

PERIODS, BUT SHALL BE READ AS SAME.

A.B. -----ANCHOR BOLT

ALT. — — — ALTERNATE

BM ----BEAM

BLK----BLOCK

BRG ----BEARING

C -----CAMBER

ARCH'L ---- ARCHITECTURAL

A/C ---- AIR CONDITIONER

**ABBREVIATIONS** 

NOTE: ABBREVIATIONS MAY OR MAY NOT HAVE

A.B.C. -----AGGREGATE BASE COURSE

A.F.F. -----ABOVE FINISHED FLOOR

AISC-----AMERICAN INSTITUTE OF STEEL

INSTITUTE

AITC-----AMERICAN INSTITUTE OF TIMBER

ANSI-----AMERICAN NATIONAL STANDARDS

APA ---- AMERICAN PLYWOOD ASSOCIATION

ASTM ------AMERICAN SOCIETY FOR TESTING

AWS-----AMERICAN WELDING SOCIETY

B.F.F ----BELOW FINISHED FLOOR

B.O.B. ----BOTTOM OF BEAM

B.O.D. ----BOTTOM OF DECK

B.O.F. -----BOTTOM OF FOOTING

CFS -----COLD FORMED STEEL

C.G. -----CENTER OF GRAVITY

C.L.B. ----CENTERLINE OF BEAM

C.L.W. ----CENTERLINE OF WALL

D.F. (D.F.L.) — — DOUGLAS FIR LARCH

E.C. -- -- END TO CENTERLINE E.E. -----END TO END E.O.S. ----EDGE OF SLAB

EXP. BOLT (E.B.) — EXPANSION BOLT EXP. JT (E.J.) — EXPANSION JOINT

F.O.M. ----FACE OF MEMBER F.O.S. ----FACE OF STEEL

F.O.W. -----FACE OF WALL

GALV -----GALVANIZED

H.F. ----HEM FIR

JST ----- JOIST

LBS (#) ---- POUNDS

LGR "-----LEDGER

LL -----LIVE LOAD

MAS ---- MASONRY

MAX ----- MAXIMUM

MIN ----- MINIMUM

MECH'L---- MECHANICAL

MFR'D ---- MANUFACTURED MFR('S) ---- MANUFACTURER('S)

N/A -----NOT APPLICABLE N.T.S. ---- NOT TO SCALE

O.F.W.----OUTSIDE FACE OF WALL

P.C. — — — PRECAST CONCRETE

± ----PLUS OR MINUS

PREFAB ---- PREFABRICATED

REINF ---- REINFORCING

SIM -----SIMILAR

STL - - - - STEEL

STD----STANDARD

TL ---- TOTAL LOAD

T.O.B. ---- TOP OF BEAM

T.O.D. --- TOP OF DECK

T.O.F. ---- TOP OF FOOTING

T.O.L. - - - TOP OF LEDGER

T.O.P. - - - TOP OF PLATE

T.O.S. --- TOP OF STEEL

T.O.W.---- TOP OF WALL

T.O.M. --- TOP OF MASONRY

T -----POST-TENSIONED

SDI -----STEEL DECK INSTITUTE

SLV-----SHORT LEG VERTICAL

SJI -----STEEL JOIST INSTITUTE

SLH-----SHORT LEG HORIZONTAL

OSHA ---- OCCUPATIONAL SAFETY AND

PCF ---- POUNDS PER CUBIC FOOT

PLF ---- POUNDS PER LINEAR FOOT

PSF-----POUNDS PER SQUARE FOOT PSI -----POUNDS PER SQUARE INCH

PTI -----POST-TENSIONING INSTITUTE

SSMA -----STEEL STUD MANUFACTURERS

T.O.C.T. ---- TOP OF CONCRETE TOPPING

T.O.P.C. ---- TOP OF PRECAST CONCRETE

TPI ---- TRUSS PLATE INSTITUTE

UBC ---- UNIFORM BUILDING CODE

VERT ----- VERTICAL REINFORCING

W.W.F.---- WELDED WIRE FABRIC

W/O ---- WITHOUT

U.N.O ---- UNLESS NOTED OTHERWISE

WWPA---- WESTERN WOOD PRODUCTS

W/C ---- WATER TO CEMENT RATIO

WCLA ---- WEST COAST LUMBER ASSOCIATION

WCLIB----- WEST COAST LUMBER INSPECTION

BUREAU

ASSOCIATION

TYP ----- TYPICAL
T&G ---- TONGUE AND GROOVE

ASSOCIATION

PCI -----PRECAST/PRESTRESSED CONCRETE

INSTITÚTE

HEALTH ADMINISTRATION

O.C. ----ON CENTER

OPP ---- OPPOSITE

K(KIP) -----1000 POUNDS

E.W.----EACH WAY F.F. - - - - - FINISHED FLOOR

C.L.C. ----CENTERLINE OF COLUMN

C.L.F. -----CENTERLINE OF FOOTING

CONC C.J. ---- CONCRETE CONTROL JOINT

C.M.U. -----CONCRETE MASONRY UNIT

CRSI-----CONCRETE REINFORCING STEEL

DSA -----DIVISION OF STATE ARCHITECT

GA ------GAGE (UNIT OF MEASUREMENT)

