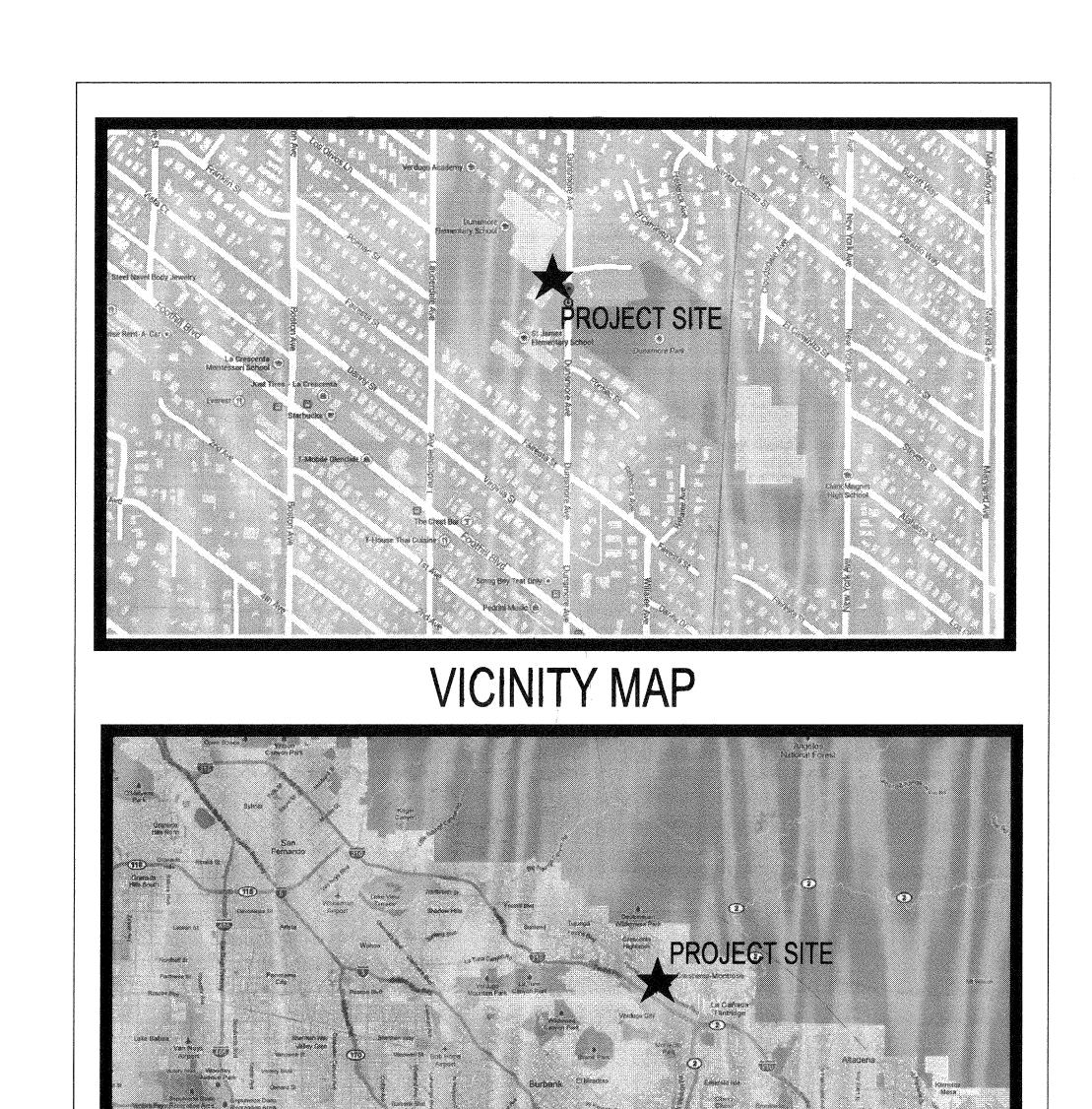
# NEW SHADE STRUCTURE DUNSMORE ELEMENTARY SCHOOL



4717 DUNSMORE AVE., LA CRESCENTA, CALIFORNIA

## GLENDALE UNIFIED SCHOOL DISTRICT

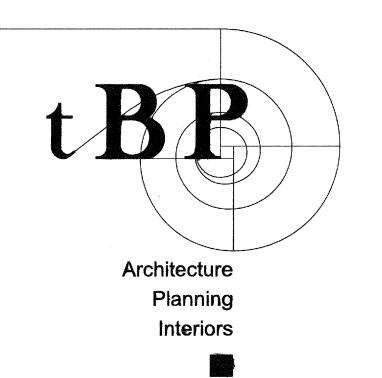


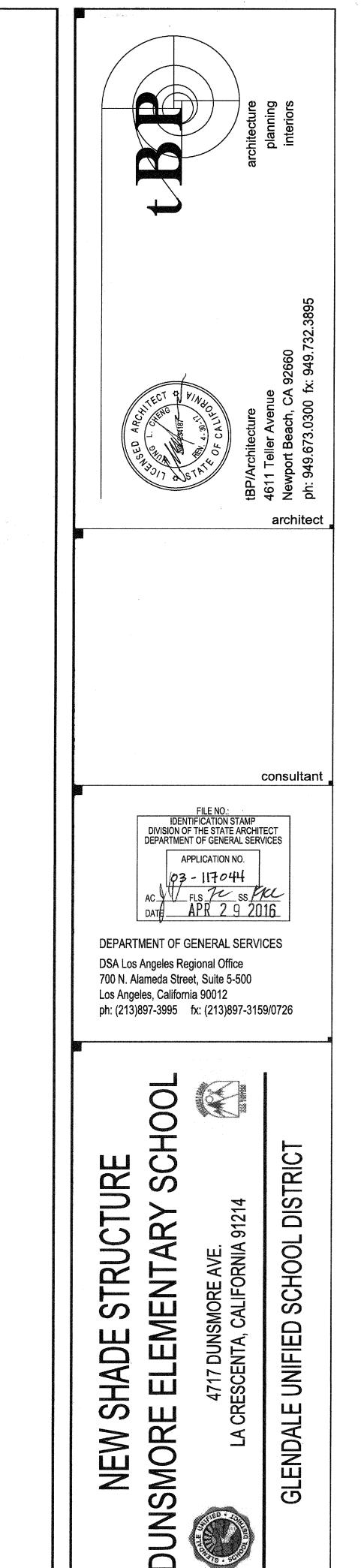


**REGIONAL MAP** 

tBP /Architecture

4611 Teller Avenue - Newport Beach - California - 92660 http://www.tbparchitecture.com ph: 949.673.0300 - fx: 949.732.3895





