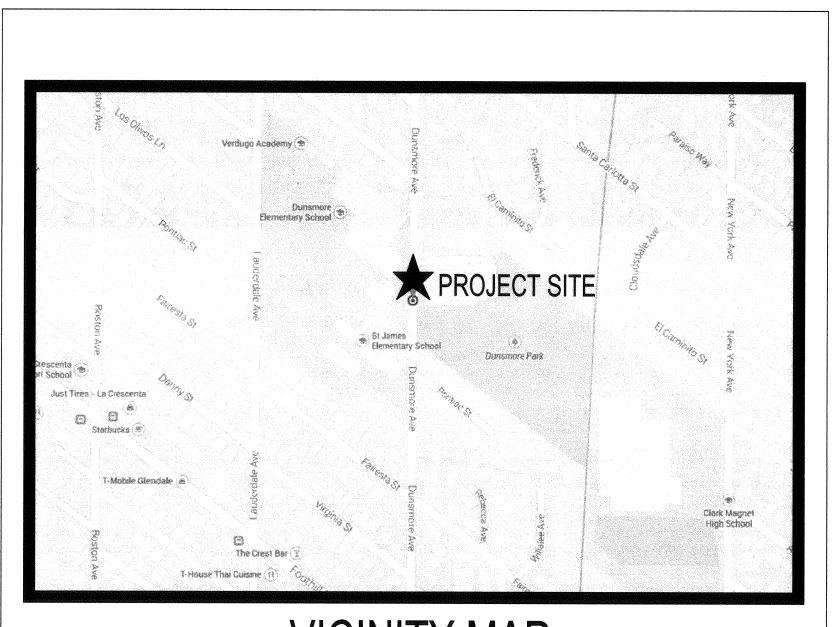
KINDERGARTEN SHADE STRUCUTRES DUNSMORE ELEMENTARY SCHOOL



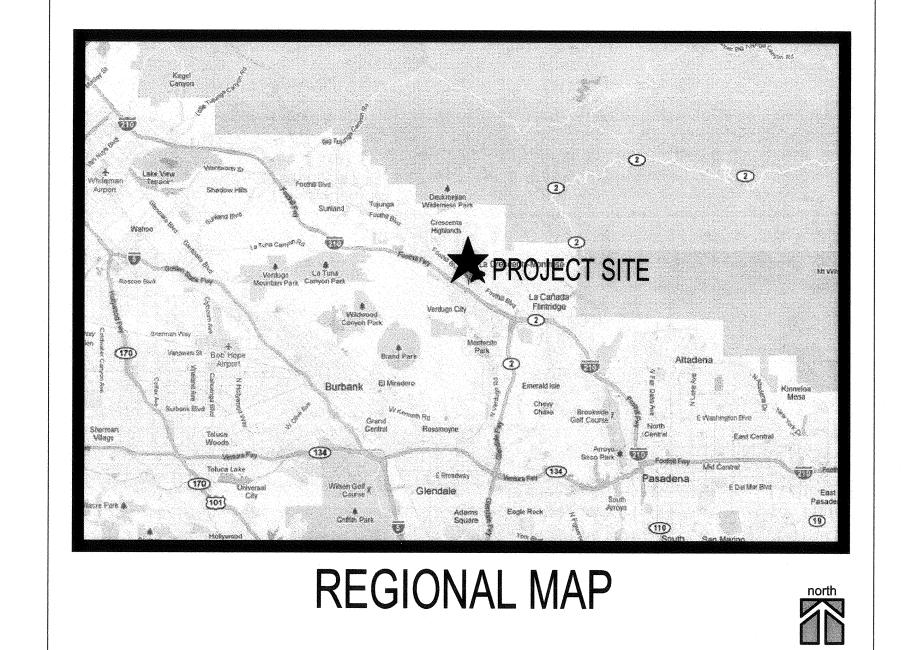
4717 DUNSMORE AVENUE, LA CRESCENTA, CALIFORNIA

GLENDALE UNIFIED SCHOOL DISTRICT



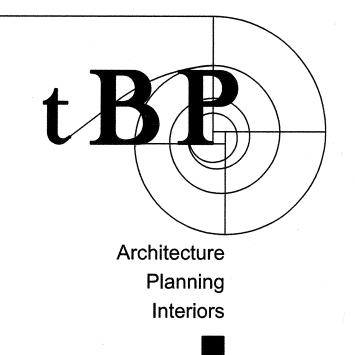


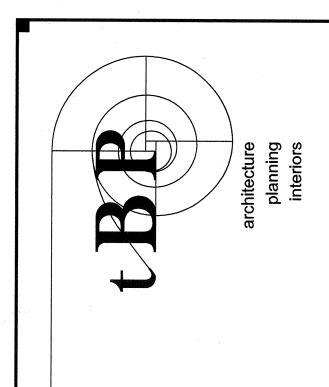
VICINITY MAP

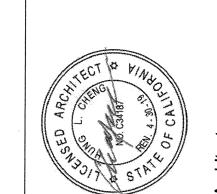


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DENTIFICATION STAMP
DIVISION OF THE STATE ARCHITECT
DEPARTMENT OF GENERAL SERVICES

O3-119278

AC ME FLS SS SS

DATE AUG 29 2018

DEPARTMENT OF GENERAL SERVICES
DSA Los Angeles Regional Office
700 N. Alameda Street, Suite 5-500
Los Angeles, California 90012
ph: (213)897-3995 fx: (213)897-3159/0726

EMENTARY SCHOOL
SHADE STRUCTURES
TED SCHOOL DISTRICT

KINDERGARTEN SHADE S

GLENDALE UNIFIED SCHOOL

4717 DUNSMORE AV

tBP project number: 20907.01

file name:

drawn by: checked by:

date: August 14, 2018

Rev: date: description:

DUNSMORE

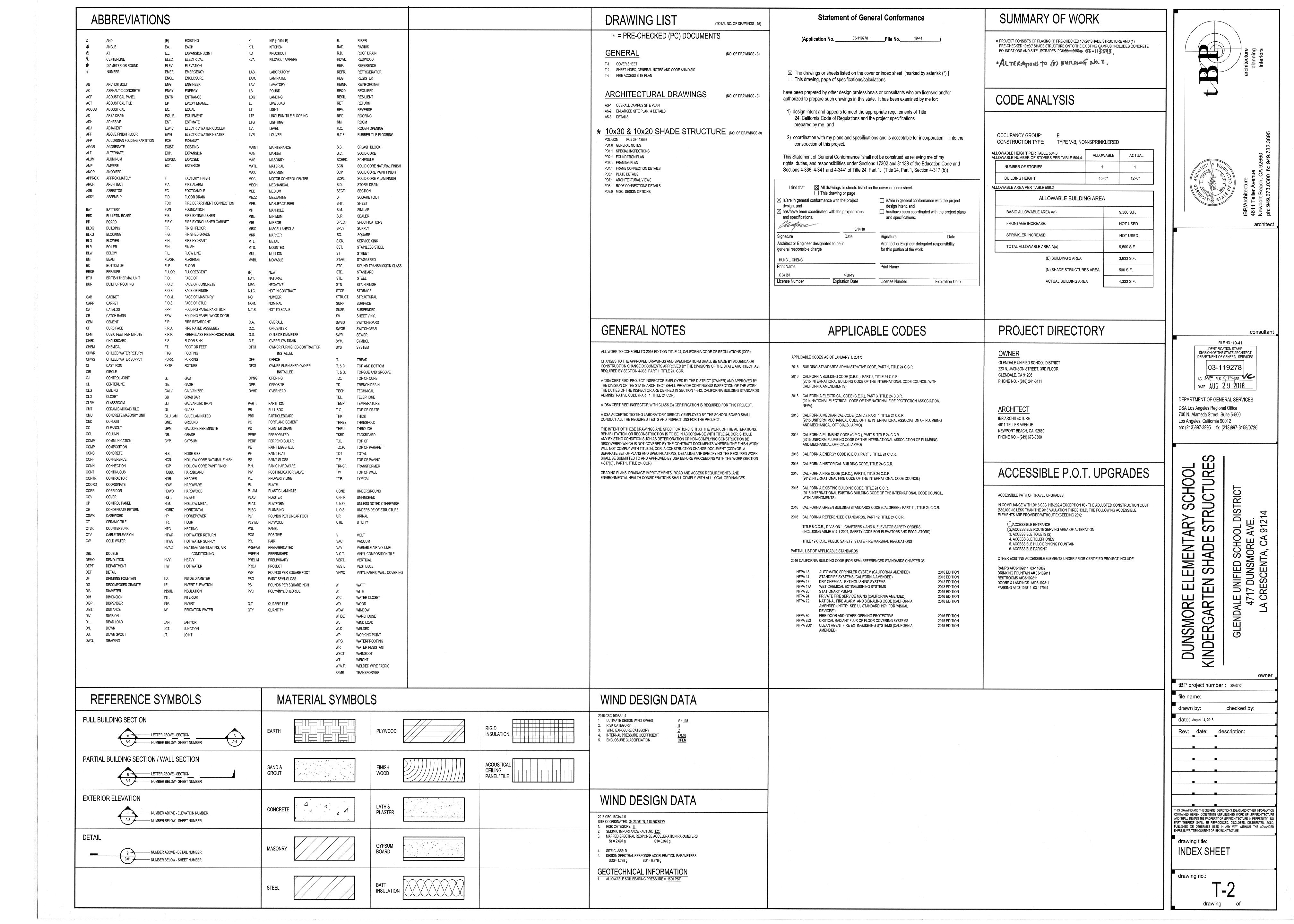
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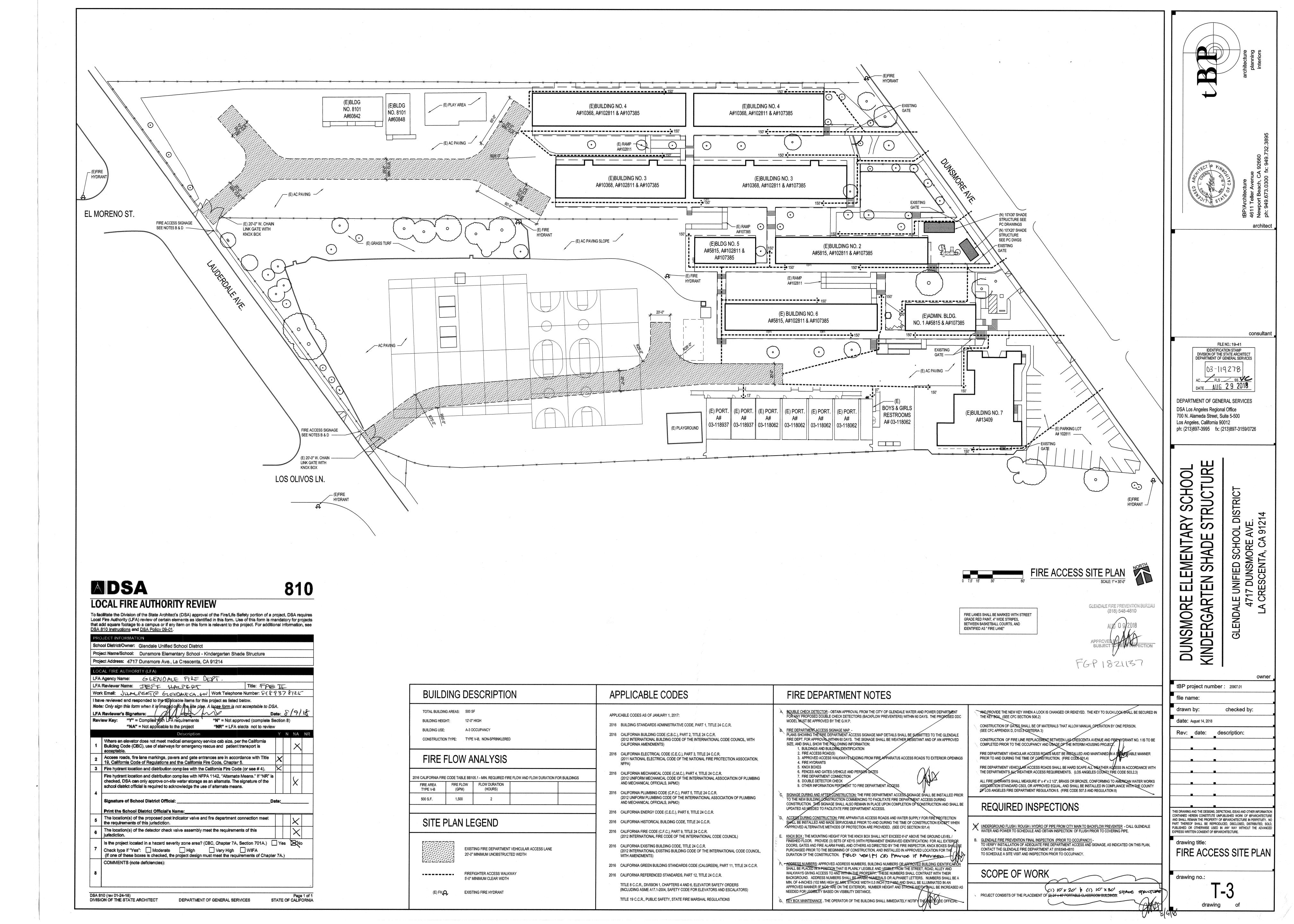
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