G.S.N. -----GENERAL STRUCTURAL NOTES

H.S. — — — — HEADED STUDS
IBC — — — — INTERNATIONAL BUILDING CODE

ICBO ---- INTERNATIONAL CONFERENCE OF

ICC -----INTERNATIONAL CODE COUNCIL

I.O.D. --- INTERPRETATION OF DRAWINGS

LGSEA ---- LIGHT GAGE STEEL ENGINEERS

ASSOCIATION

I.F.W. -----INSIDE FACE OF WALL

KLF -----KIPS PER LINEAR FOOT

L.O.D. ----- LOCATION OF DETAILS

LLH -----LONG LEG HORIZONTAL LLV -----LONG LEG VERTICAL

MAS C.J. --- - MASONRY CONTROL JOINT

MBMA---- METAL BUILDING MANUFACTURERS

MWFRS ---- MAIN WIND FORCE RESISTANCE

SYSTEM

**ASSOCIATION** 

LGS -----LIGHT GAGE STEEL

BUILDING OFFICIALS

GLB (GLULAM) - GLUED-LAMINATED BEAM

HORIZ -----HORIZONTAL REINFORCING

CONC S.J. ---- CONCRETE SAWCUT JOINT

C.I.P. ---- CAST IN PLACE

C.L. - - - - - CENTERLINE

CONN -----CONNECTION

CONT -----CONTINUOUS

DL ------DEAD LOAD

DWG(S) - - - - DRAWING(S)

DIA -----DIAMETER

DN -----DOWN

EQ -----EQUAL EQUIP -----EQUIPMENT

CLR-----CLEAR CONC -----CONCRETE

C.C. -----CENTERLINE TO CENTERLINE

C & C ---- COMPONENTS & CLADDING CBC -----CALIFORNIA BUILDING CODE

A.W.T.S. ---- AUTOMATIC WELDED THREADED

CONSTRUCTION

CONSTRUCTION

AND MATERIALS

2010 EDITION OF THE CALIFORNIA BUILDING CODE. OCCUPANCY GROUP PER SITE-SPECIFIC DOCUMENTS. ALLOWABLE AREA AND MINIMUM SEPARATION BETWEEN STRUCTURES TO BE DETERMINED AT EACH SPECIFIC LOCATION PER CBC WHICH IS TO BE CHECKED AT BACKCHECK.

GENERAL STRUCTURAL NOTES

II-B CONSTRUCTION LOADS:

ROOF DEAD LOAD = ACTUAL WEIGHT OF MEMBER: SOLAR PANEL = 3 PSF (MAX)

PURLIN = 4 PLF

FOR 10 DEGREE ROOF SLOPE: FOR 10 DEGREE ROOF SLOPE:

C&C WND LOAD = 18.9 PSF (TOWARD THE SURFACE).

C&C WND LOAD = 20.8 PSF (AWAY FROM THE SURFACE).

MWFRS WND LOAD = 18.9 PSF / 4.4 PSF (TOWARD THE SURFACE).

MWFRS WND LOAD = 17.8 PSF / 0.0 PSF (AWAY FROM THE SURFACE).

ROOF LIVE LOAD = 10 PSF. DESIGN FOR 300 POUND POINT LOAD LOCATED TO CAUSE MAXIMUM MOMENTS AND SHEAR. USE THE 300 POUND LOAD WITH WIND, BUT NOT WITH 10 PSF ROOF LIVE LOAD. NO STEEL DECK IS TO BE PLACED ON THE STRUCTURE - NOW OR IN THE FUTURE.

OCCUPANCY CATEGORY II

3 SECOND WIND GUST = 85 MPH. WIND IMPORTANCE FACTOR = 1.0.

THIS DESIGN CAN BE USED FOR ANY ROOF SLOPE FROM O DEGREES TO 10 DEGREES.

SEISMIC IMPORTANCE FACTOR = 1.0. SHORT PERIOD SPECTRAL ACCELERATION So = 2.85. ONE SECOND SPECTRAL ACCELERATION S1 = 1.15. REDUNDANCY FACTOR p = 1.3. Sds = 1.005 (MAX.).