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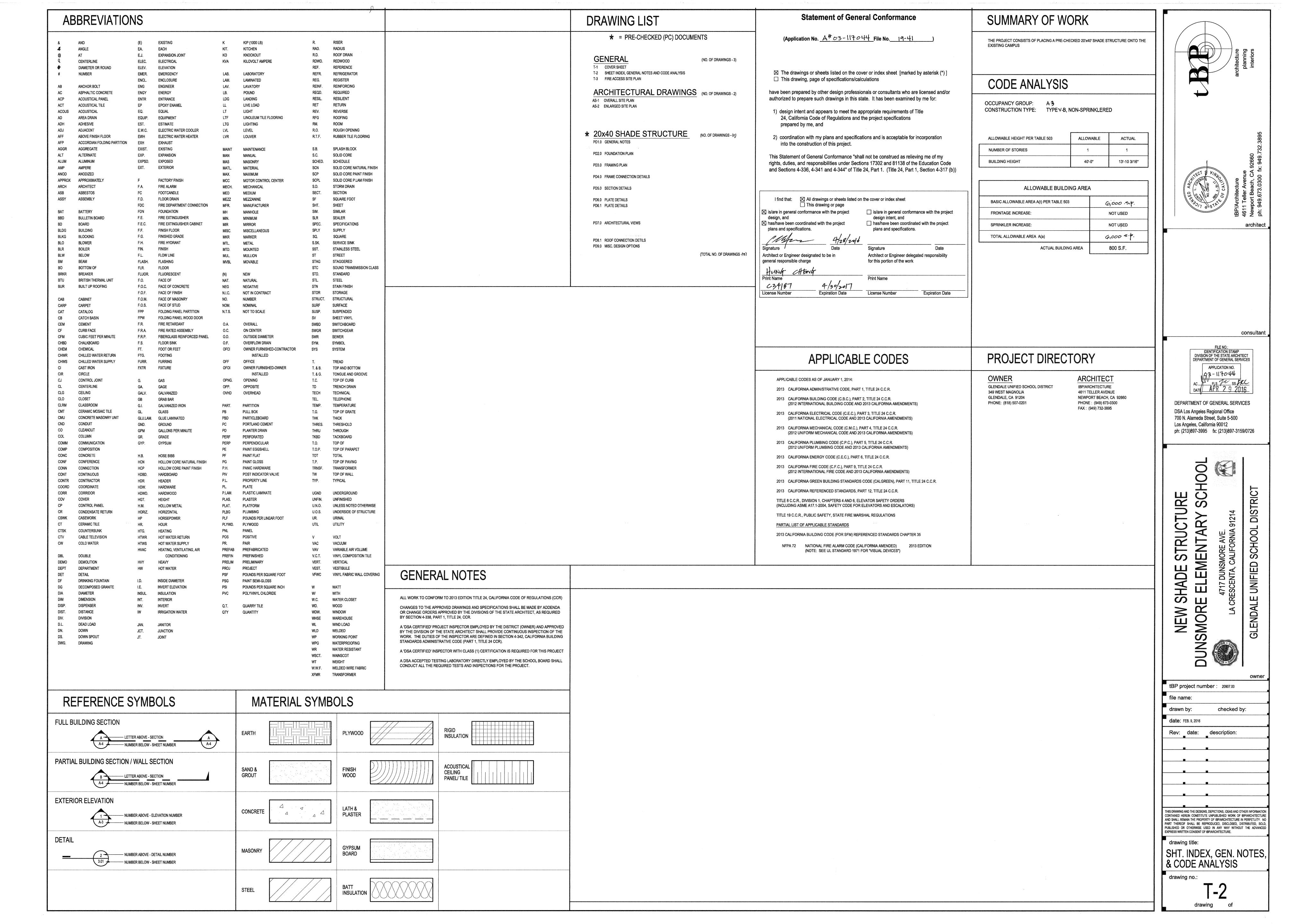
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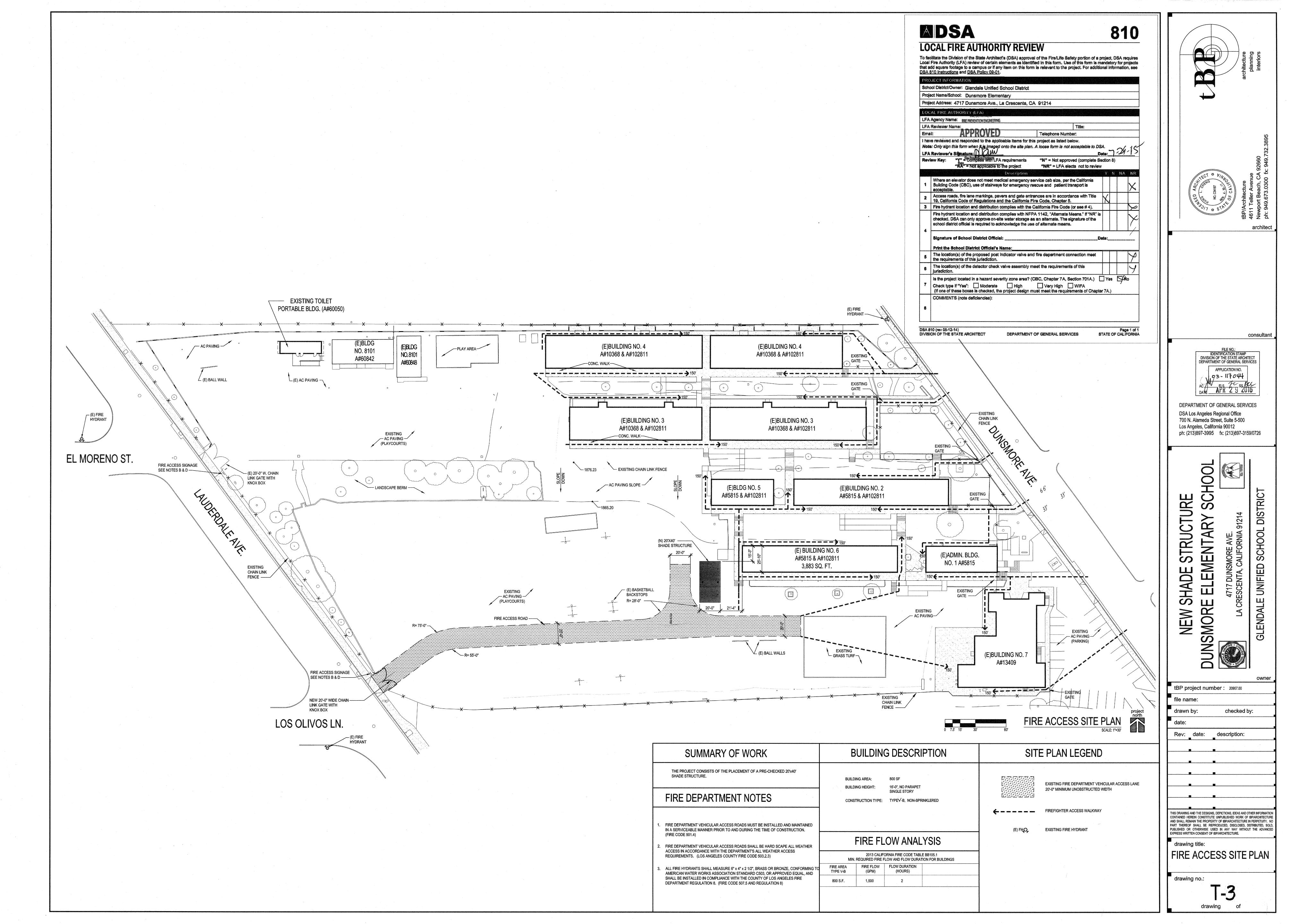
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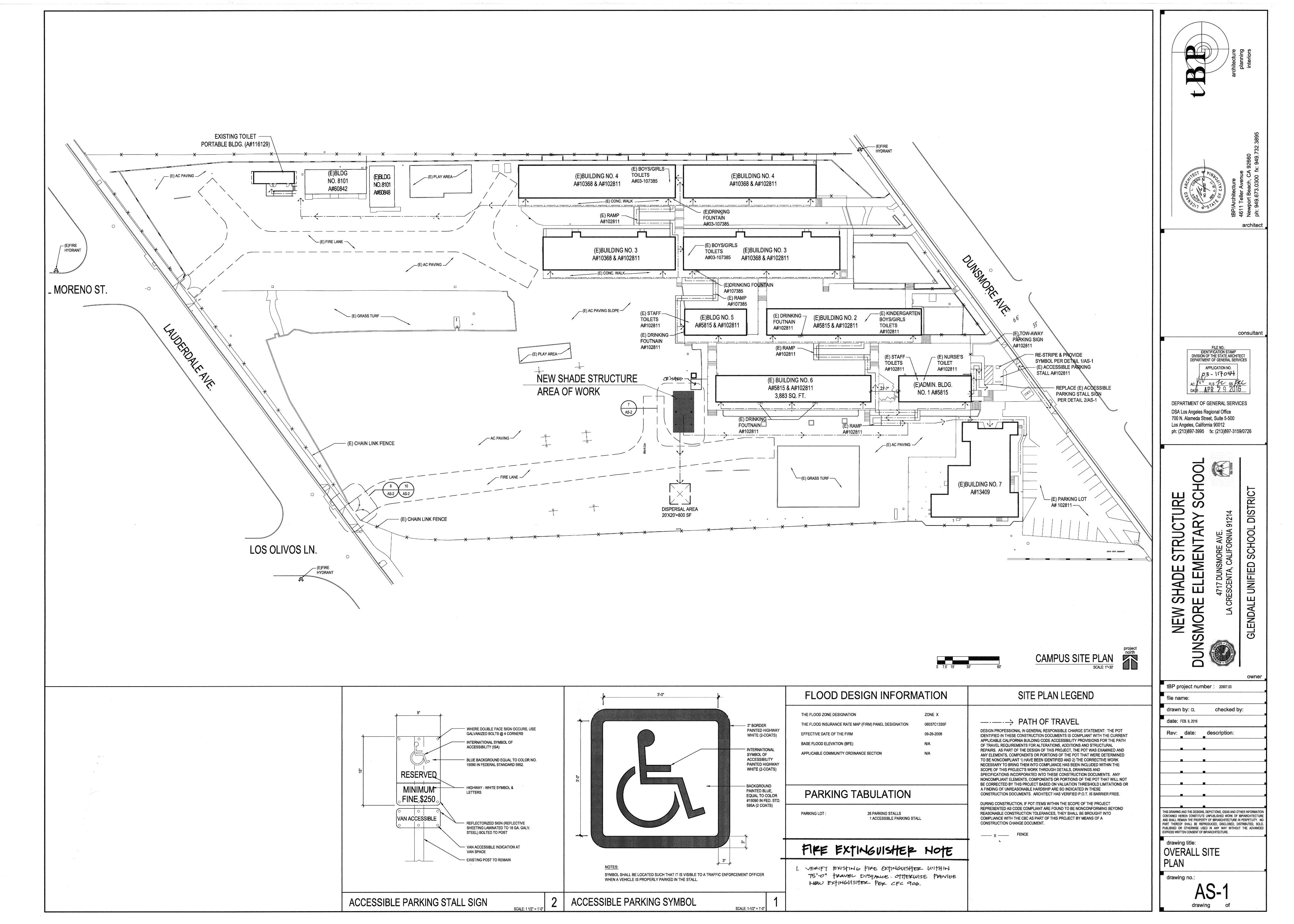
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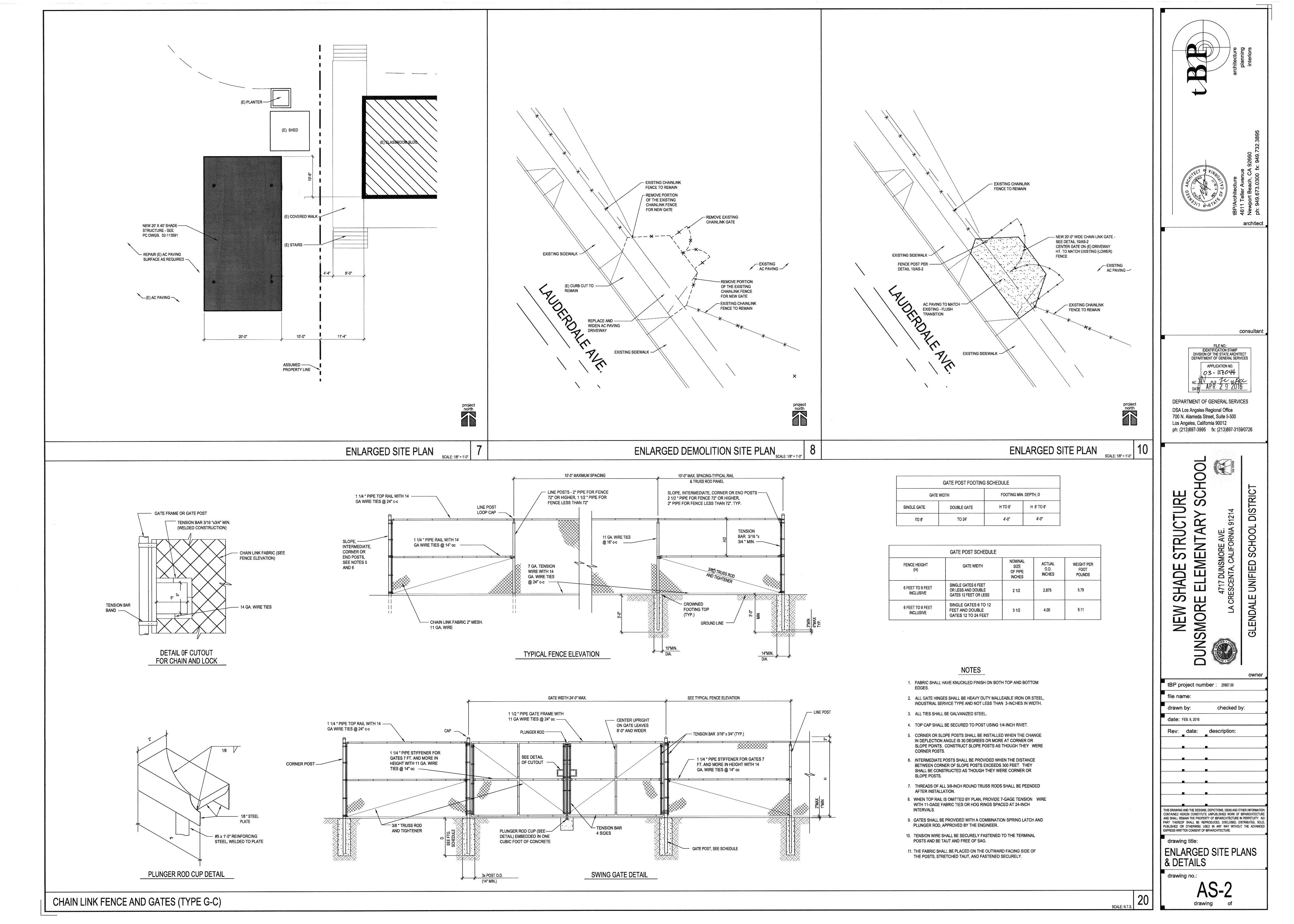
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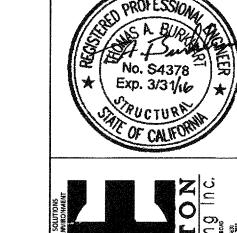
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BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIAL LIKELY TO IMPAIR BOND. BENDS SHALL BE MADE COLD. REINFORCING SHALL BE LAP SPLICED 45 BAR DIA. MINIMUM IN CONCRETE AND MUST COMPLY WITH ACI 318-11. PRIOR TO PLACING OF CONCRETE, REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE WELL SECURED IN POSITION.