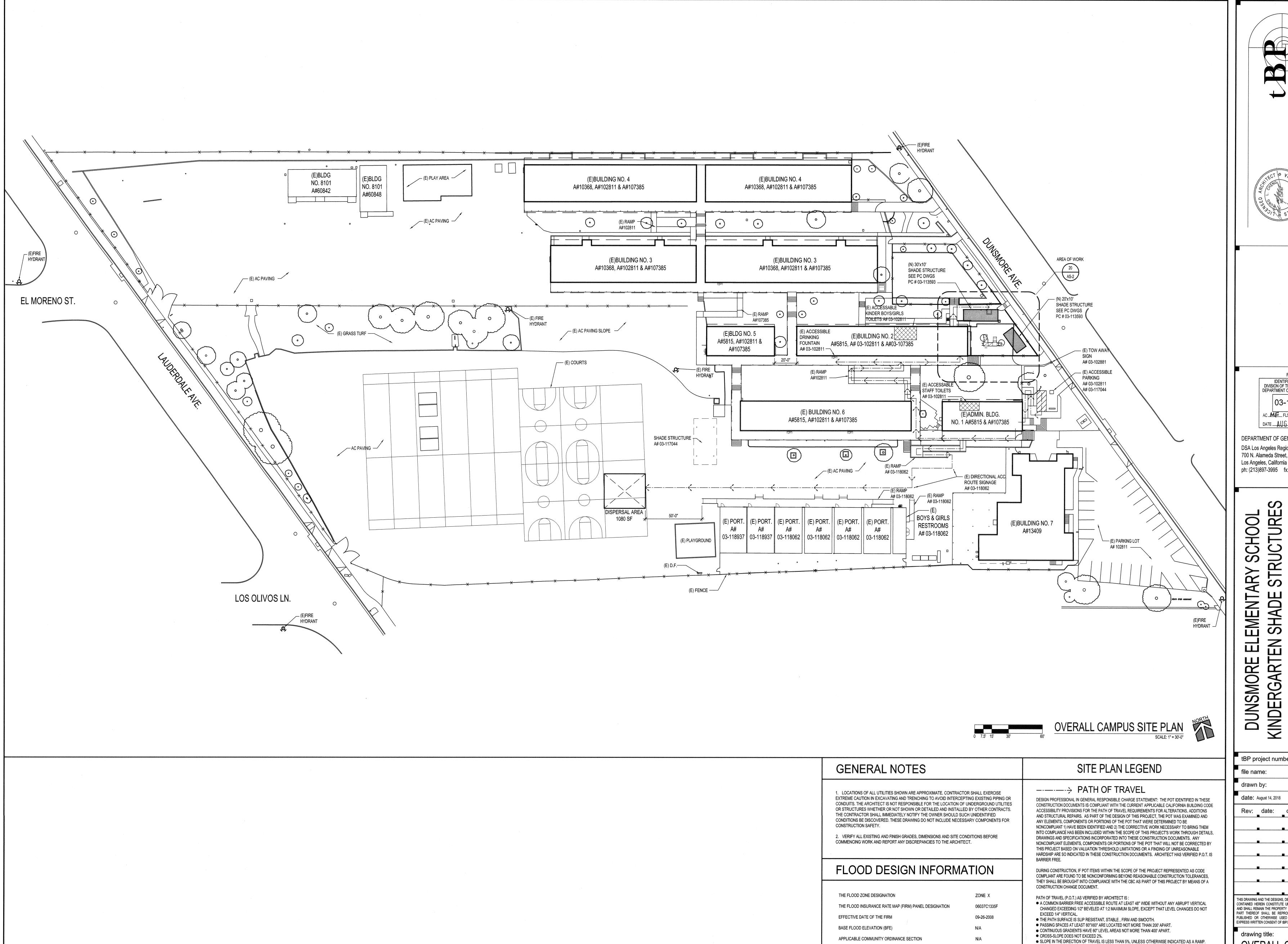
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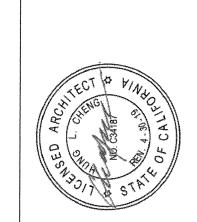
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consultant