Sd1 = 1.16 (MAX.).SEISMIC DESIGN CATEGORY D. BASIC SEISMIC-FORCE RESISTING SYSTEM - CANTILEVERED COLUMN SYSTEMS DETAILED TO CONFORM TO THE REQUIREMENTS FOR ORDINARY STEEL MOMENT FRAMES. RESPONSE MODIFICATION FACTOR (R)= 1.25. ANALYSIS PROCEDURE USED - EQUIVALENT LATERAL FORCE PROCEDURE.

DESIGN BASE SHEAR (3 PANEL) = 2690 LBS. DESIGN BASE SHEAR (4 PANEL) = 3680 LBS.

FOUNDATIONS:

ALL FOOTINGS SHALL BE DESIGNED FOR THE SPECIFIC SITE. DRILLED PIER FOOTING DESIGNS ARE BASED ON THE ALLOWABLE LATERAL BEARING PRESSURES SHOWN IN DETAIL 2. THE ALLOWABLE LATERAL BEARING PRESSURE MAY BE MULTIPLIED BY 2.0 PER CBC SECTION 1806A.3.4. THE DRILLED PIER FOOTINGS ARE DESIGNED AS CONSTRAINED (SECTION 1807A.3.2.2, EQUATION 18A-2) WHERE PLACED IN A CONCRETE PAVEMENT AREA AND AS UNCONSTRAINED (SECTION 1807A.3.2.2, EQUATION 18A-1 OR CZERNIAK, WHICHEVER IS DEEPER) WHERE PLACED IN ASPHALT PAVEMENT AREAS OR DIRT AREAS.

SPREAD FOOTING DESIGNS ARE BASED ON CBC SECTION 1806A, CLASS 5 SOILS. SPREAD FOOTINGS SHALL BEAR ON FIRM, UNDISTURBED SOIL 2 FEET MINIMUM BELOW ADJACENT EXISTING GRADE. DESIGN SOIL BEARING VALUE = 1500 PSF. SOILS ENGINEER MUST VERIFY THAT 1500 PSF SOILS (MINIMUM) ARE PRESENT AT SITE.

FOUNDATIONS ---- 3.000 PSI

CONCRETE:

SPECIFIED 28 DAY COMPRESSIVE STRENGTH F'C:

GENERAL:

ALL CAST-IN-PLACE CONCRETE CONSTRUCTION SHALL CONFORM TO THE LATEST EDITION OF THE ACI. MECHANICALLY VIBRATE ALL CONCRETE WHEN PLACED UNLESS NOTED OTHERWISE. ADMIXTURES CONTAINING CHLORIDES SHALL NOT BE USED. NO OTHER ADMIXTURES PERMITTED WITHOUT APPROVAL. FOR CONCRETE WITHOUT PLASTICIZER, MAXIMUM SLUMP 4 1/2" AT POINT OF PLACEMENT U.N.O. IF PLASTICIZER IS USED, A HIGHER FINAL SLUMP MAY BE ALLOWED UPON STRUCTURAL ENGINEER'S APPROVAL.

FOR REINFORCING INFORMATION, SEE REINFORCING SECTION OF G.S.N., PLANS, SCHEDULES AND

FLY ASH - SHALL BE LIMITED TO 50% OF TOTAL CEMENTITIOUS MATERIALS BY WEIGHT.

TEST DATA FOR EACH CONCRETE MIX SHALL BE SUBMITTED FOR REVIEW PER CHAPTER 5 OF ACI 318. REFERENCE FIGURE R5.3 FOR SUBMITTAL REQUIREMENTS AND OPTIONS. CONCRETE MIX DESIGNS THAT ARE SUBMITTED WITHOUT THE APPROPRIATE TEST DATA CANNOT BE REVIEWED.

IT IS ACCEPTABLE AND INTENDED TO USE EARTH CUTS FOR THE DRILLED PIER FOOTING AND SPREAD FOOTING. THE FOOTING DESIGNS INDICATED ON THIS SHEET DO NOT APPLY IF THE EARTH CUTS ARE UNSTABLE AND/OR DO NOT STAND ON THEIR OWN.

THE FOOTINGS INDICATED ON THIS SHEET DO NOT APPLY WHERE ORGANIC FILL MATERIALS EXIST. CONCRETE SHALL BE ADEQUATELY VIBRATED AROUND THE EMBEDDED STEEL COLUMNS TO ENSURE THE CONCRETE HAS COMPLETELY SURROUNDED THE STEEL COLUMN AND TO ENSURE THE CONCRETE AT THE INSIDE OF THE STEEL COLUMN HAS RISEN TO THE LEVEL OF THE CONCRETE IN THE REMAINDER OF THE DRILLED PIER OR SPREAD FOOTING. CONCRETE SHALL SLOPE UP SLIGHTLY TOWARDS COLUMNS TO PREVENT WATER FROM PONDING AROUND COLUMNS.

IT IS ACCEPTABLE FOR CONCRETE TO FREE FALL INTO FOOTINGS.