ALL CARBON STEEL MEMBERS (COLUMNS, BEAMS, PLATES, ETC.) PAINTED WITH PRIME COAT PER THE "AISC CODE OF TOP POWDER COAT OF SUPER DURABLE TGIC (COLOR SELECTED FROM MANUFACTURER'S STANDARD OPTIONS OR

DESIGN VALUES:	
DESCRIPTION	DESIGN VALUES
DEAD AND LIVE LOADS	
ROOF LIVE LOAD	20 PSF
ROOF DEAD LOAD (SUPERIMPOSED ON FRAME)	VARIES
ALLOWABLE SOIL PRESSURE	
DL+LL (CONCRETE FOOTING)	2000 PSF
DL+LL+SEISMIC (CONCRETE FOOTING)	2000 PSF
ROOF SNOW LOAD	
GROUND SNOW LOAD, Pg, FROM COUNTY	10 PSF
ISK CATEGORY	[X]    [ ]    [ ]
OOF SNOW LOAD: [ ] FLAT, PF OR [ ] LOW-SLOPE, Pm OR [X] SLOPED, Ps	10 PSF
NOW ROOF SLOPE FACTOR, Cs	1.0
NOW EXPOSURE FACTOR, Ce	1.2
NOW LOAD IMPORTANCE FACTOR, IS	[X] 1.0 [ ] 1.1
HERMAL FACTOR, C1	[]1.0 [X]1.2
FLOOD DESIGN	
LOOD HAZARD AREA: [ ] YES [X] NO	
WIND DESIGN	
ASIC WIND SPEED (3 SECOND GUST), Vult	110 MPH
RISK CATEGORY	[X] II [ ] III
XPOSURE CATEGORY	[X] C [ ] D
OPOGRAPHIC FACTOR, Kzt (1 MINIMUM)	1.0
NTERNAL PRESSURE COEFFICIENT, GCpi (IF APPLICABLE)	0.0
SEISMIC DESIGN	
	STEEL ORDINARY CANTILEVER
ATERAL FORCE-RESISTING SYSTEM	COLUMN SYSTEMS
	EQUIVALENT LATERAL FORCE
NALYSIS PROCEDURE	PROCEDURE
EISMIC DESIGN CATEGORY (SDC)	E
EISMIC IMPORTANCE FACTOR, le	[X] 1.0 [ ] 1.25
DESIGN BASE SHEAR, V	Cs x W
	LOAD SCENARIO = { 1, 2, 3, 4 }
EISMIC RESPONSE COEFFICIENT, Cs	Cs = { 0.80, 0.80, 1.07, 1.28 }
RESPONSE MODIFICATION FACTOR, R	1.25
ITE CLASS	[X] D [ ] E
REDUNDANCY FACTOR, p	1.3
MAPPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Ss - USED TO	LOAD SCENARIO = { 1, 2, 3, 4 } Ss (MAX) = { 1.875, 1.875, 2.500, 3.000
NETERMINE CE (WITH CAP PER CRC 14144 1 12)	
DETERMINE Cs (WITH CAP PER CBC 1616A.1.12)	
MAPPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Ss - USED TO	
MAPPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Ss - USED TO DETERMINE OTHER PARAMETERS AND NON-STRUCTURAL COMPONENT	3.00
MAPPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Ss - USED TO DETERMINE OTHER PARAMETERS AND NON-STRUCTURAL COMPONENT INHORAGE (NO CAP)	
MAPPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Ss - USED TO DETERMINE OTHER PARAMETERS AND NON-STRUCTURAL COMPONENT INHORAGE (NO CAP)	1.0
MAPPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Ss - USED TO DETERMINE OTHER PARAMETERS AND NON-STRUCTURAL COMPONENT INHORAGE (NO CAP)  HORT-PERIOD SITE COEFFICIENT, FOR STRUCTURAL RESPONSE ACCELERATION AT SHORT PERIOD, School See See See See See See See See See Se	1.0 LOAD SCENARIO = { 1, 2, 3, 4}
MAPPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Ss - USED TO DETERMINE OTHER PARAMETERS AND NON-STRUCTURAL COMPONENT UNHORAGE (NO CAP)  HORT-PERIOD SITE COEFFICIENT, Fa  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Sds - USED TO	1.0
MAPPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Ss - USED TO DETERMINE OTHER PARAMETERS AND NON-STRUCTURAL COMPONENT (NHORAGE (NO CAP))  HORT-PERIOD SITE COEFFICIENT, Fa  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Sds - USED TO DETERMINE Cs (WITH CAP PER CBC 1616.1.12)  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Sds - USED TO	1.0 LOAD SCENARIO = { 1, 2, 3, 4} Sds (MAX) = { 1.00, 1.00, 1.33, 1.60 }
MAPPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Ss - USED TO DETERMINE OTHER PARAMETERS AND NON-STRUCTURAL COMPONENT UNHORAGE (NO CAP)  HORT-PERIOD SITE COEFFICIENT, Fa  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Scs - USED TO DETERMINE Cs (WITH CAP PER CBC 1616.1.12)  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Scs - USED TO DETERMINE OTHER PARAMETERS AND NON-STRUCTURAL COMPONENTS	1.0 LOAD SCENARIO = { 1, 2, 3, 4}
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MAPPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Ss - USED TO DETERMINE OTHER PARAMETERS AND NON-STRUCTURAL COMPONENT (NHORAGE (NO CAP))  HORT-PERIOD SITE COEFFICIENT, Fa  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Sds - USED TO DETERMINE Cs (WITH CAP PER CBC 1616.1.12)  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Sds - USED TO DETERMINE OTHER PARAMETERS AND NON-STRUCTURAL COMPONENTS ANCHORAGE (NO CAP)	1.0 LOAD SCENARIO = { 1, 2, 3, 4} Sds (MAX) = { 1.00, 1.00, 1.33, 1.60 }
AAPPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Ss - USED TO DETERMINE OTHER PARAMETERS AND NON-STRUCTURAL COMPONENT ANHORAGE (NO CAP)  HORT-PERIOD SITE COEFFICIENT, Fa  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Sds - USED TO DETERMINE Cs (WITH CAP PER CBC 1616.1.12)  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Sds - USED TO DETERMINE OTHER PARAMETERS AND NON-STRUCTURAL COMPONENTS ANCHORAGE (NO CAP)  MAPPED SPECTRAL RESPONSE ACCELERATION AT 1 SECOND PERIOD, S1  ONG-PERIOD SITE COEFFICIENT, FV	1.0 LOAD SCENARIO = { 1, 2, 3, 4} Sds (MAX) = { 1.00, 1.00, 1.33, 1.60 } 2.00
MAPPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Ss - USED TO DETERMINE OTHER PARAMETERS AND NON-STRUCTURAL COMPONENT ANHORAGE (NO CAP)  HORT-PERIOD SITE COEFFICIENT, Fa  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Sds - USED TO DETERMINE Cs (WITH CAP PER CBC 1616.1.12)  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Sds - USED TO DETERMINE OTHER PARAMETERS AND NON-STRUCTURAL COMPONENTS ANCHORAGE (NO CAP)  MAPPED SPECTRAL RESPONSE ACCELERATION AT 1 SECOND PERIOD, S1	1.0 LOAD SCENARIO = { 1, 2, 3, 4} Sds (MAX) = { 1.00, 1.00, 1.33, 1.60 } 2.00

#### **ARCHITECTURAL REQUIREMENTS**

DESCRIPTION	DESIGN VALUES
TYPE OF CONSTRUCTION	VB
OCCUPANCY CLASSIFICATION	А3
NUMBER OF STORIES	1
FIRE HAZARD SEVERITY ZONE	VERY HIGH
FIRE SPRINKLER SYSTEM	NOT BY POLIGON

POLIGON ASSUMES ANY OCCUPANT LOAD CALCULATIONS ARE BASED ON 15 SQ FT/ PERSON. PROJECT ARCHITECT MAY ADJUST OCCUPANT LOAD AS PERMITTLED BY THE BUILDING CDE.

#### RELATED BUILDING CODES AND STANDARDS:

#### IITLE 24 CODES:

2013 California Administrative Code (CAC).................(Part 1, Title 24, CCR)
2013 California Building Code (CBC), Volumes 1, and 2 (Part 2, Title 24, CCR)
(2012 International Building Code with 2013 California amendments)
2013 California Florational Code
(Part 3, Title 24, CCR) 2013 California Electrica) Code ..... .....(Part 3, Title 24, CCR) National Electrical Code with 2013 California amendme ....(Part 4, Title 24, CCR) 2013 California Plumbing Code (CPC) .... ..(Part 5, Title 24, CCR) ...(Part 6, Title 24, CCR) Effective July 1, 2014) 

NFPA 13 - 2013 NFPA 72 - 2013

REFERENCE CODE SECTIONS FOR APPLICABLE STANDARDS:

2013 CBC, CHAPTER 35 2013 CFC, CHAPTER 45

#### SCOPE OF WORK NARRATIVE:

THESE DRAWINGS ILLUSTRATE THE FABRICATION AND INSTALLATION REQUIREMENTS FOR A FREE-STANDING PREFABRICATED STEEL SHADE STRUCTURE. THE ENTIRE STRUCTURAL SYSTEM IS COMPRISED OF TUBULAR STEEL MEMBERS SUPPORTED ON CONCRETE FOUNDATIONS. THE FLEXIBILITY INCLUDED HEREIN ALLOWS THIS STRUCTURE TO COMPLY WITH A WIDE VARIETY OF PROJECT SITES AND LOADING REQUIREMENTS.

- GENERAL NOTES AND TYPICAL DETAILS SHALL APPLY TO ALL PARTS OF THE JOB EXCEPT WHERE THEY MAY CONFLICT WITH DETAILS AND NOTES ON OTHER SHEETS. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER FOR THIS PROJECT.
- WORK SHALL CONFORM TO THE REQUIREMENTS, AS AMENDED TO DATE, OF THE LATEST ADOPTED EDITION OF THE CBC, C.A.C. TITLE 24, AND ALL OTHER LOCAL, STATE AND FEDERAL REGULATIONS.
- OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT PRIOR TO PROCEEDING WITH ANY WORK INVOLVED.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHALL CHECK ALL DIMENSIONS. ALL DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT AND BE RESOLVED BEFORE PROCEEDING WITH THE WORK.
- THESE CONSTRUCTION DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO, BRACING, TEMPORARY SUPPORTS, AND SHORING. OBSERVATION VISITS TO THE SITE BY FIELD REPRESENTATIVES OF THE ARCHITECT/ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. ANY SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER DURING THE CONSTRUCTION SHALL BE DISTINGUISHED FROM CONSTRUCTION AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER, WHETHER OF MATERIAL OR WORK, ARE FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DOCUMENTS, BUT DO NOT GUARANTEE
- ASTM DESIGNATIONS AND ALL STANDARDS REFER TO THE LATEST AMENDMENTS.
- CONFORM TO APPLICABLE CAL/OSHA CONSTRUCTION SAFETY REGULATIONS FOR ALL WORK PERFORMED DURING CONSTRUCTION. JOB SITE SAFETY IS STRICTLY THE RESPONSIBILITY OF THE CONTRACTOR AND NOT THE ARCHITECT/ENGINEER OR OWNER.
- THE ENGINEER AND THEIR CONSULTANTS SHALL HAVE NO RESPONSIBILITY FOR THE DISCOVERY, HANDLING, REMOVAL OR DISPOSAL OF HAZARDOUS MATERIALS AT THE PROJECT SITE, INCLUDING BUT NOT LIMITED, TO ASBESTOS, ASBESTOS PRODUCTS, POLYCHLORINATED BIPHENYL (PCB) OR OTHER TOXIC SUBSTANCES.
- SHOULD ANY CONDITIONS DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS, OR IF A CHANGE IN THE SCOPE OF WORK IS PROPOSED, A CONSTRUCTION CHANGE DOCUMENT DETAILING AND SPECIFYING THE REQUIRED CHANGE(S) SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK.
- THE SCHOOL DISTRICT'S INSPECTOR OF RECORD SHALL INSPECT AND APPROVE THE ERECTED FRAME PRIOR TO ROOF INSTALLATION. SEE REQUIREMENTS FOR LOCATION IN ANY FIRE HAZARD SEVERITY ZONE FOR WILDLAND URBAN INTERFACE AREAS
- (WUI) AS SPECIFIED IN THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. PROVIDE PROTECTION AND DETAILS OF ALL AREAS COMPLYING WITH THE WUI REQUIRMENTS.
- LOCATING THIS STRUCTURE CLOSER THAN 20 FEET TO OTHER STRUCTURES MAY AFFECT THE ALLOWABLE AREA FOR THE EXISTING CONSTRUCTION PER THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE.
- VIEWS AND DETAILS ARE NOT DRAWN TO SCALE (UNLESS NOTED OTHERWISE). DO NOT SCALE THESE DRAWINGS.
- OTHER SITE SPECIFIC ITEMS MAY BE REQUIRED.

