DSA BU 14-01 FOR A MAP OF VARIOUS SEREGION

ROOF DECK

COLLATERAL

MIX DESIGN REQUIREMENTS: (NORMAL WEIGHT CONCRETE

		STRENGTH f'c (28 DAYS)	W/C RATIO (NON-AIR ENTRAINED)	W/C RATIO (AIR ENTRAINED)	SLUMP (± 1")	UNIT WEIGHT (NORMAL WEIGHT)	
-		5000 PSI	0.63	0.55	3"	150 PCF	
	2.	CHANGES TO THE MIX DESIGN MUST BE APPROVED BY THE ENGINEER OR ARCHITECT OF RECORD AND DSA					
	3.	AGGREGATES SHALL CONFORM TO ASTM C33 WITH PROVEN SHRINKAGE CHARACTERISTICS OF LESS THAN .00. MAX AGGREGATE SIZE = 1".					

☐ (30' MAX)

¥ 40'-0" (NOMAX)

MAX DEAD LOAD

5 PSF

3.5 PSF

2 PSF

2 PSF

- CEMENT SHALL CONFORM TO ASTM C150 (TYPE V) UNLESS NOTED OTHERWISE ON THE DRAWINGS. CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF FIVE DAYS AFTER PLACEMENT.
- ALTERNATE METHODS WILL BE APPROVED IF SATISFACTORY PERFORMANCE CAN BE ASSURED. CONCRETE SHALL NOT FREE FALL MORE THAN FIVE FEET.
- CONCRETE SHALL BE PROPORTIONED PER ACI 318-11 5.2.

□ **3**0'

ROOF DECK

Ss ACCELERATION (g)

TOTAL ROOF DEAD LOAD

Ss REGIONS

 $0.000 < Ss \le 1.875$

1.875 < \$s <= 2.500

2.500 < Ss <= 2.750

2.750 < Ss <= 3.000

O PSF LIGHTING, FIRE SUPPRESSION, PV PANELS, ETC.

1.8 PSF ADD ROOF DECK AND COLLATERAL LOADS

MR = 1.2 PSF; SS = 1.8 PSF (SEE STEP 2)

□ WHITE

a BLUE

™ GREEN

YELLOW

DEAD LOAD

PROVIDE DSA WITH EVIDENCE THAT THE COLLATERAL LOAD FOR YOUR PROJECT MEETS THESE REQUIREMENTS

1.8 PSF

CONCRETE SHALL BE TESTED PER CBC 1905A.1.2, 1913A.1, 1705A.3, AND ACI 318-11 5.6.

REINFORCING STEEL:

- REINFORCING STEEL SHALL BE DEFORMED STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A615, (DEFORMATIONS SHALL BE IN ACCORDANCE WITH ASTM A305) AS FOLLOWS: GR 60: (#4 BARS AND LARGER) GR 40: (#3 BARS)
- DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS SHALL CONFORM THE ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCING CONCRETE STRUCTURES."
- MIN. COVER FOR CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS: . CAST AGAINST FORM BELOW GRADE..... FORMED SLABS (#11 BAR & SMALLER)....
- D. SLABS ON GRADE (FROM TOP OF SLÁB).. COLUMNS AND BEAMS (MAIN BARS)...
- F. WALLS EXPOSED TO WEATHER (#6-#18 BARS)...... 1#5 & SMALLER).... G. NOT EXPOSED TO WEATHER (#11 & SMALLER)...... 3/4"
- BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIAL LIKELY TO IMPAIR BOND. BENDS SHALL BE MADE COLD. REINFORCING SHALL BE LAP SPLICED 45 BAR DIA. MINIMUM IN CONCRETE AND MUST COMPLY WITH ACI 318-11. PRIOR TO PLACING OF CONCRETE, REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE WELL SECURED IN POSITION.
- WELDING OF REINFORCING IS NOT ALLOWED REINFORCING STEEL SHALL BE SAMPLED AND TESTED PER CBC 1913A.2.

POWDER COATED AND EPOXY PRIMED FINISH:

- ENTIRE POWDER COATING PROCESS COMPLETED IN SAME FACILITY AS STEEL FABRICATION.
- ALL CARBON STEEL MEMBERS (COLUMNS, BEAMS, PLATES, ETC.) PAINTED WITH PRIME COAT PER THE "AISC CODE OF STANDARD PRACTICE" AND THE "AISC SPECIFICATION SECTION M3" (UNLESS NOTED OTHERWISE).
- PARTS PRETREATED IN A 3 STAGE IRON PHOSPHATE WASHER (OR EQUAL).
- EPOXY PRIMER POWDER COAT APPLIED TO PARTS FOR SUPERIOR CORROSION PROTECTION.
- TOP POWDER COAT OF SUPER DURABLE TGIC (COLOR SELECTED FROM MANUFACTURER'S STANDARD OPTIONS OR
- SAMPLE PRODUCTION PARTS TESTED TO MEET THE FOLLOWING CRITERIA:
- A. SALT SPRAY RESISTANCE PER ASTM B 117/ ASTM D 1654

MILES PER HOUR

- 1. 10000 HOURS WITH NO CREEP FROM SCRIBE LINE AND RATING OF 10 B. HUMIDITY RESISTANCE PER ASTM D2247-02
- 5000 HOURS WITH NO LOSS OF ADHESION OR BLISTERING C. COLOR/UV RESISTANCE PER ASTM G154-04
- 2000 HOURS EXPOSURE ALTERNATE CYCLES WITH NO CHALKING, 75% COLOR RETENTION, AND COLOR VARIATION MAXIMUM 3.0 E VARIATION CIE FORMULA (BEFORE AND AFTER 2000 HOURS

ABBREVIATIONS:

LUNA BERT	FIATIONS.		
ACI	AMERICAN CONCRETE INSTITUTE	MR	MULTI-RIB ROOF PANEL (MCELROY)
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	NTS	NOT TO SCALE
ASM	ASSEMBLY (INTERNAL REFERENCE)	NO	NUMBER
ASTM	AMERICAN SOCIETY FOR TESTING AND MAT'LS	oc	ON CENTER
AW\$	AMERICAN WELDING SOCIETY	OSHA	OCCUPATIONAL HEALTH AND SAFETY ADM.
СВС	CALIFORNIA BUILDING CODE	PCF	POUNDS PER CUBIC FOOT
CJP	COMPLETE JOINT PENETRATION	PD	POLIGON DRAWING
CLR	CLEAR	PJ	PRETENSIONED JOINT
DEG	DEGREE	PLCS	PLACES
DIA	DIAMETER	PLT	PLATE
DIM	DIMENSION	PSF	POUNDS PER SQUARE FOOT
DSA	DIVISION OF THE STATE ARCHITECT	PSI	POUNDS PER SQUARE INCH
EQ	EQUAL	QTY	QUANTITY
FT	FEET	REF	REFERENCE
GA	GAGE	RH	RIGHT HAND
IN	INCHES	\$Q	SQUARE
KŞI	KIPS PER SQUARE INCH	SS	STANDING SEAM ROOF PANEL (MCELROY)
LH	LEFT HAND	TYP	TYPICAL
MAX	MAXIMUM	UNO	UNLESS NOTED OTHERWISE
MIN	MINIMUM	USGS	U.S. GEOLOGICAL SURVEY
MISC	MISCELLANEOUS	W/	WITH
1			

² MAY NOT EXCEED REQUIREMENTS LISTED IN STEP 6 BELOW **FOUNDATION REQUIREMENTS** SPREAD DRILLED LOAD Ss REGION SCENARIO PIER DL <= 2 PSF □ LOAD SCENARIO 1 WHITE □ LOAD SCENARIO 2 2 PSF < DL <= 5 PSF DL <= 3.5 PSF LOAD SCENARIO 3 GREEN DL <= 2 PSF **⋈ LOAD SCENARIO 4** YELLOW DL <= 2 PSF □ LOAD SCENARIO 4

STEP 7: SELECT MISCELLANEOUS OPTIONS FOR YOUR PROJECT - MAXIMUM CLEAR HEIGHT IS 10'-0"; (SEE 'ARCHITECTURAL VIEWS' SHEET FOR REFERENCE) - MARK UP PC DRAWINGS WITH SIZE AND LOCATION OF CUTOUTS BEFORE SUBMITTING TO DSA

STEP & SELECT APPLICABLE SHEET INDEX FOR YOUR PROJECT - REFERENCE THE BASE FRAME (STEP 1) AND THE ROOF DECK TYPE (STEP 2) IDENTIFY THE APPLICABLE SHEET INDEX

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STEP 9: INCLUDE APPLICABLE SHEETS WITH YOUR DSA SUBMITTAL - EXCLUDE 'MISC DESIGN OPTIONS' SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUTS OR GUTTERS

STEP 10: IDENTIFY PROJECT NAME AND SCHOOL DISTRICT PROJECI NAME Valley View Elementary School SCHOOL DISTRICT

ROOF DEC! MR SS MR SS SELECT ONE PD1.0 PD1.0 PD1.0 PD1.0 PD1.1 PD1.1 PD1.1 SPECIAL INSPECTIONS PD2.0 PD2.0 PD2.1 PD2.1 FOUNDATION PLAN PD3.0 PD3.0 PD3.1 PD3.1 FRAMING PLAN PD4.1 PD4.1 PD4.0 | PD4.0 | PD5.0 PD5.0 SECTION DETAILS PD5.1 PD5.1 -PD6.0 - PD6.1 PLATE DETAILS (PD6.2 - PD6.3 PD7.1 PD7.1 PD7.0 PD7.0 ARCHITECTURAL VIEWS PD8.0 PD8.1 PD8.0 PD8.1 OOF CONNECTION DETAILS PD9.0 PD9.0 PD9.0 PD9.0 MISC DESIGN OPTIONS