REINFORCING:

LL REINFORCING PER CRSI SPECIFICATIONS AND HANDBOOK. ASTM A615 (Fy = 60 KSI / GRADE 60) DEFORMED BARS FOR ALL BARS. WHERE SHOWN ON DRAWINGS ALL GRADE 60 REINFORCING TO BE WELDED SHALL BE ASTM A706. NO TACK WELDING OF REINFORCING BARS ALLOWED WITHOUT PRIOR REVIEW OF PROCEDURE WITH THE STRUCTURAL ENGINEER. LATEST ACI CODE AND DETAILING MANUAL APPLY. CLEAR CONCRETE COVERAGES AS FOLLOWS:

CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH -----EXPOSED TO EARTH OR WEATHER

#6 OR LARGER ----5 AND SMALLER ----- 1 1/2" ALL OTHER PER LATEST EDITION OF ACI 318

ALL REINFORCING SHALL BE CHAIRED TO ENSURE PROPER CLEARANCES. SUPPORT OF FOUNDATION REINFORCING MUST PROVIDE ISOLATION FROM MOISTURE/CORROSION BY USE OF A PLASTIC OR CONCRETE CHAIR. DUCT-TAPE COVERED REINFORCING IS NOT AN ACCEPTABLE CHAIR.

ALL DIMENSIONS REFERENCED IN DRAWINGS AS "CLEAR" SHALL BE FROM FACE OF STRUCTURE TO EDGE OF REINFORCING, AND SHALL NOT BE LESS THAN STATED, NOR GREATER THAN "CLEAR" DIMENSION PLUS 3/8". ALL OTHERS SHALL BE PLUS OR MINUS 1/4" TYPICAL UNLESS NOTED

FIELD BENDING OR STRAIGHTENING OF DEFORMED BARS SHALL BE LIMITED TO #5 BARS AND SMALLER AND SHALL BE FIELD BENT OR STRAIGHTENED ONLY ONCE. ANY BEND SHALL BE LIMITED TO 90 DEGREES. IF FIELD BENDING OR STRAIGHTENING OF #6 BARS OR LARGER IS REQUIRED, OR IF A SECOND BEND IS REQUIRED FOR #5 BARS AND SMALLER, HEAT SHALL BE APPLIED FOR BENDING OR STRAIGHTENING. CONTRACTOR SHALL SUBMIT PROCEDURE FOR APPLYING HEAT TO ENGINEER FOR REVIEW AND APPROVAL PRIOR TO BENDING OR STRAIGHTENING BARS.

STRUCTURAL STEEL:

ALL CONSTRUCTION PER LATEST AISC STEEL CONSTRUCTION

ALL CONSTRUCTION PER LATEST AISC STEEL CONSTRUCTION MANUAL. ALL WIDE FLANGE STEEL SHALL BE ASTM A992 (Fy = 50 KSI). ALL PIPE STEEL SHALL BE ASTM A500 (Fy = 42 KSI) OR ASTM A53, TYPE E OR S, GRADE B (Fy = 35 KSI). ALL MISCELLANEOUS STEEL UNLESS NOTED OTHERWISE SHALL BE ASTM A36 (Fy = 36 KSI). IF CALLED OUT ON PLANS, Fy = 50 KSI PLATE STEEL SHALL BE ASTM A529 OR A572.

ALL STRUCTURAL ROLLED STEEL MEMBERS WITH FY GREATER THAN 36 KSI ARE TO BE IDENTIFIED WITH AN ASTM SPECIFICATION MARK OR TAG PER IBC SEC. 2203.1. HOLLOW STRUCTURAL SHAPE (HSS):

HSS COLUMNS ARE CALLED OUT ON THE DRAWINGS AS EITHER ASTM A500 (Fy = 46 KSI) OR ASTM A572 (Fy = 65 KSI).ASTM A500 (Fy = 46 KSI) HSS SECTIONS ARE TO BE PRODUCED PER THE SPECIFICATIONS SET FORTH IN AISC.

ASTM A572 (Fy = 65 KSI) HSS SECTIONS ARE TO BE PRODUCED BY DIRECT-FORMING OR FOLDING OF THE PLATE FOLLOWED BY AN ELECTRIC RESISTANCE WELD ALONG THE SEAM. INLINE INSPECTION OF THE WELD ZONE DURING PRODUCTION BY NON-DESTRUCTIVE TESTING (NDT) (ULTRASONIC

THE TERMS PIPE AND ROUND HOLLOW STRUCTURAL SHAPE (HSS) ARE USED SYNONYMOUSLY THROUGHOUT THESE DOCUMENTS ALONG WITH THE TERMS TUBE STEEL AND RECTANGULAR OR