IDENTIFICATION STAMP DIVISION OF THE STATE ARCHITECT DEPARTMENT OF GENERAL SERVICES AC ME FLS SET SS VE DATE AUG 29 2018

DEPARTMENT OF GENERAL SERVICES DSA Los Angeles Regional Office 700 N. Alameda Street, Suite 5-500

Los Angeles, California 90012 ph: (213)897-3995 fx: (213)897-3159/0726

STRUCTURES

GLENDALE UNIFIED SCHOOL DISTRIC 4717 DUNSMORE AVE. LA CRESCENTA, CA 91214

tBP project number: 20907.01 checked by:

date: August 14, 2018

Rev: date: description:

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OVERALL CAMPUS SITE PLAN

drawing no.:

MAINTAIN POT FREE OF OVERHANGING OBSTRUCTIONS TO 80" MINIMUM, PROTRUDING OBJECTS

SHADED AREA INDICATES AREA OF NEW WORK

(E) ACCESSIBLE RESTROOMS - A# 03-102811

GREATER THAN 4". PROJECTION FROM WALL OR EDGE AND 27" ABOVE FINISH GRADE.

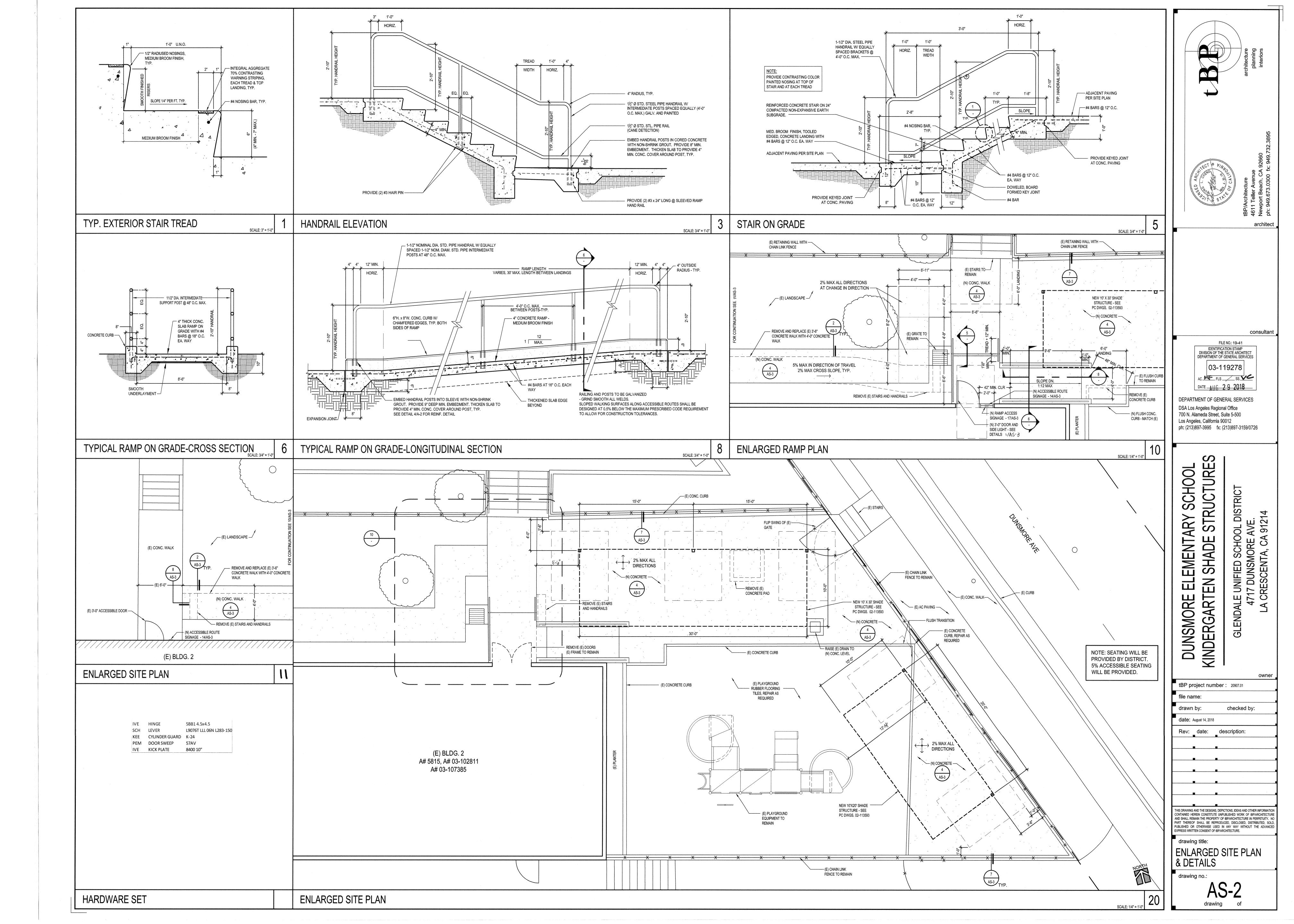
PARKING TABULATION

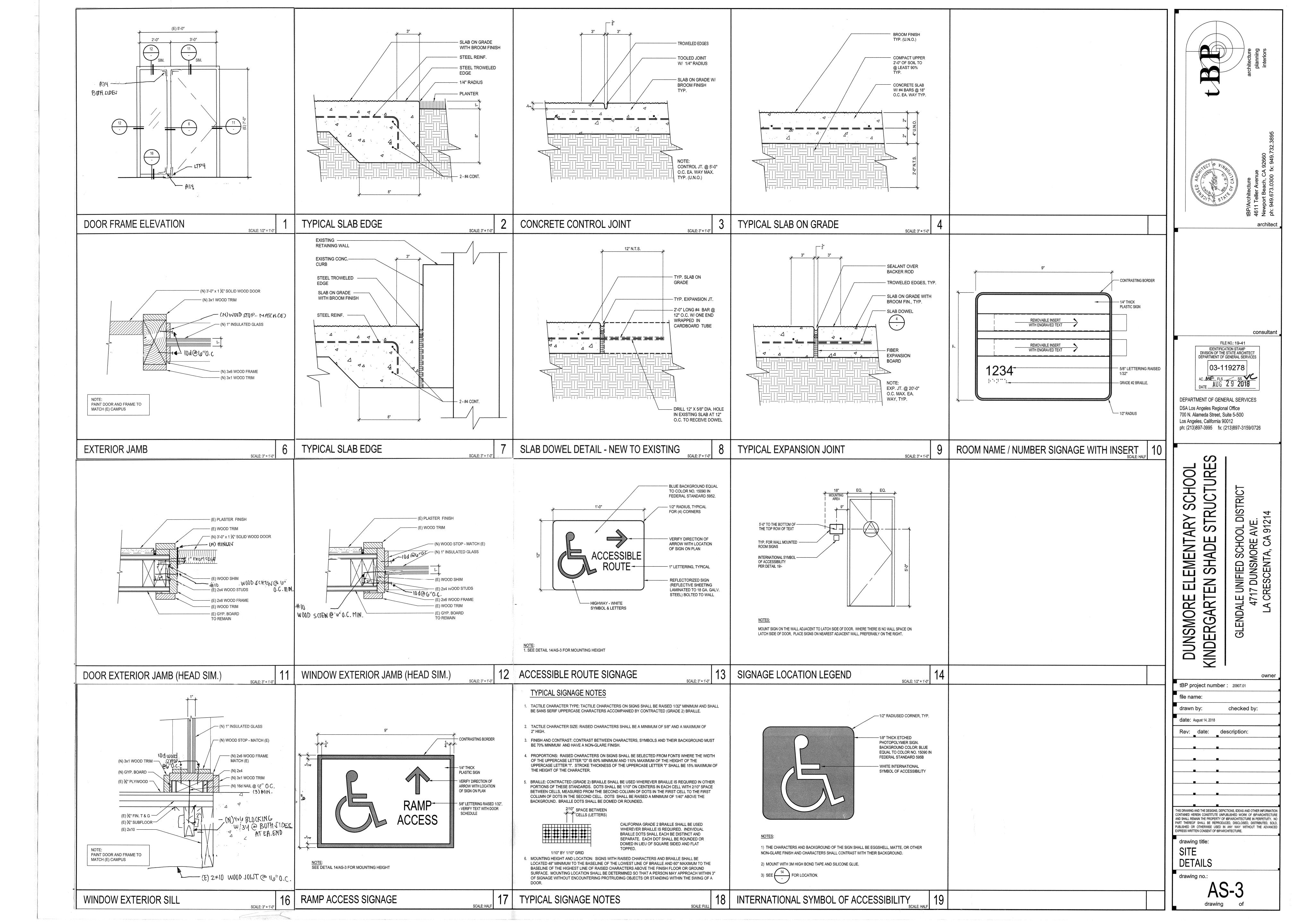
25 PARKING STALLS

1 ACCESSIBLE PARKING STALL

PARKING LOT:

drawing





DESCRIPTION

MAY ADJUST OCCUPANT LOAD AS PERMITTLED BY THE BUILDING CDE.

RELATED BUILDING CODES AND STANDARDS:

2013 California Building Code (CBC), Volumes 1, and 2 (Part 2, Title 24, CCR) (2012 International Building Code with 2013 California amendments)

National Electrical Code with 2013 California amendments

(2012 Uniform Plumbing Code with 2013 California amendments)