FLECTRICAL CUTOUTS

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DESIGN OPTIONS

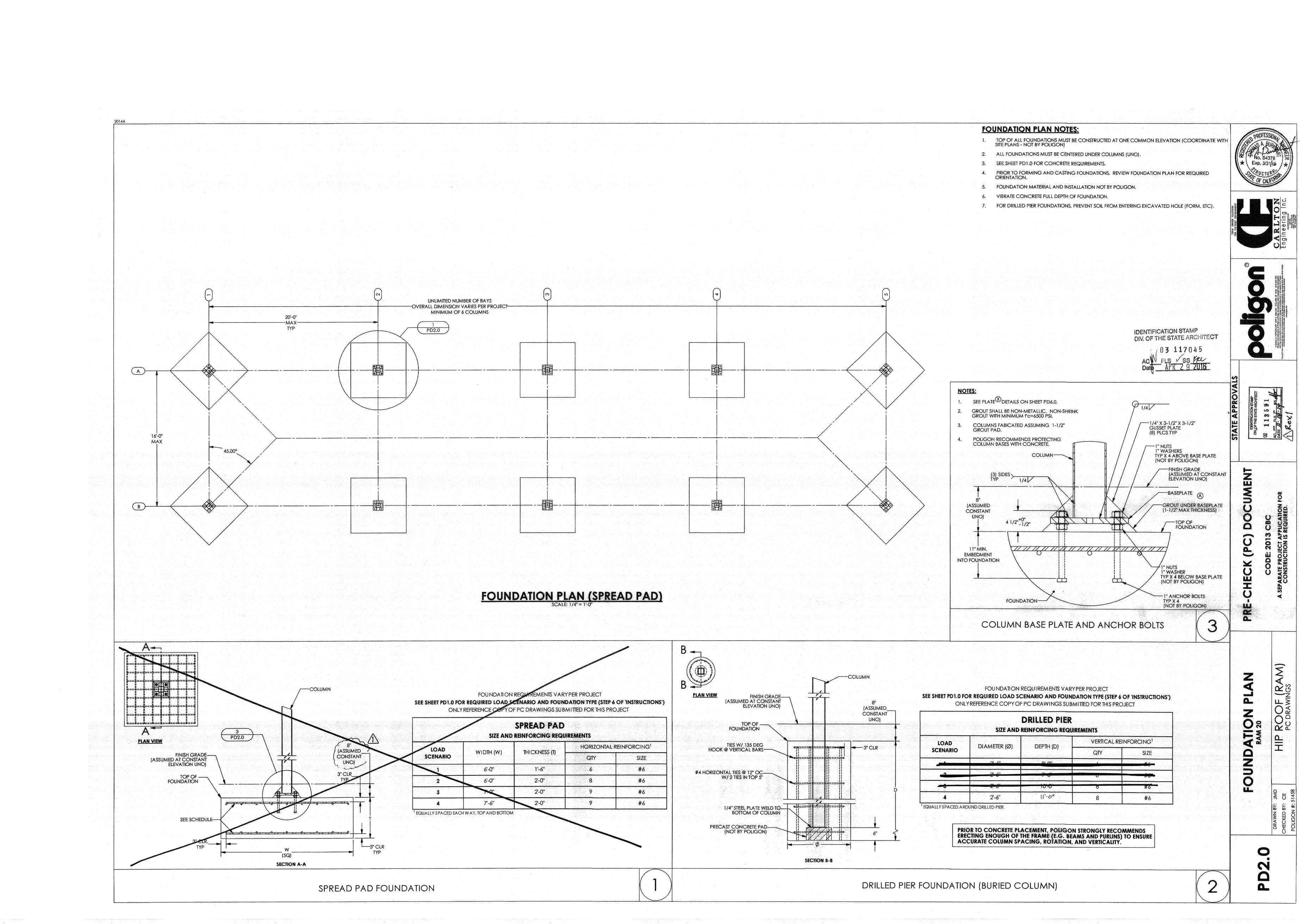
(10, WYX)

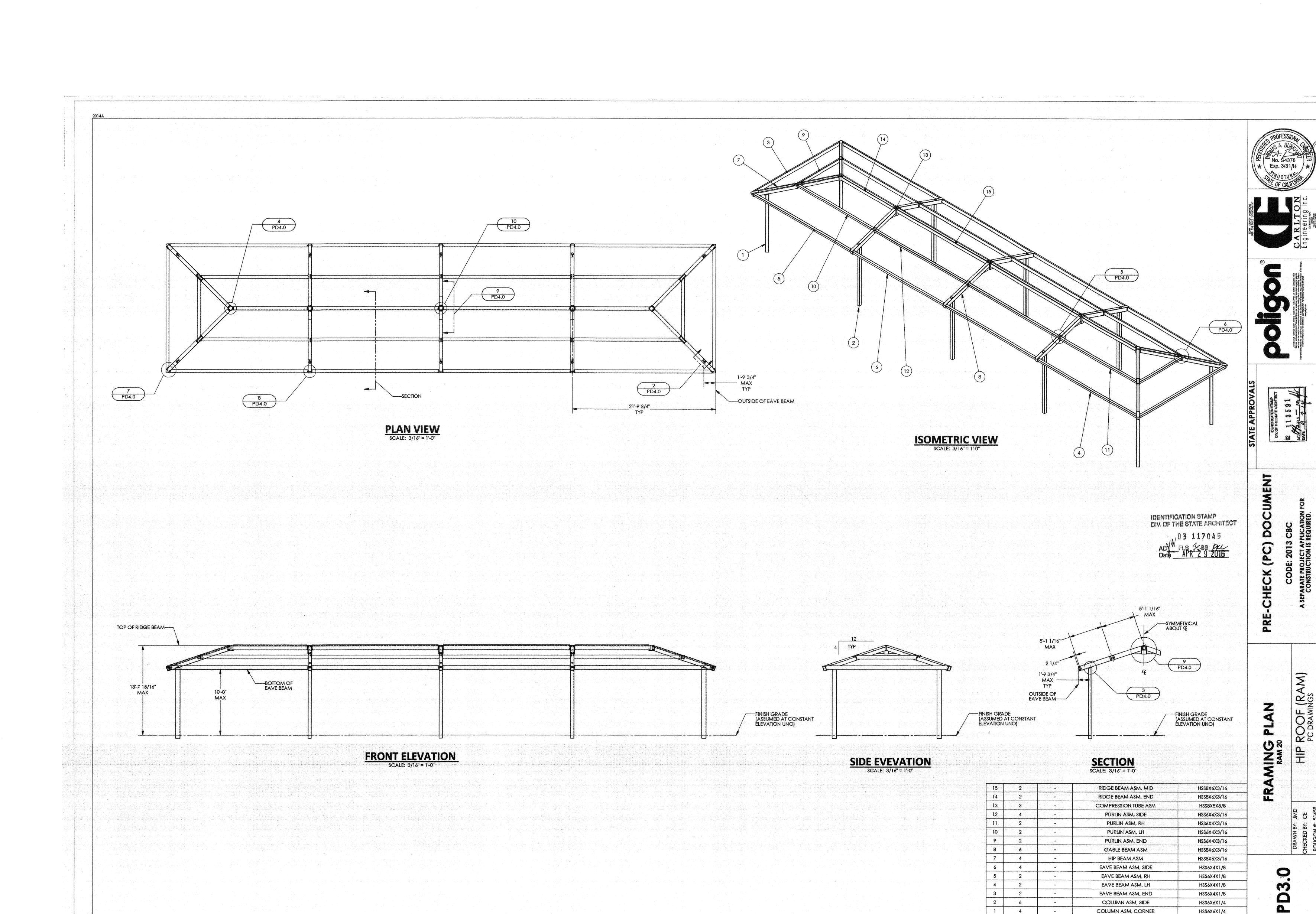
× 10'-0"