ALL BOLTS SHALL BE ASTM A325 AND SHALL BE INSTALLED AS SLIP CRITICAL CONNECTIONS WITH THREADS INCLUDED IN SHEAR PLANE. TIGHTEN BOLTS PER AISC SPECIFICATIONS. IT IS ACCEPTABLE TO USE OVERSIZE HOLES OR SLOTTED HOLES PER AISC SPECIFICATIONS. PATENTS PENDING

WELDING:

UNLESS NOTED OTHERWISE, ALL WELDS PER LATEST EDITION OF THE AWS STANDARDS. ALL WELDING SHALL BE PERFORMED BY WELDERS HOLDING VALID CERTIFICATES AND HAVING CURRENT EXPERIENCE IN THE TYPE OF WELD SHOWN ON THE DRAWINGS OR NOTES. CERTIFICATES SHALL BE HOSE ISSUED BY AN ACCEPTED TESTING AGENCY. ALL WELDING DONE BY E70 SERIES LOW HYDROGEN RODS UNLESS NOTED OTHERWISE. FOR GRADE 60 REINFORCING BARS, USE E90 SERIES. THESE DRAWINGS DO NOT DISTINGUISH BETWEEN SHOP AND FIELD WELDS: THE CONTRACTOR MAY SHOP WELD OR FIELD WELD AT THEIR DISCRETION. SHOP WELDS AND FIELD WELDS SHALL BE SHOWN ON THE SHOP DRAWINGS SUBMITTED FOR REVIEW.

ALL FULL (COMPLETE) PENETRATION WELDS SHALL BE TESTED AND CERTIFIED BY AN INDEPENDENT

ALL SPOT WELDS SHALL BE PER LATEST AISI AND AWS STANDARDS.

STEEL CONNECTORS: SCREW FASTENERS:

ALL STEEL SCREWS SHALL BE IN ACCORDANCE WITH AISI-GENERAL AND AISI-NAS. Fy = 50 ksi AND Ft = 70 ksi FOR ALL SCREWS.

. MINIMUM SPACING OF SCREWS SHALL NOT BE LESS THAN 3 TIMES THE NOMINAL DIAMETER. MINIMUM EDGE DISTANCE FOR SCREWS SHALL NOT BE LESS THAN 1.5 TIMES THE NOMINAL SCREW 2. THE HEAD OF THE SCREW OR WASHER SHALL HAVE A DIAMETER, DW, OF NOT LESS THAN 5/16". WASHERS SHALL BE AT LEAST 0.05" THICK.

SCREW NUMBER DESIGNATION	8	10	12 (12–14)	14
NOMINAL DIAMETER	0.164"	0.190"	0.216"	0.250*

COLD FORMED STRUCTURAL STEEL FRAMING:

ALL COLD FORMED STEEL COMPONENTS INDICATED ON THE STRUCTURAL DRAWINGS SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND IN ACCORDANCE WITH THE LATEST EDITION OF "SPECIFICATIONS FOR THE DESIGN OF COLD—FORMED steel structural members" by the AISI.

ALL STRUCTURAL STEEL FRAMING MATERIAL AND ITS ERECTION SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE AMERICAN IRON AND STEEL INSTITUTE "SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBER".

ALL WELDING TO BE PERFORMED BY WELDERS HOLDING A VALID CERTIFICATE AND HAVING CURRENT EXPERIENCE IN LIGHT GAUGE STEEL. CERTIFICATES SHALL BE ISSUED BY AN ACCEPTED TESTING AGENCY. DO NOT NOTCH FLANGES OF MEMBERS WITHOUT EXPRESSED APPROVAL OF THE ENGINEER OF RECORD. ALL WELDING TO BE PERFORMED IN AN APPROVED FABRICATORS SHOP.

STRUCTURAL STEEL MEMBERS ARE FURNISHED TO A SPECIFIED MINIMUM Fy = 55,000 PSI. U.N.O. THE GRADE AND THE ASTM SPECIFICATION NUMBER OR OTHER SPECIFICATION DESIGNATION SHALL BE INDICATED BY PAINTING, DECAL, TAGGING OR OTHER SUITABLE MEANS ON EACH BUNDLE OF FABRICATED ELEMENTS. IT IS ACCEPTABLE TO USE THE FY SHOWN ON THE MILL CERTIFICATION IN LIEU OF THE "ORDERED" FY. IT IS ACCEPTABLE TO USE STEEL WITH FY = 70 KSI IF THE STEEL USED IS IN THE AISI AND/OR AISC SPECIFICATION, THE ELONGATION IN A 2" COUPON IS A MINIMUM OF 10% AND THE RATIO OF Ft OVER FY IS AT LEAST 1.08.