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- ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) SPECIFIATION MANUAL REFERENCED BY THE LATEST EDITION OF THE CALIFORNIA BUILDING CODE.
- PIPE SECTIONS SHALL CONFORM TO ASTM A53, Fy = 35 ksi, GRADE B OR A501 UNLESS NOTED OTHERWISE.
- STRUCTURAL TUBING (HSS SHAPES) SHALL CONFORM TO ASTM A500, GRADE B (OR HIGHER), Fy = 46 KSI. IF MATERIAL AVAILABILITY IS LIMITED, MEMBER THICKNESSES CAN BE INCREASED BEYOND WHAT IS SHOWN IN THESE DRAWINGS (MAXIMUM INCREASE OF 1/8").
- ALL CHANNELS, ANGLES, AND MISC. STEEL SHALL CONFORM TO ASTM A36, Fy = 36 KSI.

THE POLIGON ENGINEERING DEPARTMENT IS AVAILABLE TO HELP YOU COMPLETE THESE STEPS (616-399-1963).

- FRAME WIDTHS AND LENGTHS ASSUME 2' OVERHANGS (UNO BY ARCHITECT - 2' MAX DIMENSION)

- SS VALUE DEPENDS ON THE PROJECT'S GEOGRAPHICAL LOCATION (VALUES RANGE FROM 0.00 TO 3.73)

- THE SS REGION DICTATES THE MAXIMUM DEAD LOAD PERMITTED ON THE FRAME (SEE TABLE TO THE RIGHT)

- E.G. A PROJECT IN THE WHITE SS REGION WITH A 4 PSF ROOF DEAD LOAD IS LOAD SCENARIO 2

- LOAD SCENARIOS HAVE NO IMPACT ON FRAME DESIGN OR COST (BUT DO AFFECT FOUNDATION SIZE)

- THE COLLATERAL LOAD REPRESENTS ADDITIONAL LOAD THAT CAN BE SUPPORTED BY THE FRAME

- THIS PC IS NOT APPROVED FOR SS VALUES GREATER THAN 3.00 (CONTACT POLIGON FOR ADDITIONAL OPTIONS)

- THE 20' AND 30' WIDTHS ARE SUGGESTED BECAUSE THEY ARE THE MOST ECONOMICAL

- "SS" REPRESENTS MCFLROYMETAL "MEDALLION-LOK" 16" STANDING SEAM ROOF DECK

- MAXIMUM WIDTH IS 30'; (SEE 'ARCHITECTURAL VIEWS' SHEET FOR REFERENCE)

STEP 1: SELECT FRAME DIMENSIONS FOR YOUR PROJECT

STEP 2: SELECT ROOF DECK FOR YOUR PROJECT

- STRUCTURES UP TO 20' WIDE USE THE "RAM 20" BASE FRAME

- STRUCTURES UP TO 30' WIDE USE THE "RAM 30" BASE FRAME

- "MR" REPRESENTS MCELROY METAL "MULTI-RIB" ROOF DECK

- Ss VALUE DETERMINES THE REQUIRED SEISMIC DESIGN FORCES

- THE REGIONS ARE DEPENDANT ON THE SS VALUE DETERMINED IN STEP 3

- REFERENCE DSA BU 14-01 FOR A MAP OF VARIOUS SS REGIONS

STEP 6: IDENTIFY THE FOUNDATION REQUIREMENTS FOR YOUR PROJECT

- SELECT EITHER SPREAD PAD OR DRILLED PIER FOUNDATION

- REFERENCE THE SS REGION (STEP 4) AND THE TOTAL ROOF DEAD (STEP 5)

STEP 3: IDENTIFY THE SS ACCELERATION (g) FOR YOUR PROJECT

STEP 4: IDENTIFY THE SS REGION FOR YOUR PROJECT

- IDENTIFY A SINGLE LOAD SCENARIO

STEP 5: IDENTIFY THE ROOF DEAD LOAD FOR YOUR PROJECT - THE ROOF DECK DEAD LOAD WILL ALWAYS BE INCLUDED

- ALL COLD FORM STEEL SHALL CONFORM TO ASTM A653, CS = TYPE B, Fy = 50 KSI. STRUCTURAL STEEL AND DECK SHALL BE IDENTIFIED FOR CONFORMITY PER CBC 2203A.1.
- ROOF DECK SHALL HAVE KYNAR 5000 METAL COATING.
- ROOF DECK SHALL CONFORM TO ATSM A792, Fy = 50 KSI. MR ROOF SCREWS MEET ASTM A510 WITH A HEAD DIMENSION OF 0.31" (FLAT-TO-FLAT) AND INTEGRAL WASHER

INSTRUCTIONS FOR ARCHITECTS SUBMITTING THESE PRE-CHECKED DRAWINGS TO DSA:

BEFORE SUBMITTING THESE PRE-CHECKED DRAWINGS FOR YOUR PROJECT, FOLLOW THE STEPS BELOW TO PROPERLY DEFINE THE APPROVED OPTIONS:

- THE 44', 64', AND 84', LENGTHS ARE SUGGESTED BECAUSE THEY ARE THE MOST COMMON (20' BAYS ARE MOST ECONOMICAL)

- FIND SS VALUES FOR YOUR PROJECT ON THE USGS WEBSITE (SEARCH INTERNET FOR "USGS SEISMIC DESIGN MAPS")

- BE SURE THE TOTAL ROOF DEAD LOAD FOR YOUR PROJECT IS LESS THAN OR EQUAL TO THE MAX DEAD LOAD SHOWN IN STEP 4

- FOUNDATION TYPE IMPACTS STEEL FABRICATION (COLUMN LENGTH) AND CONSTRUCTION (TIMING, SEQUENCE, COST, ETC.)

- POLIGON CAN REVIEW THE SITE-SPECIFIC SOILS REPORT TO EVALUATE THE POSSIBILITY OF SMALLER FOUNDATIONS

DIMENSION OF 0.58" (OUTSIDE DIAMETER). SS ROOF SCREWS MEET ASTM A510 WITH A HEAD DIMENSION OF 0.437" (OUTSIDE DIAMETER).

#### WELDING:

- ALL WELDING SHALL COMPLY WITH AWS D1.1 SPECIFICATIONS AND SHALL BE DONE BY AWS QUALIFIED WELDERS CERTIFIED FOR THE TYPE OF WELDING TO BE PERFORMED AS REQUIRED BY DSA.
- ALL WELDING SHALL BE DONE BY GAS METAL ARC PROCESS WITH E70XX ELECTRODES. FLUX CORE ARC WELD SHALL CONFORM TO CHARPY NOTCH TOUGHNESS RATING OF 20 ft-lb @ (0° F).
- ALL WELDING SHALL BE DONE IN THE SHOP WITH REQUIRED INSPECTION, PRE-APPROVED BY DSA, TO INSURE PROPER MATERIAL ID AND WELDING.
  - WELD FILLER METAL MANUFACTURER SHALL PROVIDE WRITTEN CERTIFICATION OF COMPLIANCE WITH CODE AND SPECIFICATIONS.

#### BOLTING:

- ALL BOLTS SHOWN ON THESE DRAWINGS ARE ASTM A325 HIGH STRENGTH BOLTS (UNO), TYPE 3.
- HIGH STRENGTH BOLTS SHALL BE SAMPLED AND TESTED IN COMPLANCE WITH CBC 2213A.1. BEFORE ERECTING THE FRAME, VERIFY ALL BOLTS AND NUTS ARE CLEAN OF DEBRIS AND BURRS - INCLUDING THE HARDWARE ALREADY FASTENED INSIDE THE MEMBERS. CHASING SOME OF THE BOLTS AND NUTS MAY BE REQUIRED.
- ANCHOR BOLTS (HEAVY HEX HEAD, ASTM F1554, GRADE 55) SHALL BE HOT DIPPED GALVANIZED PER ASTM F2329. ANCHOR BOLTS MAY BE HEADED OR THREADED WITH A NUT THAT IS PREVENTED FROM ROTATING.
- HIGH STRENGTH NUTS SHALL CONFORM TO ASTM A563.
- HIGH STRENGTH WASHERS SHALL CONFORM TO ASTM F436. THE BOLTING INSTALLATION REQUIREMENTS OUTLINED BELOW ARE CRITICAL TO THE STRUCTURE'S DESIGN AND PERFORMANCE. THE INSTALLER IS REQUIRED TO COORDINATE THIS PHASE OF CONSTRUCTION WITH THE SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD PRIOR TO THE ERECTION OF THE FRAME. ALL BOLTS SHALL BE INSTALLED AND INSPECTED PER THE APPLICABLE VERSION OF AISC'S "SPECIFICATION FOR STRUCTURAL JOINTS
- PRETENSIONED JOINTS (IDENTIFIED ON THE FRAME CONNECTION DETAILS WITH A "PJ REQUIRED") MUST BE INSTALLED AND INSPECTED TO MEET ONE OF FOLLOWING REQUIREMENTS:
  - TURN-OF-NUT PRETENSIONING
  - 2. CALIBRATED WRENCH PRENTENSIONING

USING HIGH-STRENGTH BOLTS", CBC 1705A.2.1; AISC 341-10 J7; AISC 360-10 N5.6.

- 3. DIRECT-TENSION-INDICATOR PRETENSIONING (CONTRACTOR RESPONSIBLE FOR PURCHASE OF REQUIRED WASHERS)
- ALL OTHER JOINTS MUST BE INSTALLED AND INSPECTED TO MEET THE REQUIREMENTS OF SNUG-TIGHTENED JOINTS. NOTE TO INSTALLER AND INSPECTOR(S): THE SNUG-TIGHT CONDITION EXISTS, IN PART, WHEN ALL THE BOLTS IN THE JOINT HAVE BEEN TIGHTENED SUFFICIENTLY TO PREVENT THE REMOVAL OF THE NUTS WITHOUT
- THE CONTRACTOR, SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD MUST ALL AGREE ON WHICH APPROACH WILL BE USED TO PRETENSION THE BOLTS. THE CONTRACTOR IS RESPONSIBLE FOR DOCUMENTING THE APPROACH AGREED TO BY ALL PARTIES LISTED ABOVE.

#### <u>FOUNDATIONS:</u>

- ALLOWABLE SOIL PRESSURES ASSUME CLASS 4 SOIL CLASSIFICATION PER CBC TABLE 1806A.
- A GEOTECHNICAL REPORT / LETTER IS REQUIRED AT THE OVER-THE-COUNTER APPOINTMENT FOR EACH PROJECT. FILL AND BACKFILL SHALL BE COMPACTED TO 95% OF MAX. DENSITY IN ACCORDANCE WITH ASTM TEST METHOD D1557-70. FLOODING NOT PERMITTED.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING, ETC. NECCESSARY TO SUPPORT OUT AND/OR FILL BANKS DURING EXCAVATION, AND FORMING AND PLACEMENT OF CONCRETE.

#### **CONCRETE:**

FRAME WIDTH

ROOF DECK TYPE

<sup>2</sup> MAY NOT EXCEED REQUIREMENTS LISTED IN STEP 6 BELOW

GREEN

MIX DESIGN REQUIREMENTS: (NORMAL WEIGHT CONCRETE)

	STRENGTH f'c (28 DAYS)	W/C RATIO (NON-AIR ENTRAINED)	W/C RATIO (AIR ENTRAINED)	SLUMP (± 1")	UNIT WEIGHT (NORMAL WEIGHT)	
	5000 PSI	0.63	0.55	3"	150 PCF	
2.	CHANGES TO TH	IE MIX DESIGN MUST BE A	PPROVED BY THE ENG	GINEER OR AR	CHITECT OF RECORD AN	ND DSA
3.	AGGREGATES SI MAX AGGREGA	HALL CONFORM TO ASTM ATE SIZE = 1".	C33 WITH PROVEN	SHRINKAGE CH	HARACTERISTICS OF LESS	00. NAHT 8

- CEMENT SHALL CONFORM TO ASTM C150 (TYPE V) UNLESS NOTED OTHERWISE ON THE DRAWINGS.
- CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF FIVE DAYS AFTER PLACEMENT. ALTERNATE METHODS WILL BE APPROVED IF SATISFACTORY PERFORMANCE CAN BE ASSURED.