SHEET INDEX

□ YES

0





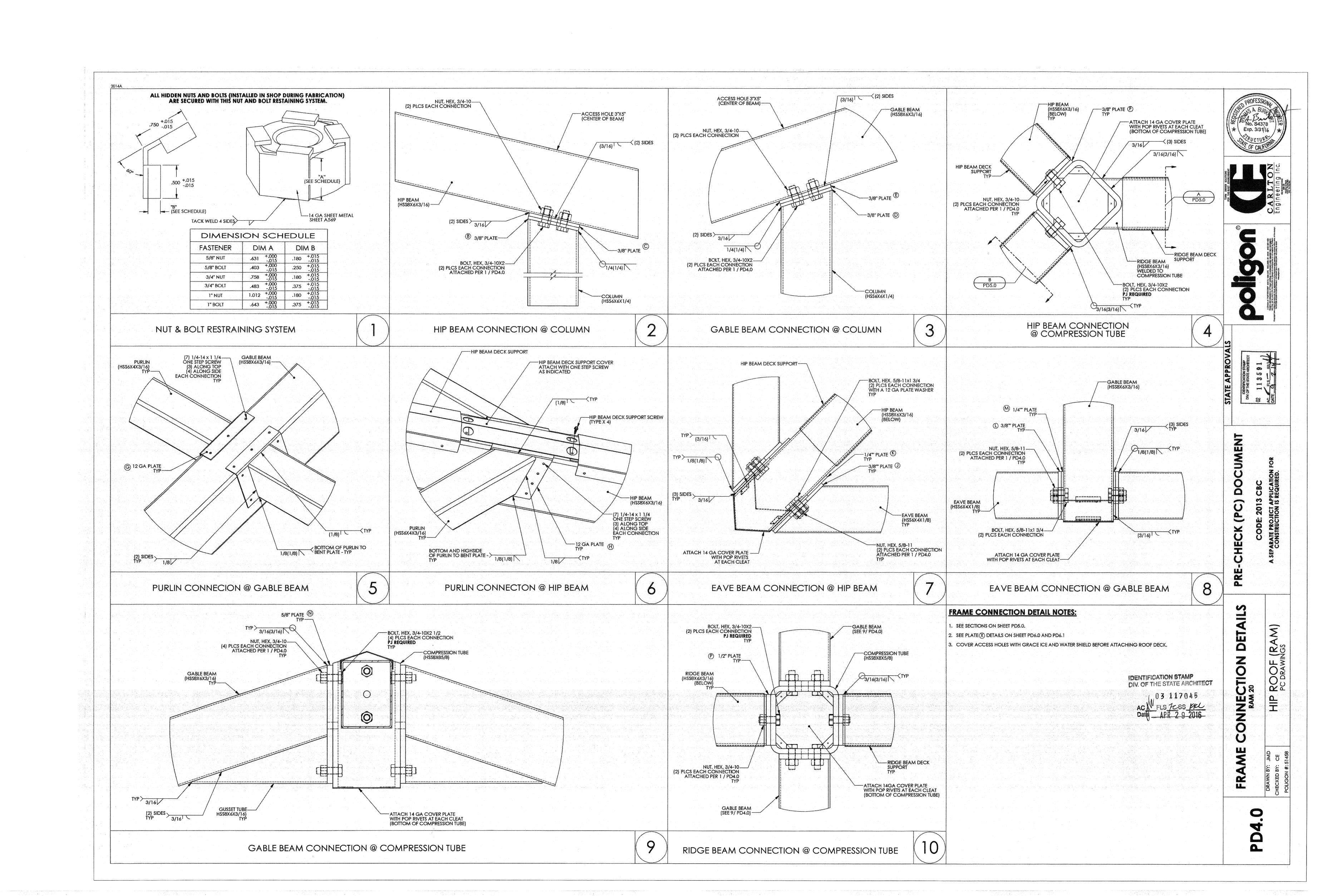
ITEM FRAME /QTY. PART NO.