GAGE NO.	MIN DELIVERED THICKNESS	DESIGN THICKNESS
30	0.0120"	0.0126
29	0.0132*	0.0139*
26	0.0174"	0.0183"
20	0.0336°	0.0354*
18	0.0447"	0.0470*
16	0.0561*	0.0590"
14	0.0713"	0.0750*
12	0.0998"	0.1050"
10	0.1283*	0.1350*
	30 29 26 20 18 16 14	30 0.0120" 29 0.0132" 26 0.0174" 20 0.0338" 18 0.0447" 16 0.0561" 14 0.0713" 12 0.0998"

## **GENERAL NOTES:**

THE STRUCTURAL CONSTRUCTION DOCUMENTS REPRESENT THE FINISHED STRUCTURE. EXCEPT WHERE NOTED, THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. THE STRUCTURAL ENGINEER OF RECORD SHALL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S MEANS, METHODS, TECHNIQUES, SEQUENCES FOR PROCEDURE OF CONSTRUCTION, OR THE SAFETY PRECAUTIONS AND THE PROGRAMS INCIDENT THERETO (NOR SHALL OBSERVATION VISITS TO THE SITE INCLUDE INSPECTION OF THESE ITEMS).

WHERE REFERENCE IS MADE TO VARIOUS TEST STANDARDS FOR MATERIALS, SUCH STANDARDS SHALL BE THE LATEST EDITION AND/OR ADDENDA. ANY ENGINEERING DESIGN, PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW, SHALL BEAR THE SEAL OF A REGISTERED ENGINEER RECOGNIZED BY THE BUILDING CODE JURISDICTION OF THIS PROJECT.

NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT, AND/OR AS PROVIDED FOR IN THE CONTRACT DOCUMENTS. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL STRUCTURAL NOTES AND SPECIFICATIONS. THE GREATER REQUIREMENTS SHALL GOVERN.

CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION OF ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION. RESOLVE ANY DISCREPANCY WITH THE ARCHITECT. ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING AND ELECTRICAL ITEMS WITH THE APPROPRIATE TRADE DRAWINGS AND SUBCONTRACTORS

TYPICAL DETAILS MAY NOT NECESSARILY BE CUT ON PLANS, BUT APPLY UNLESS NOTED OTHERWISE. CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED CONSTRUCTION. LOAD SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT.

OPTIONS ARE FOR CONTRACTOR'S CONVENIENCE. IF AN OPTION IS CHOSEN, CONTRACTOR SHALL BE RESPONSIBLE FOR ALL NECESSARY CHANGES, APPROVALS AND THE COORDINATION OF THE WORK WITH ALL RELATED TRADES AND SUPPLIERS.

SPECIAL INSPECTION - STRUCTURAL ONLY:

SPECIAL INSPECTION IS REQUIRED PER CHAPTER 17A OF THE CBC FOR THE FOLLOWING: CONCRETE CONSTRUCTION:

DURING THE TAKING OF TEST SPECIMENS. B. THE PLACEMENT OF ALL FOUNDATION CONCRETE.

2. REINFORCING STEEL: INSPECTION OF IN-PLACE REINFORCING FOR CONFORMANCE PRIOR TO THE CLOSING OF FORMS OR THE DELIVERY OF CONCRETE TO THE JOBSITE FOR THE FOLLOWING: A. REINFORCING FOR SPREAD FOOTING AND DRILLED PIER CONCRETE FOUNDATIONS.
B. REINFORCING FOR INVERTER SLABS ON THE GROUND.

STEEL CONSTRUCTION:

1. WELDING: PERIODIC VISUAL INSPECTION OF ALL FIELD WELDS. 3. CONTINUOUS INSPECTION OF ALL MULTIPASS FILLET WELDS OR SINGLE PASS FILLET WELDS C. NON-DESTRUCTIVE TESTING OF ALL COMPLETE PENETRATION WELDS BY AN AWS CERTIFIED INDEPENDENT TESTING LABORATORY AT THE CONTRACTORS EXPENSE.

VERIFICATION OF VALID WELDER'S CERTIFICATES. E. ALL STRUCTURAL STEEL FABRICATORS SHALL EMPLOY AN AWS CERTIFIED INDEPENDENT TESTING LAB TO PROVIDE SHOP WELD INSPECTIONS PER CODE. INSPECTION REPORTS SHALL BE SUBMITTED TO ENGINEER OF RECORD PRIOR TO STEEL INSTALLATION.

2. STEEL FRAMES: VERIFICATION OF BRACING, STIFFENING, MEMBER LOCATIONS, AND PROPER JOINT DETAIL APPLICATION AT ALL STEEL FRAME CONNECTIONS.