(2012 International Fire Code with 2010 California Amendments)

2013 California Mechanical Code (CMC)(Part 4, Title 24, CCR) (2012 Uniform Mechanical Code with 2013 California amendments)

2013 California Plumbing Code (CPC)(Part 5, Title 24, CCR)

2013 California Green Building Standards Code.....(Part 11, Title 24, CCR)

013 California Administrative Code (CAC).......

2013 California Referenced Standards Code

REFERENCE CODE SECTIONS FOR APPLICABLE STANDARDS:

SCOPE OF WORK NARRATIVE:

OF PROJECT SITES AND LOADING REQUIREMENTS.

POLIGON ASSUMES ANY OCCUPANT LOAD CALCULATIONS ARE BASED ON 15 SQ FT/ PERSON. PROJECT ARCHITECT

....(Part 3, Title 24, CCR)

.....(Part 6, Title 24, CCR)

....(Part 9, Title 24, CCR)

.... (Part 12, Title 24, CCR)

HESE DRAWINGS ILLUSTRATE THE FABRICATION AND INSTALLATION REQUIREMENTS FOR A FREE-STANDING PREFABRICATED TEEL SHADE STRUCTURE. THE ENTIRE STRUCTURAL SYSTEM IS COMPRISED OF TUBULAR STEEL MEMBERS SUPPORTED ON

CONCRETE FOUNDATIONS. THE FLEXIBILITY INCLUDED HEREIN ALLOWS THIS STRUCTURE TO COMPLY WITH A WIDE VARIETY

TYPE OF CONSTRUCTION

NUMBER OF STORIES

FIRE SPRINKLER SYSTEM

TITLE 24 CODES:

OCCUPANCY CLASSIFICATION

FIRE HAZARD SEVERITY ZONE

2013 California Electrical Code .

2013 California Energy Code .

NFPA 13 - 2013

NFPA 72 - 2013

2013 CBC, CHAPTER 35 2013 CFC, CHAPTER 45

(Effective July 1, 2014)