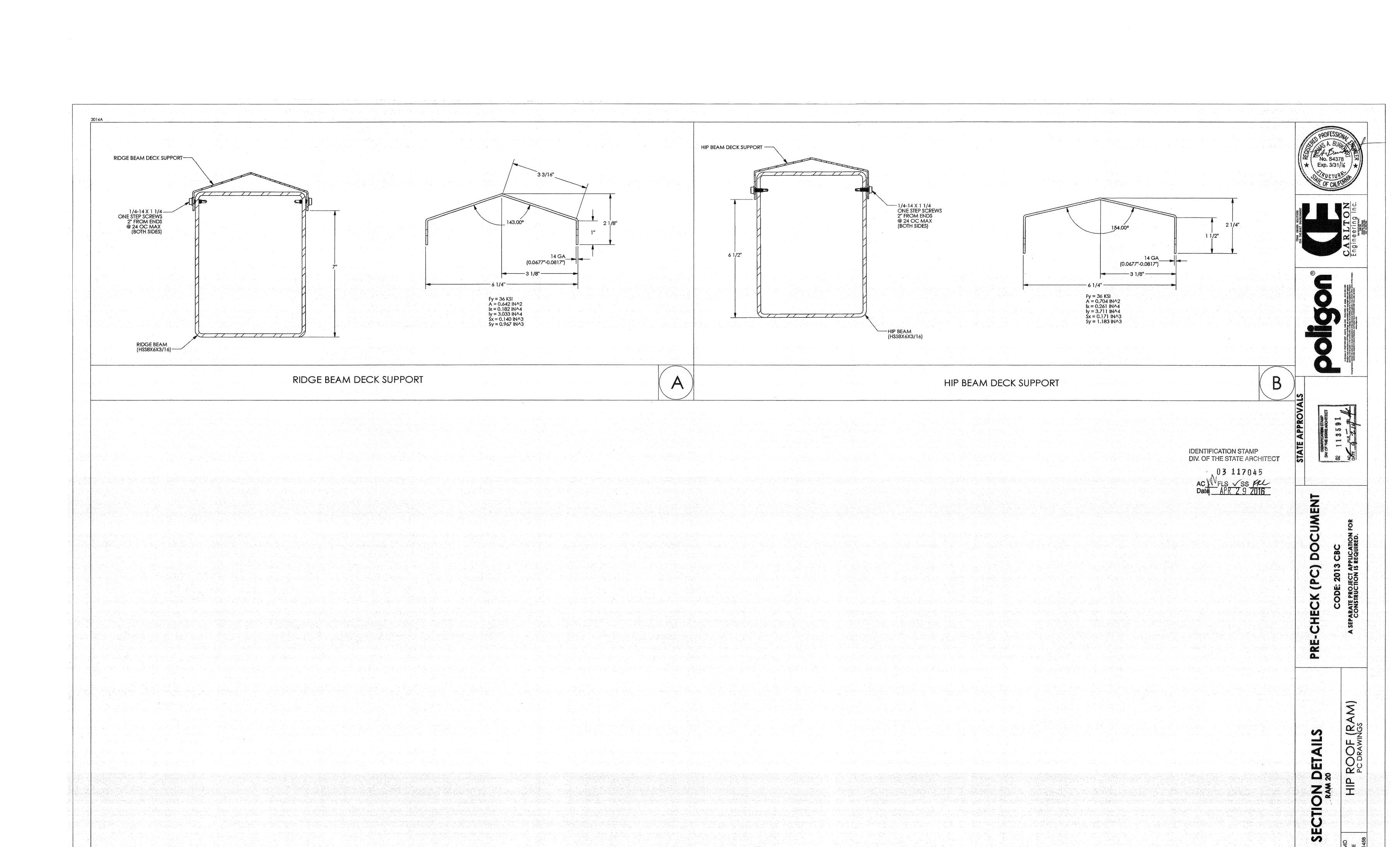
COLUMN ASM, CORNER

DESCRIPTION

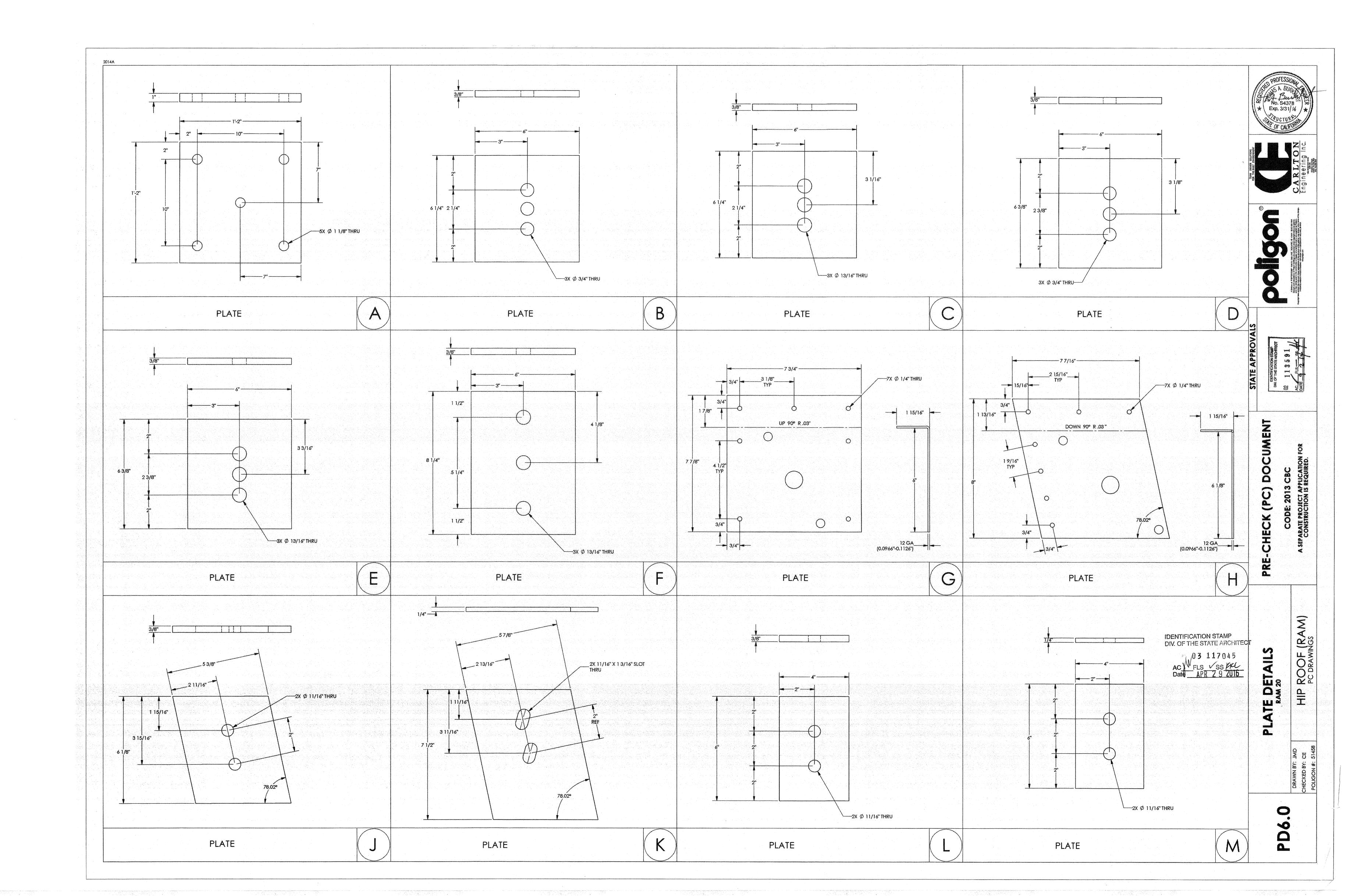