A. VERIFICATION OF SLIP CRITICAL BOLT INSTALLATION FOR ASTM A325 BOLTS. DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:

A. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED TO BE CERTAIN IT CONFORMS TO THE APPROVED DESIGN DRAWINGS AND SPECIFICATION. B. THE SPECIAL INSPECTOR IS NOT AUTHORIZED TO APPROVE DEVIATIONS FROM THE DESIGN DRAWINGS OR SPECIFICATIONS, AND ALL DEVIATIONS MUST BE APPROVED BY THE STRUCTURAL ENGINEER OF RECORD AND/OR DSA PRIOR TO PROCEEDING WITH THE WORK. ALL REQUESTS FOR DEVIATIONS SHALL BE INITIATED BY THE CONTRACTOR VIA WRITTEN REQUEST FOR INFORMATION (RFI). C. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE DSA AND TO THE ENGINEER OR ARCHITECT OF RECORD. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF UNCORRECTED, TO THE DSA AND/OR THE ARCHITECT OR ENGINEER OF RECORD. CONTRACTOR SHALL PROVIDE THE SPECIAL INSPECTOR ACCESS TO ALL ITEMS REQUIRING SPECIAL INSPECTION. ACCESS SHALL BE PROVIDED BY IN-PLACE LADDERS, SCAFFOLDS, LIFTS AND/OR OTHER EQUIPMENT OPERATED BY THE CONTRACTOR'S PERSONNEL AS REQUIRED FOR SAFE OBSERVATION. INSPECTOR IS NOT RESPONSIBLE OR AUTHORIZED TO OPERATE CONTRACTOR'S E. UPON COMPLETION OF THE ASSIGNED WORK THE ENGINEER OR ARCHITECT SHALL COMPLETE AND SIGN THE APPROPRIATE FORMS CERTIFYING THAT TO THE BEST OF THEIR KNOWLEDGE THE WORK IS IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS, AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE CODE.

THE SOLAR PANELS AND THEIR ANCHORAGE SYSTEMS ARE DEFERRED ITEMS. PER TITLE 24, PART 1, SECTION 4-317 (g), THEIR DESIGNS SHALL BE REVIEWED AND APPROVED BY DSA PRIOR TO INSTALLATION. THE DEFERRED SUBMITTAL DOCUMENTS SHALL BE STAMPED AND SIGNED BY EITHER AN ARCHITECT OR REGISTERED ENGINEER WITH A VALID CALIFORNIA LICENSE. PLEASE NOTE THAT ADDITIONAL CANOPY FRAMING AND BEARING BLOCKS MAY BE REQUIRED FOR CONNECTING THE SOLAR PANEL ANCHORAGE SYSTEM TO

NOTES FOR SITE SPECIFIC PHOTOVOLTAIC (PV) INSTALLATION:

THESE DRAWINGS ARE FOR THE STEEL STRUCTURES SUPPORTING PV PANELS. NO PROVISIONS ARE INCLUDED IN THESE DRAWINGS FOR THE PV PANELS OR THE PV

THE PV PANELS AND THE PV PANEL INSTALLATION SHALL BE SUBMITTED AS A SITE SPECIFIC APPLICATION. (REFER TO THE BOX NOTE REGARDING THE SOLAR PANELS AND THEIR ANCHORAGE BEING A DEFERRED ITEM). PV PANELS SHALL BE INSTALLED PER DRAWINGS THAT HAVE BEEN SUBMITTED TO AND REVIEWED PERMITTED BY DSA. THE PV DRAWINGS SHALL PROVIDE THE MINIMUM FOLLOWING INFORMATION.

LOCATION ALL ELECTRICAL EQUIPMENT.
WRING DIAGRAMS TO AND FROM ALL PV PANELS AND ELECTRICAL EQUIPMENT.
ALL GROUNDING DETAILS FOR STRUCTURES AND EQUIPMENT.

ALL DISCONNECTION LOCATIONS AND DETAILS. EQUIPMENT WARNING LABELS FOR INVERTER OVER VOLTAGE. SINGLE 120 VOLT SUPPLY WITHOUT MULTI BRANCH CIRCUITS AND ELECTRICAL SHOCK HAZARD.

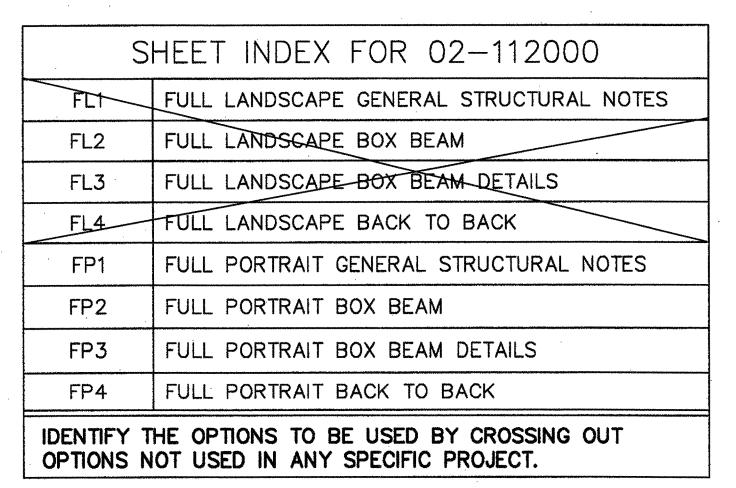
REFER TO CEC ARTICLE 690 FOR ADDITIONAL REQUIREMENTS AND DETAILS.