2013 California Fire Code (CFC).

(Effective January 1, 2014)

- GENERAL NOTES AND TYPICAL DETAILS SHALL APPLY TO ALL PARTS OF THE JOB EXCEPT WHERE THEY MAY CONFLICT WITH DETAILS AND NOTES ON OTHER SHEETS. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER FOR THIS PROJECT.
- WORK SHALL CONFORM TO THE REQUIREMENTS, AS AMENDED TO DATE, OF THE LATEST ADOPTED EDITION OF THE CBC, C.A.C. TITLE 24, AND ALL OTHER LOCAL, STATE AND FEDERAL REGULATIONS.
- OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT PRIOR TO PROCEEDING WITH ANY WORK INVOLVED.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHALL CHECK ALL DIMENSIONS. ALL DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT AND BE RESOLVED BEFORE PROCEEDING WITH THE WORK.
- THESE CONSTRUCTION DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO, BRACING, TEMPORARY SUPPORTS, AND SHORING. OBSERVATION VISITS TO THE SITE BY FIELD REPRESENTATIVES OF THE ARCHITECT/ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE 2. PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. ANY SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER DURING THE CONSTRUCTION SHALL BE DISTINGUISHED FROM CONSTRUCTION AND DETAILED NSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER, WHETHER OF MATERIAL OR WORK, ARE FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DOCUMENTS, BUT DO NOT GUARANTEE
- ASTM DESIGNATIONS AND ALL STANDARDS REFER TO THE LATEST AMENDMENTS.
- CONFORM TO APPLICABLE CAL/OSHA CONSTRUCTION SAFETY REGULATIONS FOR ALL WORK PERFORMED DURING CONSTRUCTION. JOB SITE SAFETY IS STRICTLY THE RESPONSIBILITY OF THE CONTRACTOR AND NOT THE ARCHITECT/ENGINEER OR OWNER.
- THE ENGINEER AND THEIR CONSULTANTS SHALL HAVE NO RESPONSIBILITY FOR THE DISCOVERY, HANDLING, REMOVAL OR DISPOSAL OF HAZARDOUS MATERIALS AT THE PROJECT SITE, INCLUDING BUT NOT LIMITED, TO ASBESTOS, ASBESTOS PRODUCTS, POLYCHLORINATED BIPHENYL (PCB) OR OTHER TOXIC SUBSTANCES.
- SHOULD ANY CONDITIONS DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS, OR IF A CHANGE IN THE SCOPE OF WORK IS PROPOSED, A CONSTRUCTION CHANGE DOCUMENT DETAILING AND SPECIFYING THE REQUIRED CHANGE(S) SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK
- 10. THE SCHOOL DISTRICT'S INSPECTOR OF RECORD SHALL INSPECT AND APPROVE THE ERECTED FRAME PRIOR TO SEE REQUIREMENTS FOR LOCATION IN ANY FIRE HAZARD SEVERITY ZONE FOR WILDLAND URBAN INTERFACE AREAS (WUI) AS SPECIFIED IN THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. PROVIDE PROTECTION AND
- DETAILS OF ALL AREAS COMPLYING WITH THE WUI REQUIRMENTS. LOCATING THIS STRUCTURE CLOSER THAN 20 FEET TO OTHER STRUCTURES MAY AFFECT THE ALLOWABLE AREA FOR
- THE EXISTING CONSTRUCTION PER THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. 13. VIEWS AND DETAILS ARE NOT DRAWN TO SCALE (UNLESS NOTED OTHERWISE). DO NOT SCALE THESE DRAWINGS.
- 14. OTHER SITE SPECIFIC ITEMS MAY BE REQUIRED.

STRUCTURAL AND MISCELLANEOUS STEEL:

- ALL STRUCTURAL. STEEL SHALL BE DETAILED. FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) SPECIFIATION MANUAL REFERENCED BY THE LATEST EDITION OF THE CALIFORNIA BUILDING CODE.
- PIPE SECTIONS SHALL CONFORM TO ASTM A53, Fy = 35 ksi, GRADE B OR A501 UNLESS NOTED OTHERWISE. STRUCTURAL TUBING (HSS SHAPES) SHALL CONFORM TO ASTM A500, GRADE B (OR HIGHER), Fy = 46 KSI.
- IF MATERIAL AVAILABILITY IS LIMITED, MEMBER THICKNESSES CAN BE INCREASED BEYOND WHAT IS SHOWN IN THESE DRAWINGS (MAXIMUM INCREASE OF 1/8").
- ALL CHANNELS, ANGLES, AND MISC. STEEL SHALL CONFORM TO ASTM A36, Fy = 36 KSI. ALL COLD FORM STEEL SHALL CONFORM TO ASTM A653, CS = TYPE B, Fy = 50 KSI.
- STRUCTURAL STEEL AND DECK SHALL BE IDENTIFIED FOR CONFORMITY PER CBC 2203A.1.
- ROOF DECK SHALL HAVE KYNAR 5000 METAL COATING.
- ROOF DECK SHALL CONFORM TO ASTM A792, Fy = 50 KSI.
- MR ROOF SCREWS MEET ASTM A510 WITH A HEAD DIMENSION OF 0.31" (FLAT-TO-FLAT) AND INTEGRAL WASHER DIMENSION OF 0.58" (OUTSIDE DIAMETER).
- SS ROOF SCREWS MEET ASTM A510 WITH A HEAD DIMENSION OF 0.437" (OUTSIDE DIAMETER).