OTHER

≥ 40'-0" (NOMAX)

(30° MAX)

MAX DEAD LOAD

5 PSF

3.5 PSF

- CONCRETE SHALL NOT FREE FALL MORE THAN FIVE FEET.
- CONCRETE SHALL BE PROPORTIONED PER ACI 318-11 5.2.

FRAME DIMENSION:

**ROOF DECK** 

Ss ACCELERATION (g)

□ 84'

Ss REGIONS

0.000 < Ss <= 1.875

1.875 < Ss <= 2.500

2.500 < Ss <= 2.750

★ LOAD SCENARIO 4

I LOAD SCENARIO 4

□ 30°

□ 64°

- CONCRETE SHALL BE TESTED PER CBC 1905A.1.2, 1913A.1, 1705A.3, AND ACI 318-11 5.6.
  - STEP 7: SELECT MISCELLANEOUS OPTIONS FOR YOUR PROJECT MAXIMUM CLEAR HEIGHT IS 10'-0"; (SEE 'ARCHITECTURAL VIEWS' SHEET FOR REFERENCE) MARK UP PC DRAWINGS WITH SIZE AND LOCATION OF CUTOUTS BEFORE SUBMITTING TO DSA

REINFORCING STEEL:

GR 40: (#3 BARS)

GR 60: (#4 BARS AND LARGER)

CAST AGAINST EARTH.

(DEFORMATIONS SHALL BE IN ACCORDANCE WITH ASTM A305) AS FOLLOWS:

STANDARD PRACTICE FOR DETAILING REINFORCING CONCRETE STRUCTURES."

MIN. COVER FOR CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:

. WALLS EXPOSED TO WEATHER (#6-#18 BARS)......

G. NOT EXPOSED TO WEATHER (#11 & SMALLER)....... 3/4"

REINFORCING STEEL SHALL BE SAMPLED AND TESTED PER CBC 1913A.2.

PARTS PRETREATED IN A 3 STAGE IRON PHOSPHATE WASHER (OR EQUAL).

SAMPLE PRODUCTION PARTS TESTED TO MEET THE FOLLOWING CRITERIA:

A. SALT SPRAY RESISTANCE PER ASTM B 117/ ASTM D 1654

B. HUMIDITY RESISTANCE PER ASTM D2247-02

<u>ABBREVIATIONS</u>

GA

C. COLOR/UV RESISTANCE PER ASTM G154-04

AMERICAN CONCRETE INSTITUTE

AMERICAN INSTITUTE OF STEEL CONSTRUCTION

ASSEMBLY (INTERNAL REFERENCE)

AMERICAN WELDING SOCIETY

CALIFORNIA BUILDING CODE

COMPLETE JOINT PENETRATION

DEGREE

DIMENSION

DIVISION OF THE STATE ARCHITECT

EQUAL

FEET

GAGE

INCHES

KIPS PER SQUARE INCH

LEFT HAND

MAXIMUM

MINIMUM

MISCELLANEOUS

MILES PER HOUR

DIAMETER

ASTM AMERICAN SOCIETY FOR TESTING AND MAT'LS

**POWDER COATED AND EPOXY PRIMED FINISH:** 

CAST AGAINST FORM BELOW GRADE.....

FORMED SLABS (#11 BAR & SMALLER). ). SLABS ON GRADE (FROM TOP OF SLÁB)..

COLUMNS AND BEAMS (MAIN BARS)..

WELDING OF REINFORCING IS NOT ALLOWED

DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS SHALL CONFORM THE ACI "MANUAL OF

(#5 & SMALLER)...

ENTIRE POWDER COATING PROCESS COMPLETED IN SAME FACILITY AS STEEL FABRICATION.

EPOXY PRIMER POWDER COAT APPLIED TO PARTS FOR SUPERIOR CORROSION PROTECTION.

5000 HOURS WITH NO LOSS OF ADHESION OR BLISTERING

STANDARD PRACTICE" AND THE "AISC SPECIFICATION SECTION M3" (UNLESS NOTED OTHERWISE).

10000 HOURS WITH NO CREEP FROM SCRIBE LINE AND RATING OF 10

2000 HOURS EXPOSURE ALTERNATE CYCLES WITH NO CHALKING, 75% COLOR RETENTION, AND

MULTI-RIB ROOF PANEL (MCELROY)

NOT TO SCALE

NUMBER

ON CENTER

OCCUPATIONAL HEALTH AND SAFETY ADM.

POUNDS PER CUBIC FOOT

POLIGON DRAWING

PRETENSIONED JOINT

PLACES

PLATE

POUNDS PER SQUARE FOOT

POUNDS PER SQUARE INCH

QUANTITY

REFERENCE

RIGHT HAND

STANDING SEAM ROOF PANEL (MCELROY)

TYPICAL

**UNLESS NOTED OTHERWISE** 

U.S. GEOLOGICAL SURVEY

WITH

COLOR VARIATION MAXIMUM 3.0 E VARIATION CIE FORMULA (BEFORE AND AFTER 2000 HOURS

PD

**PLCS** 

PLT

PSF

PSI

QTY

REF

RH

SQ

SS

STEP &: SELECT APPLICABLE SHEET INDEX FOR YOUR PROJECT REFERENCE THE BASE FRAME (STEP 1) AND THE ROOF DECK TYPE (STEP 2) IDENTIFY THE APPLICABLE SHEET INDEX

 ALL THE CLASS OF THE PARTY OF T
- EXCLUDE 'MISC DESIGN OPTIONS' SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUTS OR GUTTER
 STEP 10: IDENTIFY PROJECT NAME AND SCHOOL DISTRICT
 PROJECT NAME:
 Dunsmore Elementary School

		8 IETTOM		2./30 < 35 <= 3.000	Z 1.2L		
7	REFERENCE DSA BU 14-01 FOR A MAP OF VA	RIOUS SEREGIO	ONS				
	TOTAL ROOF DEAD LOAD						
<b>1</b>		DEAD	LOAD	EXAMPLES			
STEP	ROOF DECK	1.8	_ PSF	MR = 1.2 PSF; SS = 1.8 PSF (SEE STEP 2)			
, co	COLLATERAL <sup>1</sup>	_0.0	_ PSF	LIGHTING, FIRE SUPPRESSION, PV PANELS, ETC.			
	TOTAL <sup>2</sup>	_1.8	PSF	ADD ROOF DECK AND COLLATE	RAL LOADS		
7	PROVIDE DSA WITH EVIDENCE THAT THE CO	LATERAL LOAD	FOR YOUR P	ROJECT MEETS THESE REQUIREMENTS			

a BLUE

GREEN

		FOUNDATION	REQUIREMENTS		
	Ss REGION	DEAD LOAD (DL)	LOAD SCENARIO	SPREAD PAD	DRILLED PIER
2	)### HTT	DL <= 2 PSF	D LOAD SCENARIO 1		1
STEP	WHITE	2 PSF < DL <= 5 PSF	n LOAD SCENARIO 2		
	RILLE	DI <= 35 PSE	= IOAD SCENARIO 3		