NOTE: PV SYSTEM SHALL BE MARKED. MARKING IS NEEDED TO PROVIDE EMERGENCY RESPONDERS WITH APPROPRIATE WARNING AND GUIDANCE WITH RESPECT TO ISOLATING THE SOLAR ELECTRIC SYSTEM. THIS CAN FACILITATE IDENTIFYING ENERGIZED ELECTRICAL LINES THAT CONNECT THE SOLAR PANELS TO THE INVERTER AND MAIN SERVICE DISCONNECT. THE LABEL SHALL BE OF A WEATHER-RESISTANT MATERIAL SUITABLE FOR THE ENVIRONMENT. MARKING CONTENT SHALL READ: "CAUTION: SOLAR ELECTRIC SYSTEM CONNECTED". THIS LABEL SHALL BE PLACED ADJACENT TO THE MAIN SERVICE DISCONNECT IN A LOCATION CLEARLY VISIBLE FROM THE LOCATION WHERE THE LEVER IS

ADDITIONAL MARKING IS REQUIRED OF THE DC CIRCUIT. MARKING IS REQUIRED ON ALL INTERIOR AND EXTERIOR DC CONDUIT, RACEWAYS, ENCLOSURES, CABLE ASSEMBLIES AND JUNCTION BOXES TO ALERT FIRE SERVICE TO AVOID CUTTING THEM. MARKING SHALL BE PLACED EVERY 10 FEET, AT TURNS AND ABOVE AND/OR BELOW PENETRATIONS AND AT ALL DC COMBINER AND JUNCTION BOXES. MARKING FOR CIRCUIT SHALL READ: "CAUTION: SOLAR CIRCUIT".

		,		
	GOVERNING LO	M MAX(K')	V MAX(K)	
	PURLIN	DL + 0.75W + 0.75Lr	4.05	0.68
	BEAM 3P	DL + 0.75W + 0.75Lr	50.53	6.73
	BEAM 4P	DL + 0.75W + 0.75Lr	99.07	9.65
10.5' CLR.	COLUMN AND FOOTING STRONG AXIS 3P	DL + 0.75W (MWFRS) + 0.75Lr	56.75	2.68
	COLUMN AND FOOTING STRONG AXIS 4P	DL + 0.75W (MWFRS) + 0.75Lr	104.46	3.64
	COLUMN AND FOOTING WEAK AXIS 3P	(1 + .14 SDS) DL + 0.7pE	<del>37.80</del>	2.69
	COLUMN AND FOOTING WEAK AXIS 4P	(1 + .14 SDS) DL + 0.7pE	56.71	3.67
12' CLR.	COLUMN AND FOOTING STRONG AXIS 3P	DL + 0.75W (MWFRS) + 0.75Lr	<del>57.84</del>	2.68
	COLUMN AND FOOTING STRONG AXIS 4P	DL + 0.75W (MWFRS) + 0.75Lr	106.04	3.65
	COLUMN AND FOOTING WEAK AXIS 3P	(1 + .14 SDS) DL + 0.7pE	41.95	2.69
	COLUMN AND FOOTING WEAK AXIS 4P	(1 + .14 SDS) DL + 0.7pE	62.39	3.68

3P = 3 PANELS. 4P = 4 PANELS



IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT

APP03 1 1 4 5 8 5 ACNIN. FLS 7c SS CMC

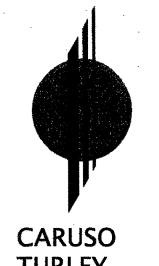
PATENTS PENDING

S 3325

## PRE-CHECK (PC) **DOCUMENT** CODE: 2010 CBC

A SEPARATE PROJECT APPLICATION FOR **CONSTRUCTION IS REQUIRED** 

IDENTIFICATION STAMP



SCOTT consulting structural engineers 1215 W. Rio Salado Pkwy Suite 200 Tempe, Arizona 85281 (480) 774-1700 (480) 774-1701 FAX

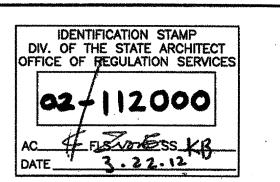
www.ctsaz.com

DRAWING EDITION/REF JOB #

REVISIONS:

SITE PROJECT:

DSA APP. NO 02-112000



JOB NUMBER: 11-071 DRAWN: ENGINEER: CHECKED BLP PGS DST 3/15/12 FP1