- ALL WELDING SHALL COMPLY WITH AWS D1.1 SPECIFICATIONS AND SHALL BE DONE BY AWS QUALIFIED WELDERS CERTIFIED FOR THE TYPE OF WELDING TO BE PERFORMED AS REQUIRED BY DSA.
- ALL WELDING SHALL BE DONE BY GAS METAL ARC PROCESS WITH E70XX ELECTRODES. FLUX CORE ARC WELD SHALL CONFORM TO CHARPY NOTCH TOUGHNESS RATING OF 20 ft-lb @ (0° F).
- ALL WELDING SHALL BE DONE IN THE SHOP WITH REQUIRED INSPECTION, PRE-APPROVED BY DSA, TO INSURE PROPER MATERIAL ID AND WELDING. WELD FILLER METAL MANUFACTURER SHALL PROVIDE WRITTEN CERTIFICATION OF COMPLIANCE WITH CODE AND

SPECIFICATIONS.

- ALL BOLTS SHOWN ON THESE DRAWINGS ARE ASTM A325 HIGH STRENGTH BOLTS (UNO), TYPE 3.
- HIGH STRENGTH BOLTS SHALL BE SAMPLED AND TESTED IN COMPLIANCE WITH CBC 2213A.1. BEFORE ERECTING THE FRAME, VERIFY ALL BOLTS AND NUTS ARE CLEAN OF DEBRIS AND BURRS - INCLUDING THE HARDWARE ALREADY FASTENED INSIDE THE MEMBERS. CHASING SOME OF THE BOLTS AND NUTS MAY BE REQUIRED.
- ANCHOR BOLTS (HEAVY HEX HEAD, ASTM F1554, GRADE 55) SHALL BE HOT DIPPED GALVANIZED PER ASTM F2329. ANCHOR BOLTS MAY BE HEADED OR THREADED WITH A NUT THAT IS PREVENTED FROM ROTATING.
- HIGH STRENGTH NUTS SHALL CONFORM TO ASTM A563. HIGH STRENGTH WASHERS SHALL CONFORM TO ASTM F436.
- THE BOLTING INSTALLATION REQUIREMENTS OUTLINED BELOW ARE CRITICAL TO THE STRUCTURE'S DESIGN AND PERFORMANCE. THE INSTALLER IS REQUIRED TO COORDINATE THIS PHASE OF CONSTRUCTION WITH THE SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD PRIOR TO THE ERECTION OF THE FRAME. ALL BOLTS SHALL BE INSTALLED AND INSPECTED PER THE APPLICABLE VERSION OF AISC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS", CBC 1705A.2.1; AISC 341-10 J7; AISC 360-10 N5.6.
 - PRETENSIONED JOINTS (IDENTIFIED ON THE FRAME CONNECTION DETAILS WITH A "PJ REQUIRED") MUST BE INSTALLED AND INSPECTED TO MEET ONE OF FOLLOWING REQUIREMENTS:
 - 1. TURN-OF-NUT PRETENSIONING
 - 2. CALIBRATED WRENCH PRENTENSIONING
 - 3. DIRECT-TENSION-INDICATOR PRETENSIONING (CONTRACTOR RESPONSIBLE FOR PURCHASE OF REQUIRED WASHERS)
- B. ALL OTHER JOINTS MUST BE INSTALLED AND INSPECTED TO MEET THE REQUIREMENTS OF SNUG-TIGHTENED JOINTS. NOTE TO INSTALLER AND INSPECTOR(S): THE SNUG-TIGHT CONDITION EXISTS, IN PART, WHEN ALL THE BOLTS IN THE JOINT HAVE BEEN TIGHTENED SUFFICIENTLY TO PREVENT THE REMOVAL OF THE NUTS WITHOUT THE THE CONTRACTOR, SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD MUST ALL AGREE ON WHICH
- APPROACH WILL BE USED TO PRETENSION THE BOLTS. THE CONTRACTOR IS RESPONSIBLE FOR DOCUMENTING THE APPROACH AGREED TO BY ALL PARTIES LISTED ABOVE.

FOUNDATIONS

- ALLOWABLE SOIL PRESSURES ASSUME CLASS 4 SOIL CLASSIFICATION PER CBC TABLE 1806A.
- A GEOTECHNICAL REPORT / LETTER IS REQUIRED AT THE OVER-THE-COUNTER APPOINTMENT FOR EACH PROJECT. FILL AND BACKFILL SHALL BE COMPACTED TO 95% OF MAX. DENSITY IN ACCORDANCE WITH ASTM TEST METHOD
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING, ETC. NECCESSARY TO SUPPORT CUT AND/OR FILL BANKS DURING EXCAVATION, AND FORMING AND PLACEMENT OF CONCRETE.

CONCRETE:

D1557-70. FLOODING NOT PERMITTED.

- MIX DESIGN REQUIREMENTS: (NORMAL WEIGHT CONCRETE)
- STRENGTH f'c W/C RATIO W/C RATIO UNIT WEIGHT (NON-AIR ENTRAINED) (28 DAYS) (AIR ENTRAINED) (NORMAL WEIGHT) 5000 PSI 0.55
- CHANGES TO THE MIX DESIGN MUST BE APPROVED BY THE ENGINEER OR ARCHITECT OF RECORD AND DSA AGGREGATES SHALL CONFORM TO ASTM C33 WITH PROVEN SHRINKAGE CHARACTERISTICS OF LESS THAN .005.
- CEMENT SHALL CONFORM TO ASTM C150 (TYPE V) UNLESS NOTED OTHERWISE ON THE DRAWINGS. CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF FIVE DAYS AFTER PLACEMENT.

ALTERNATE METHODS WILL BE APPROVED IF SATISFACTORY PERFORMANCE CAN BE ASSURED.

- CONCRETE SHALL NOT FREE FALL MORE THAN FIVE FEET.
- CONCRETE SHALL BE PROPORTIONED PER CBC ACI 318-11 5.2. CONCRETE SHALL BE TESTED PER CBC 1905A.1.2, 1913A.1, 1705A.3, AND ACI 318-11 5.6.