DL <= 2 PSF

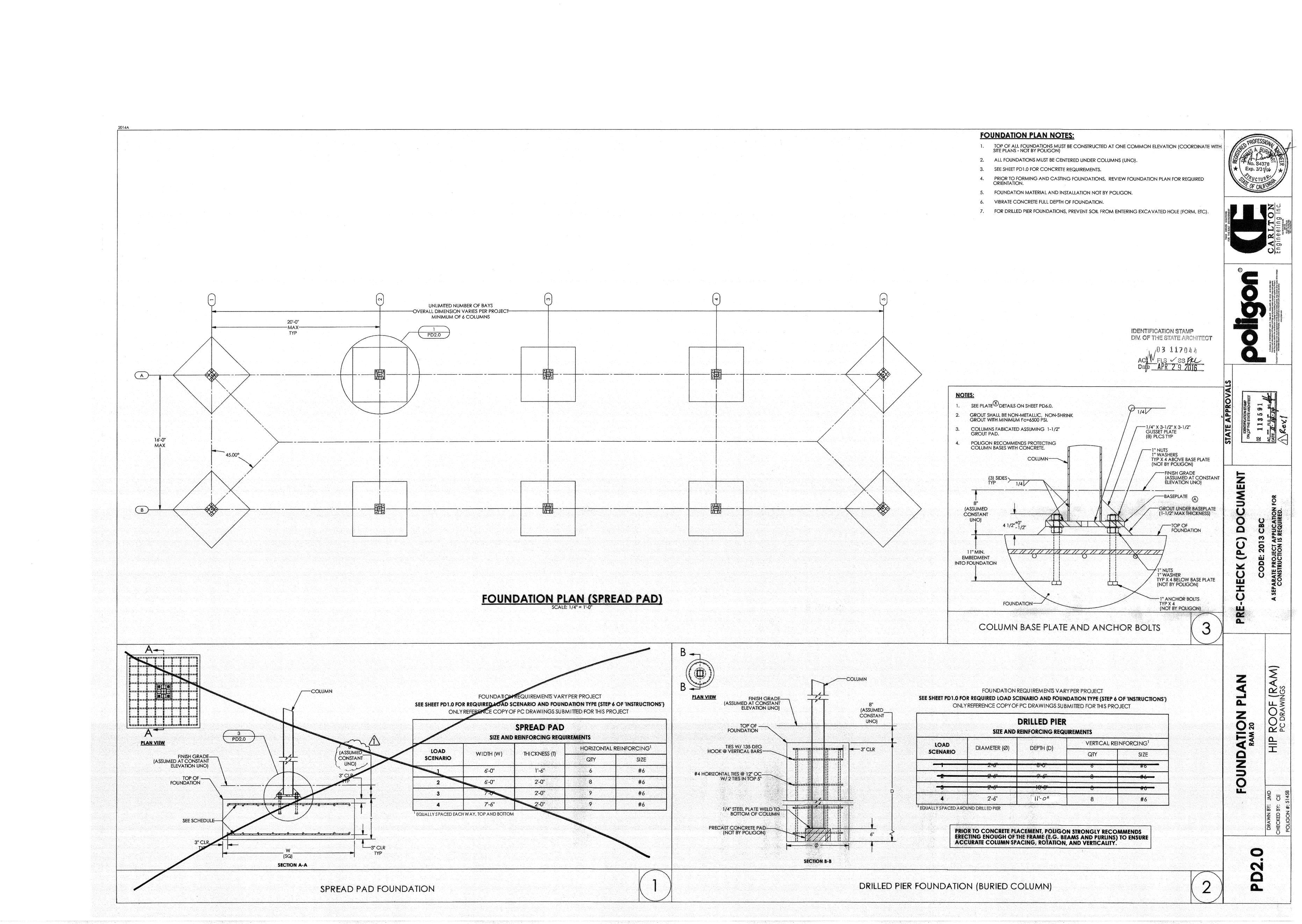
DL <= 2 PSF

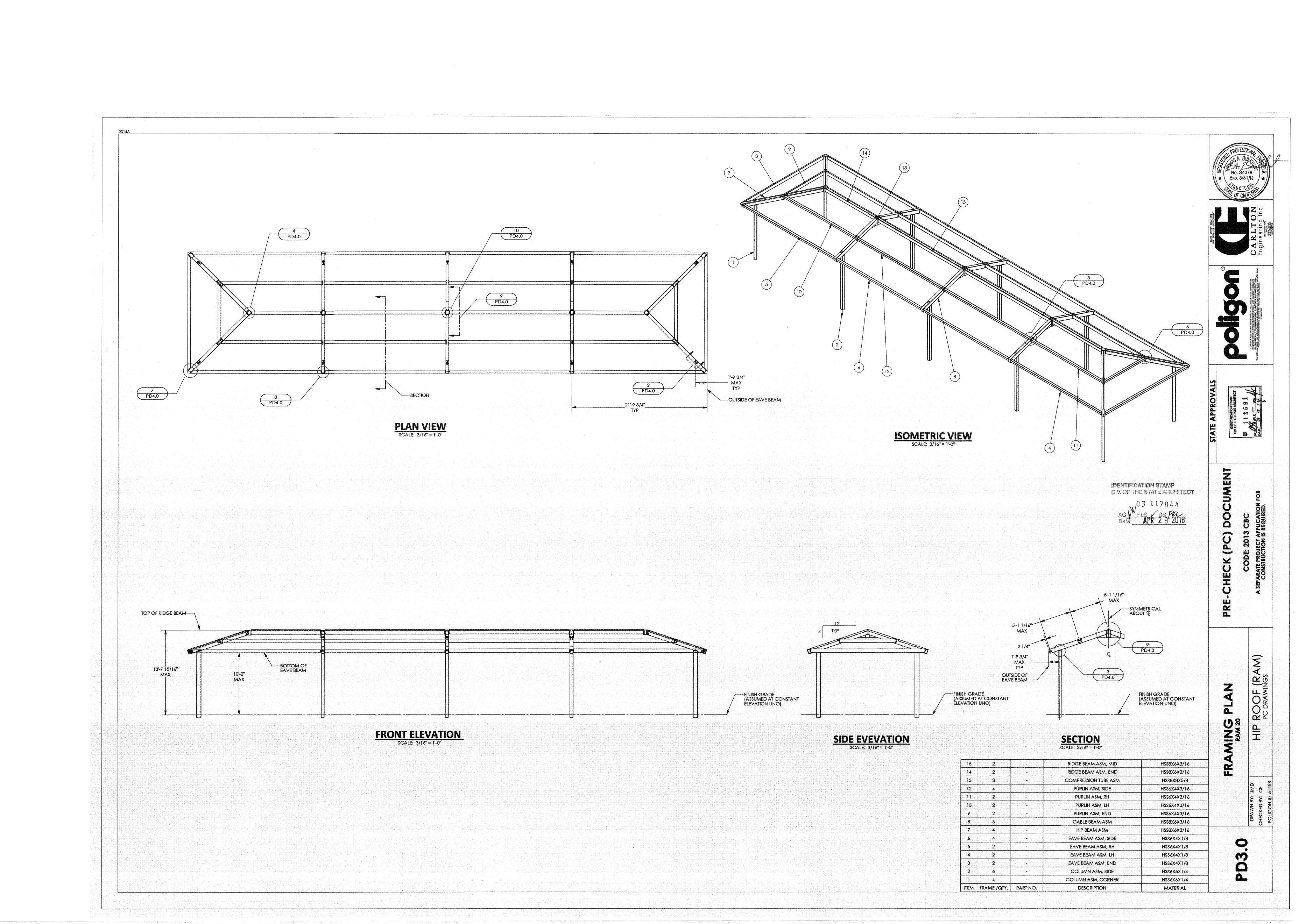
STEP 9: INCLUDE APPLICABLE SHEETS WITH YOUR DSA SUBMITTAL

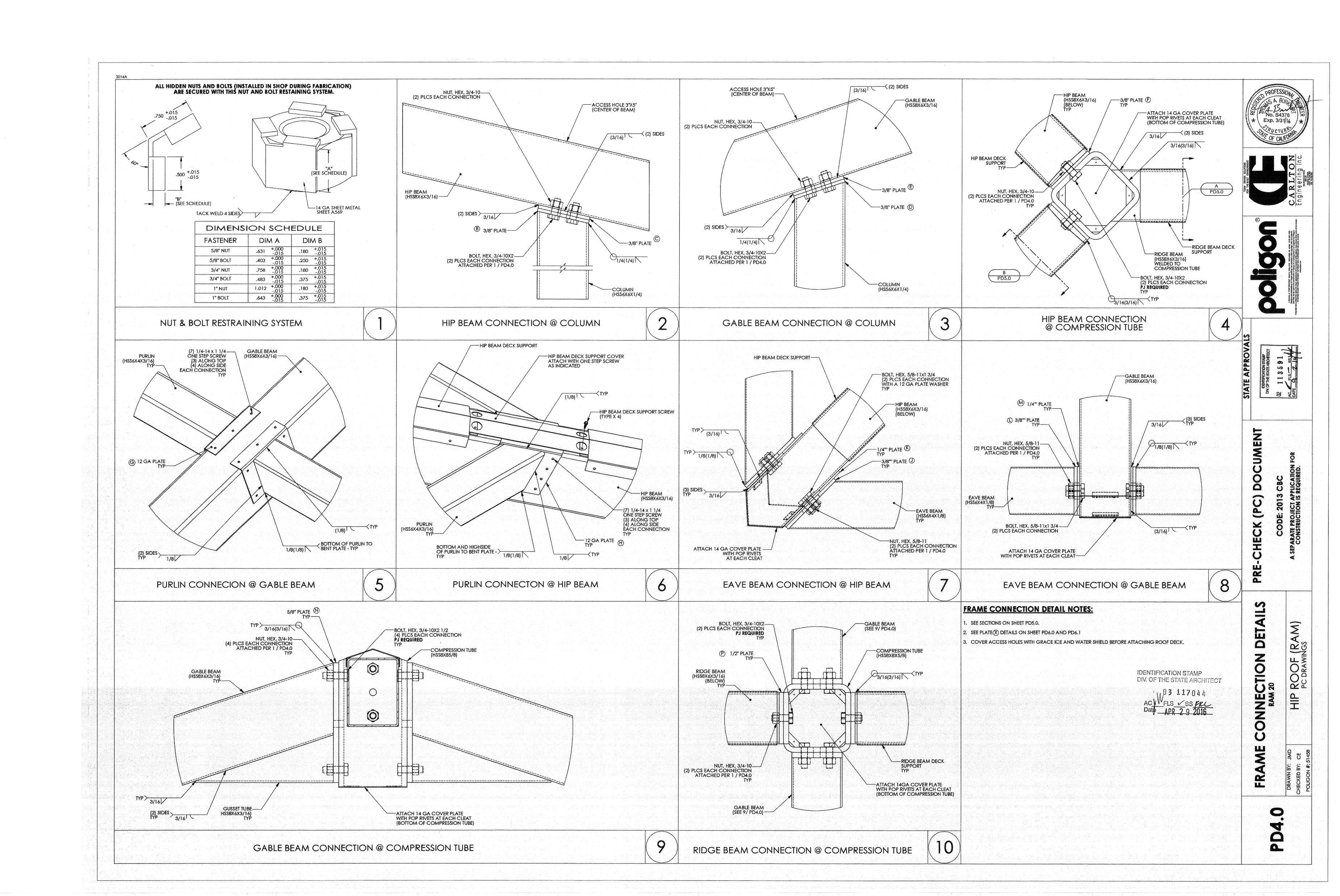
	- EXCLUDE 'MISC DESIGN OPTIONS' SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUTS OR GUT
STE	10: IDENTIFY PROJECT NAME AND SCHOOL DISTRICT
	PROJECT NAME:
	Dunsmore Elementary School
	SCHOOL DISTRICT:
	Glendale Unified School District

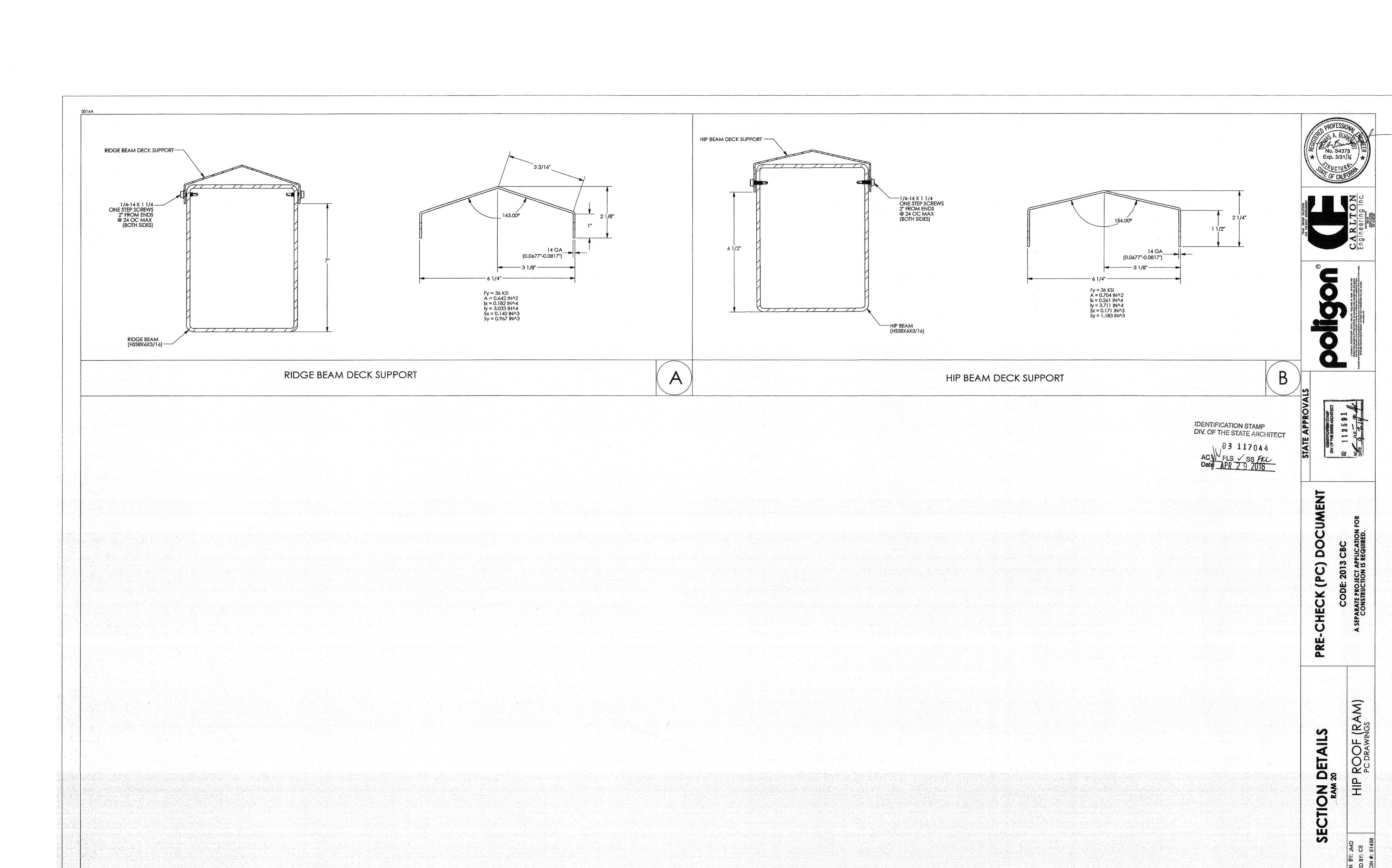
MISCELLANEOUS **DESIGN OPTIONS >** 10'-0" (10, WYX **ELECTRICAL CUTOUTS** □ YES ≫ NO 💌 YES a NO