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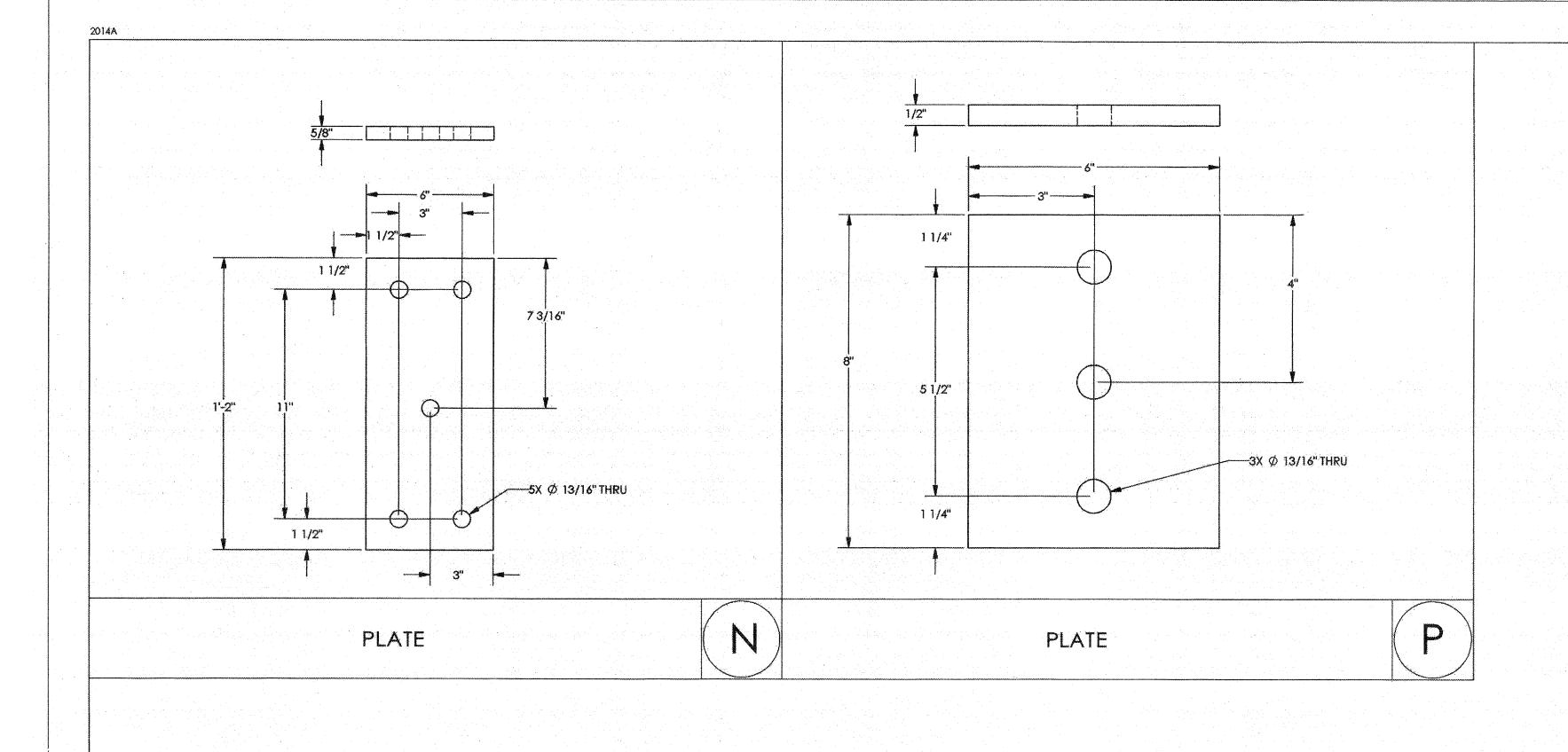
MATERIAL