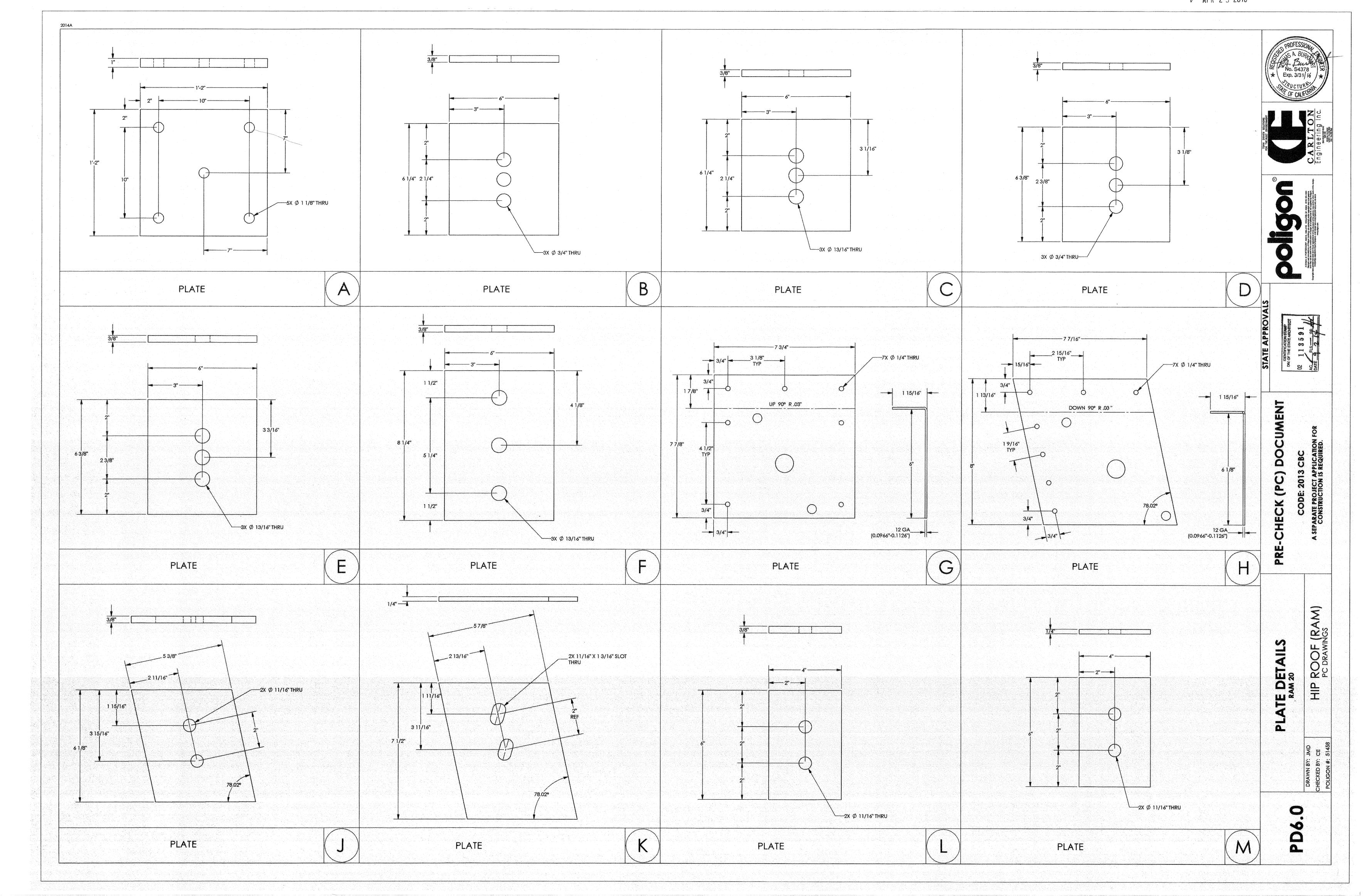
***************************************		SHEET INDEX						
s		BASE FRAME	T	RAA	A 20	П	RAA	A 30
		ROOF DECK		MR	SS	li	MR	SS
		SELECT ONE	ſ	a	×			
	ω,	GENERAL NOTES	T	PD1.0	PD1.0		PD1.0	PD1.0
	STEP	SPECIAL INSPECTIONS	Γ	PD1.1	PD1.1		PD1.1	PD1.1
	vs	FOUNDATION PLAN	Γ	PD2.0	PD2.0		PD2.1	PD2.1
		FRAMING PLAN	Γ	PD3,0	PD3.0		PD3.1	PD3.1
ĺ		FRAME CONNECTION DETAILS		PD4.0	PD4.0		PD4.1	PD4.1
		SECTION DETAILS	ړ[	PD5.0	PD5.0		PD5.1	PD5.1
		PLATE DETAILS	ſ[	PD6.0	- PD6.1°	3	PD6.2	-PD6.3
		ARCHITECTURAL VIEWS	Y	PD7.0	PD7:0	۲4	PD7.1	PD7.1
١		ROOF CONNECTION DETAILS		PD8.0	PD8:1		PD8.0	PD8.1
		MISC DESIGN OPTIONS	Γ	PD9.0	PD9.0		PD9.0	PD9.0

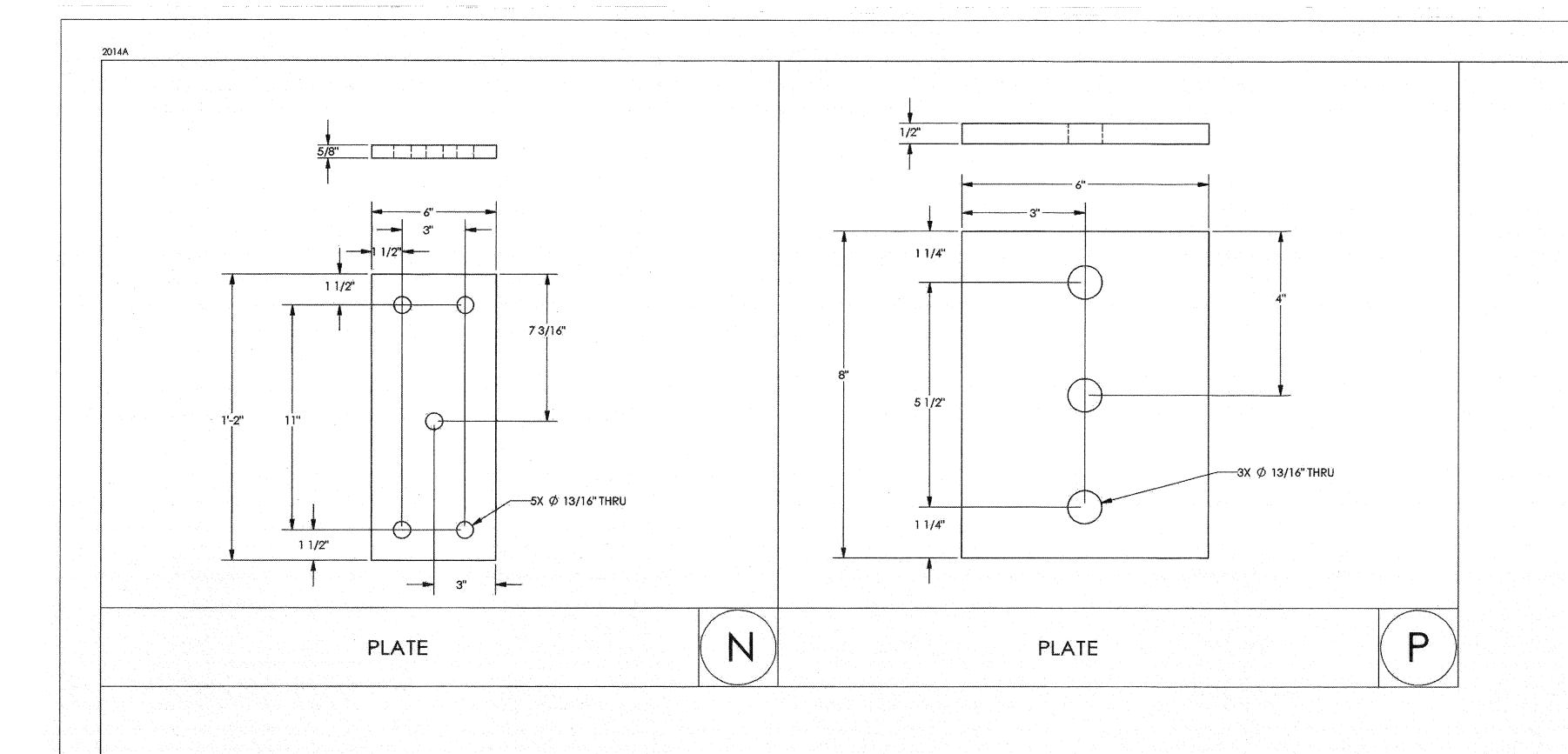


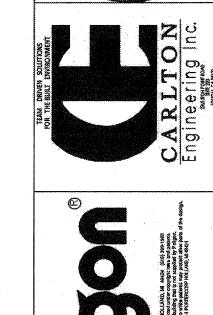














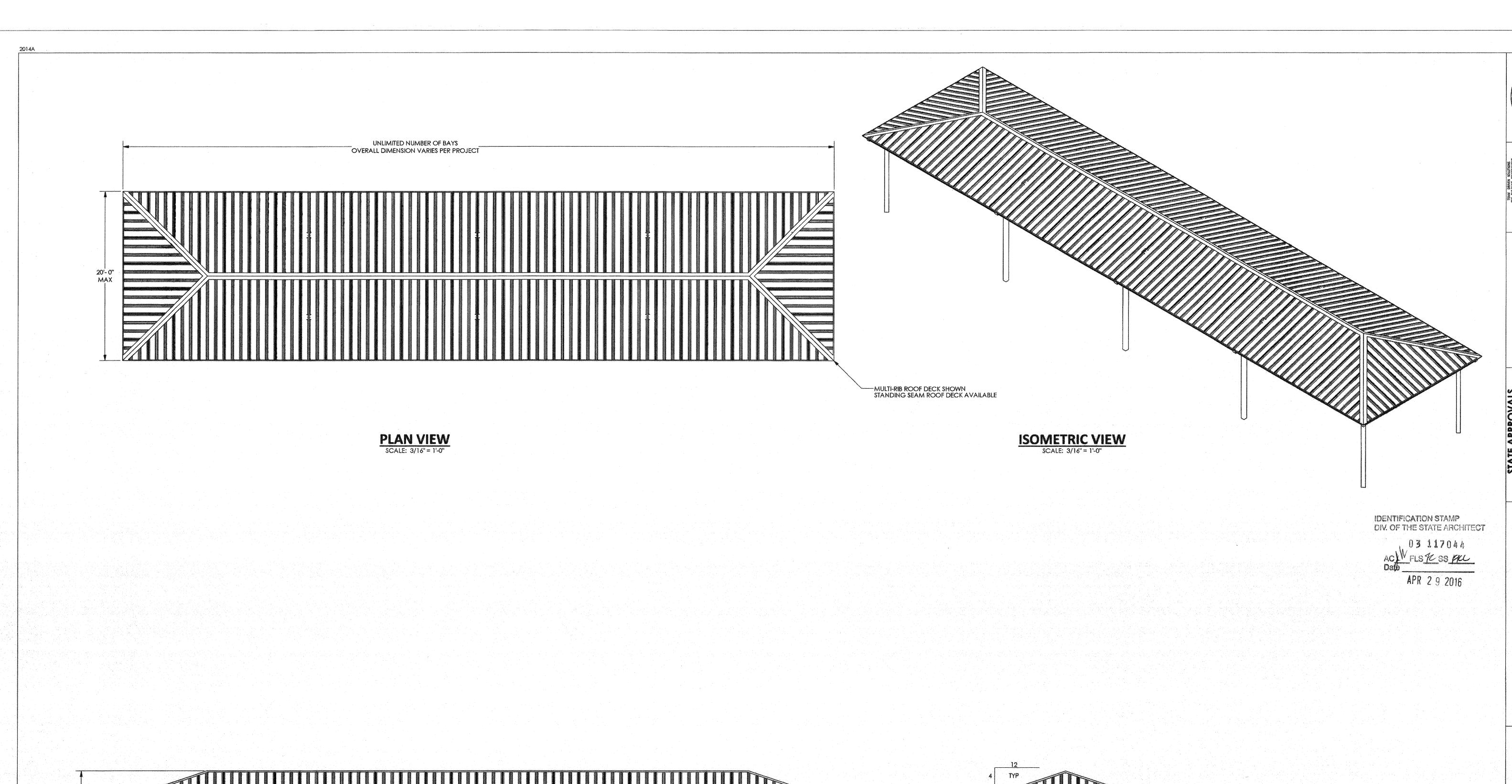
IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT