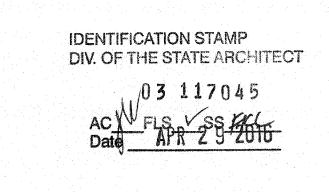


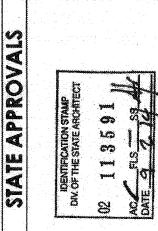


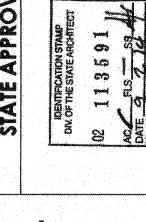
D5.0



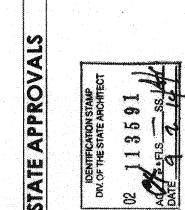




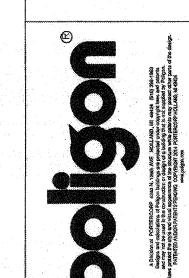


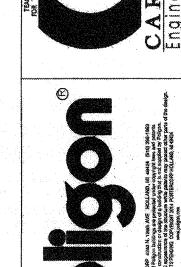


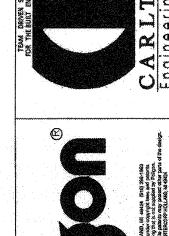
HIP ROOF (RAM) PC DRAWINGS

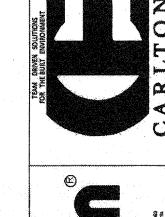


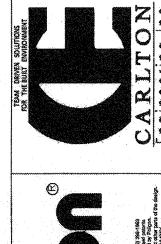


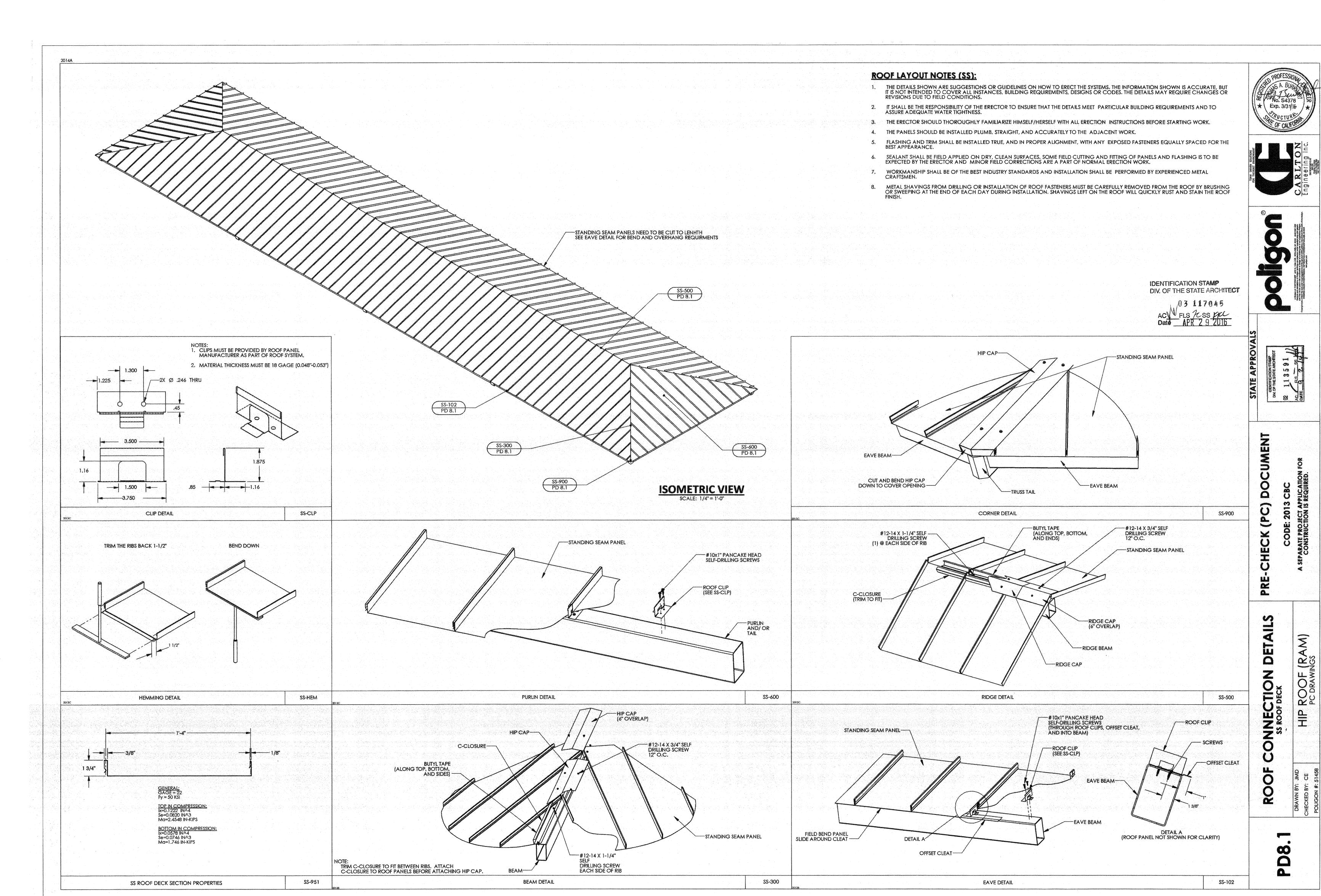


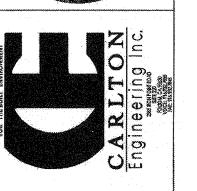


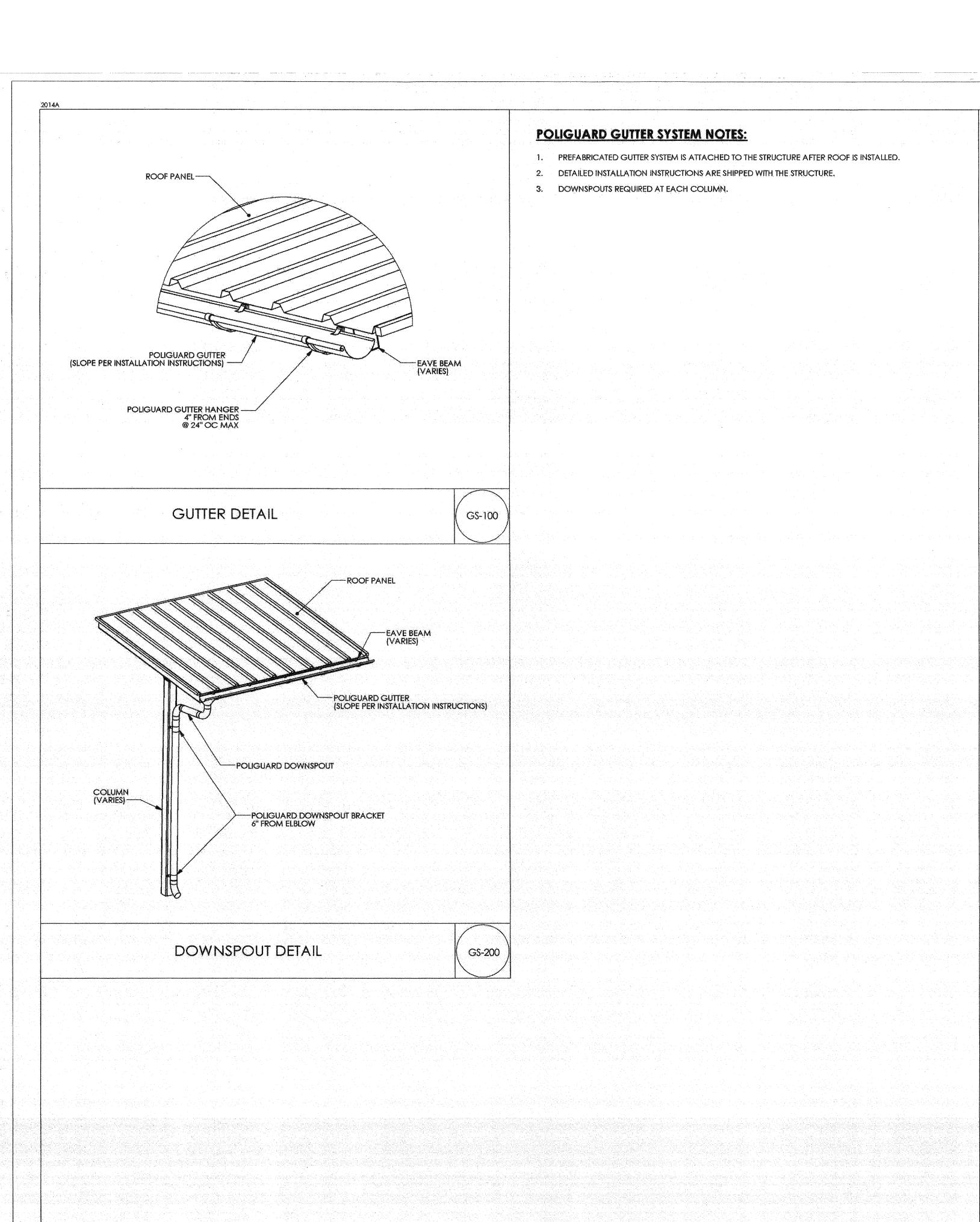


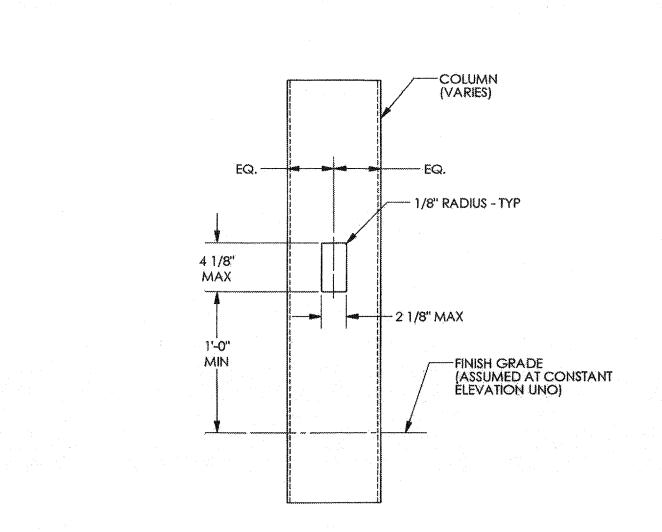


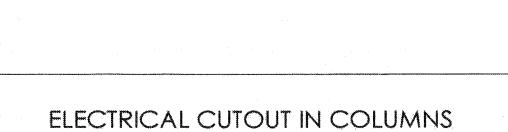