PRE-CHECK (PC) DOCUMENT

HIP ROOF (RAM)
PC DRAWINGS

ATE DETAILS

Ω



13'-10<sup>'</sup>3/16" MAX

FRONT ELEVATION
SCALE: 3/16" = 1'-0"

SCALE: 3/16" = 1'-0"

—FINISH GRADE (ASSUMED AT CONSTANT ELEVATION UNO)

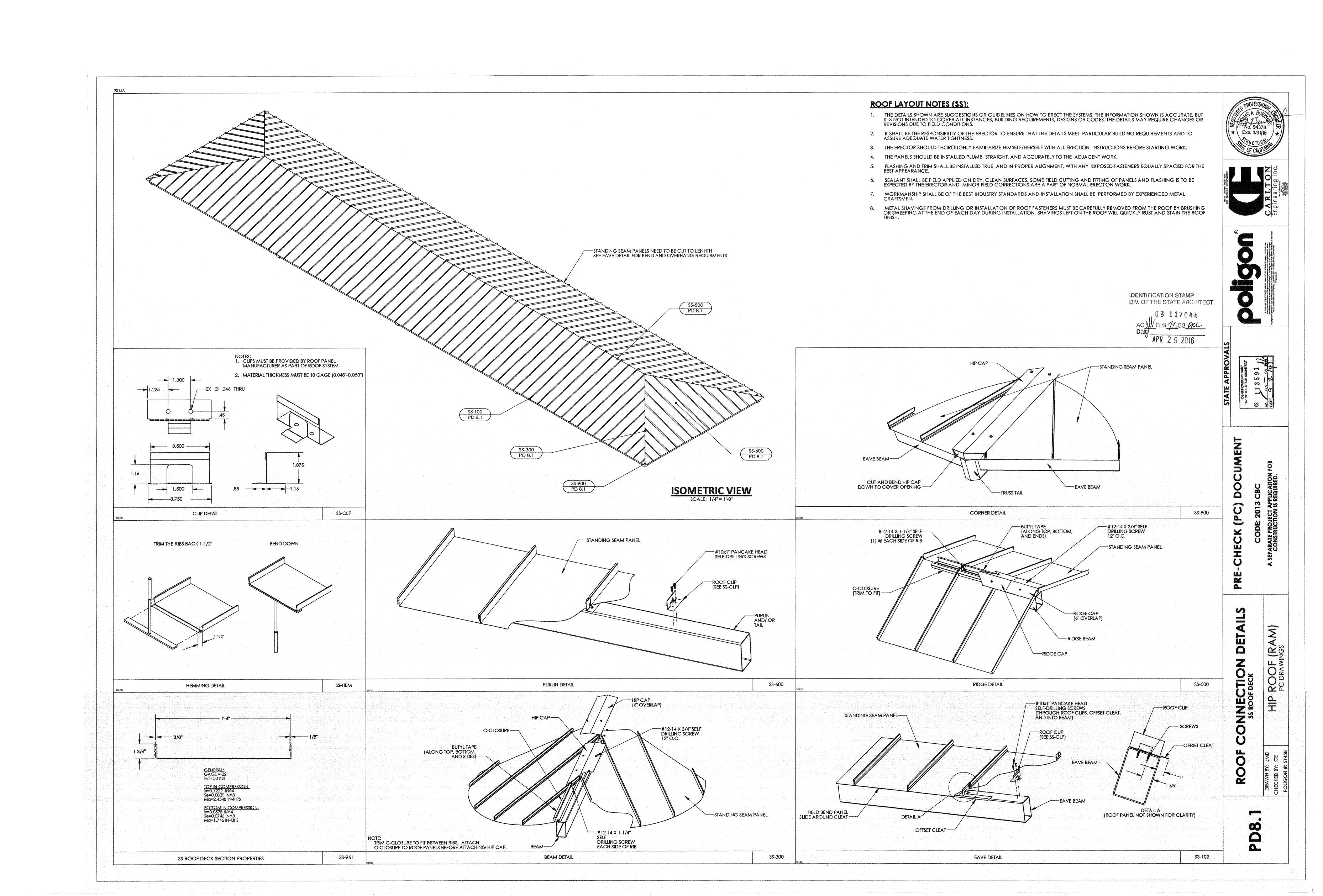
— FINISH GRADE (ASSUMED AT CONSTANT ELEVATION UNO)

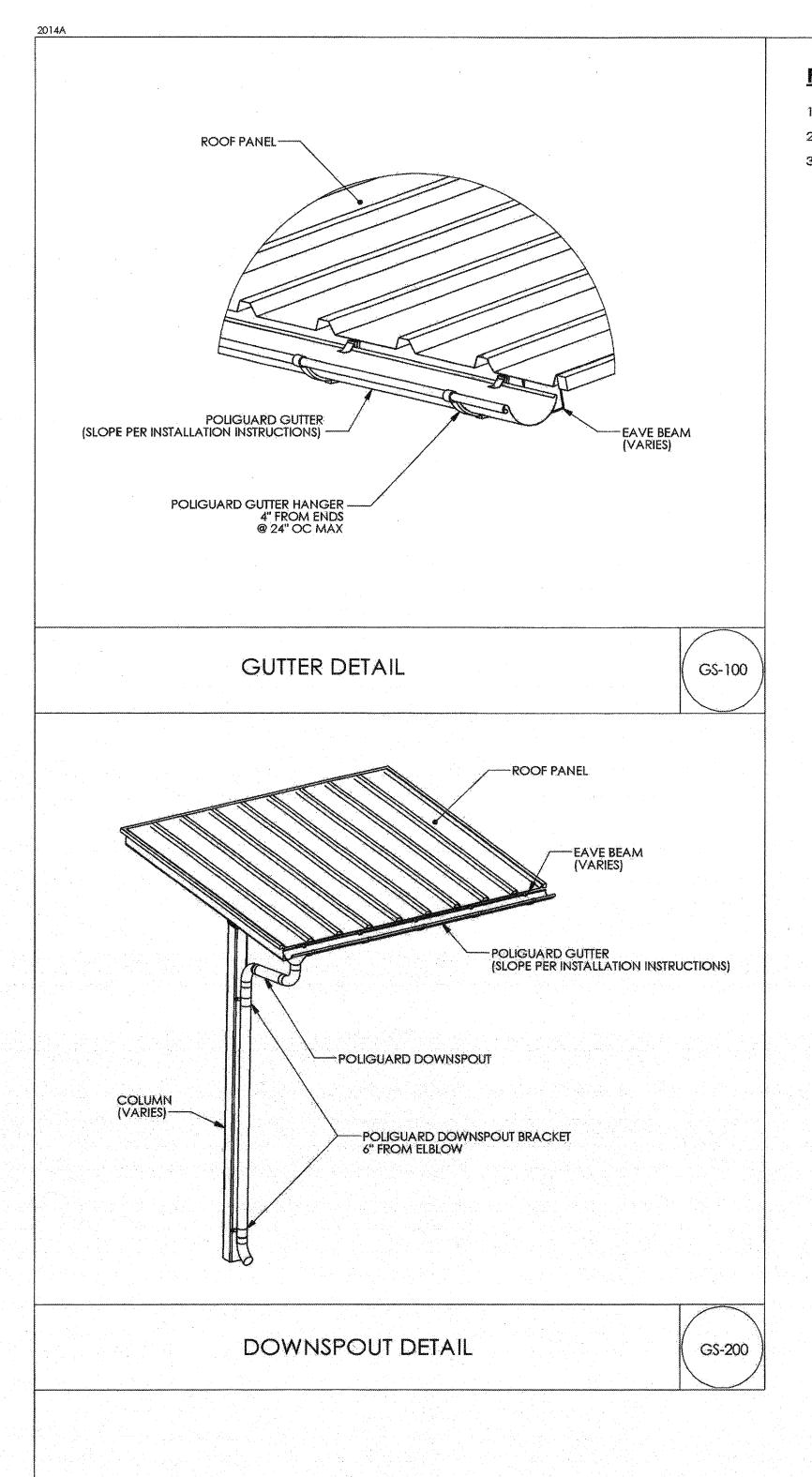
ARCHITECTURAL VIEWS

Ö

HIP ROOF (RAM)
PC DRAWINGS

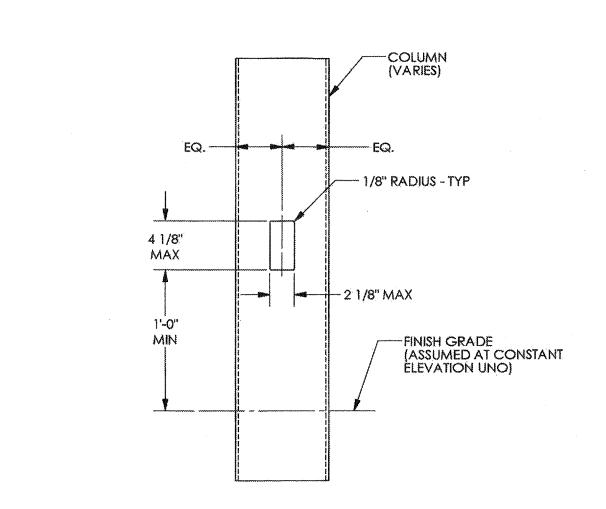
PD7





### POLIGUARD GUTTER SYSTEM NOTES:

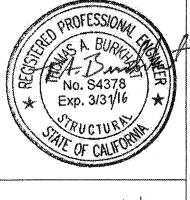
- 1. PREFABRICATED GUTTER SYSTEM IS ATTACHED TO THE STRUCTURE AFTER ROOF IS INSTALLED.
- 2. DETAILED INSTALLATION INSTRUCTIONS ARE SHIPPED WITH THE STRUCTURE.
- 3. DOWNSPOUTS REQUIRED AT EACH COLUMN.



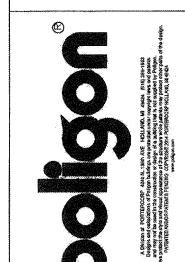
1. MAXIMUM ONE CUTOUT PERMITTED IN EACH MEMBER.

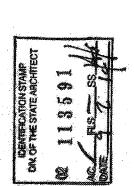
**ELECTRICAL CUTOUT NOTES:** 

- 2. CUTOUTS CAN BE PLACED ON ANY SIDE OF A MEMBER.
- 3. CUTOUTS CAN BE PLACED ALONG MEMBERS AS INDICATED IN THE DETAILS. ARCHITECTS REQUESTING CUTOUTS MUST MARKUP APPROVED PC DRAWINGS TO LOCATE CUTOUTS FOR APPROVAL AND FABRICATION.









IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT

PRE-CHECK (PC) DOCUM

HIP ROOF (RAM)