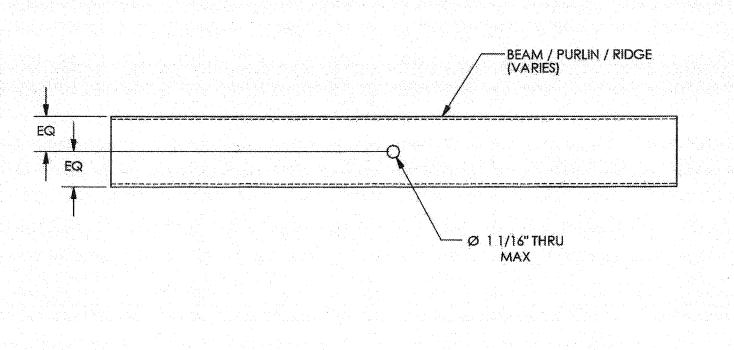












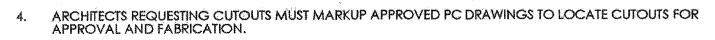
ELECTRICAL CUTOUT IN BEAMS / PURLINS / RIDGES

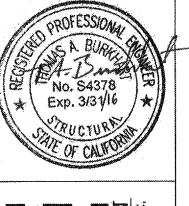
ELECTRICAL CUTOUT NOTES:

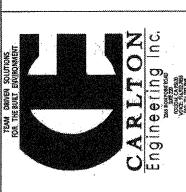
EC-100

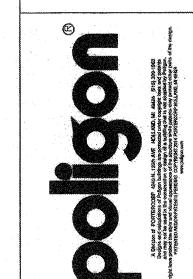
EC-200

- 1. MAXIMUM ONE CUTOUT PERMITTED IN EACH MEMBER.
- 2. CUTOUTS CAN BE PLACED ON ANY SIDE OF A MEMBER.
- 3. CUTOUTS CAN BE PLACED ALONG MEMBERS AS INDICATED IN THE DETAILS.

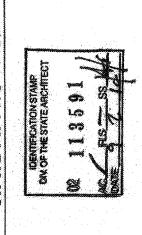








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PRE-CHECK (PC) DOCUMENT

HIP ROOF (RAM) ESIGN OPTIONS

O.