X LOAD SCENARIO 3

- LOAD SCENARIO 4

REINFORCING STEEL:

GR 40: (#3 BARS)

- REINFORCING STEEL SHALL BE DEFORMED STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A615, (DEFORMATIONS SHALL BE IN ACCORDANCE WITH ASTM A305) AS FOLLOWS: GR 60: (#4 BARS AND LARGER)
- DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS SHALL CONFORM THE ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCING CONCRETE STRUCTURES."
- MIN. COVER FOR CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS: . CAST AGAINST FORM BELOW GRADE
 - FORMED SLABS (#11 BAR & SMALLER)... SLABS ON GRADE (FROM TOP OF SLAB)... COLUMNS AND BEAMS (MAIN BARS)... F. WALLS EXPOSED TO WEATHER (#6-#18 BARS)....... (#5 & SMALLER). G. NOT EXPOSED TO WEATHER (#11 & SMALLER)...... 3/4"
- BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIAL LIKELY TO IMPAIR BOND. BENDS SHALL BE MADE COLD.
- REINFORCING SHALL BE LAP SPLICED 45 BAR DIA. MINIMUM IN CONCRETE AND MUST COMPLY WITH ACI 318-11. PRIOR TO PLACING OF CONCRETE, REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE WELL SECURED IN POSITION.
- IDENTIFICATION STAMP WELDING OF REINFORCING IS NOT ALLOWED DIV. OF THE STATE ARCHITEC REINFORCING STEEL SHALL BE SAMPLED AND TESTED PER CBC 1913A.2.

POWDER COATED AND EPOXY PRIMED FINISH:

- ENTIRE POWDER COATING PROCESS COMPLETED IN SAME FACILITY AS STEEL FABRICATION.
- ALL CARBON STEEL MEMBERS (COLUMNS, BEAMS, PLATES, ETC.) PAINTED WITH PRIME COAT PER THE "AISC CODE OF STANDARD PRACTICE" AND THE "AISC SPECIFICATION SECTION M3" (UNLESS NOTED OTHERWISE). PARTS PRETREATED IN A 3 STAGE IRON PHOSPHATE WASHER (OR EQUAL).
- EPOXY PRIMER POWDER COAT APPLIED TO PARTS FOR SUPERIOR CORROSION PROTECTION.
- TOP POWDER COAT OF SUPER DURABLE TGIC (COLOR SELECTED FROM MANUFACTURER'S STANDARD OPTIONS OR
- SAMPLE PRODUCTION PARTS TESTED TO MEET THE FOLLOWING CRITERIA: A. SALT SPRAY RESISTANCE PER ASTM B 117/ ASTM D 1654 10000 HOURS WITH NO CREEP FROM SCRIBE LINE AND RATING OF 10
- B. HUMIDITY RESISTANCE PER ASTM D2247-02 5000 HOURS WITH NO LOSS OF ADHESION OR BLISTERING
- C. COLOR/UV RESISTANCE PER ASTM G154-04 2000 HOURS EXPOSURE ALTERNATE CYCLES WITH NO CHALKING, 75% COLOR RETENTION, AND
- COLOR VARIATION MAXIMUM 3.0 E VARIATION CIE FORMULA (BEFORE AND AFTER 2000 HOURS

ABBREVIATIONS

ACI	AMERICAN CONCRETE INSTITUTE	MR	MULIT-RIB ROOF PANEL (MCELROY)
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	NTS	NOT TO SCALE
ASM	ASSEMBLY (INTERNAL REFERENCE)	NO	NUMBER
ASTM	AMERICAN SOCIETY FOR TESTING AND MAT'LS	oc	ON CENTER
AWS	AMERICAN WELDING SOCIETY	OSHA	OCCUPATIONAL HEALTH AND SAFETY ADM.
CBC	CALIFORNIA BUILDING CODE	PCF	POUNDS PER CUBIC FOOT
CJP	COMPLETE JOINT PENETRATION	PD	POLIGON DRAWING
CLR	CLEAR	PJ	PRETENSIONED JOINT
DEG	DEGREE	PLCS	PLACES
DIA	DIAMETER	PLT	PLATE
DIM	DIMENSION	PSF	POUNDS PER SQUARE FOOT
DSA	DIVISION OF THE STATE ARCHITECT	PSI	POUNDS PER SQUARE INCH
EQ	EQUAL	QTY	QUANTITY
FT	FEET	REF	REFERENCE
GA	GAGE	SQ	SQUARE
IN	INCHES	SS	STANDING SEAM ROOF PANEL (MCELROY)
KSI	KIPS PER SQUARE INCH	TYP	TYPICAL
MAX	MAXIMUM	UNO	UNLESS NOTED OTHERWISE
MIN	MINIMUM	USGS	U.S. GEOLOGICAL SURVEY
MISC	MISCELLANEOUS	W/	WITH
MPH	MILES PER HOUR		

Δ_

INSTRUCTIONS FOR ARCHITECTS SUBMITTING THESE PRE-CHECKED DRAWING TO DSA:

BEFORE SUBMITTING THESE PRE-CHECKED DRAWINGS FOR YOUR PROJECT, FOLLOW THE STEPS BELOW TO PROPERLY DEFINE THE APPROVED OPTIONS: THE POLIGON ENGINEERING DEPARTMENT IS AVAILABLE TO HELP YOU COMPLETE THESE STEPS (616-399-1963).

STEP 1: SELECT FRAME DIMENSIONS FOR YOUR PROJECT

DESIGN VALUES

VB

A3

VERY HIGH

NOT BY POLIGON

- STRUCTURES UP TO 8' WIDE USE THE "CWC 8" BASE FRAME
- STRUCTURES UP TO 10' WIDE USE THE "CWC 10" BASE FRAME STRUCTURES UP TO 14' WIDE USE THE "CWC 14" BASE FRAME
- THE 8', 10', AND 14' WIDTHS ARE SUGGESTED BECAUSE THEY ARE THE MOST ECONOMICAL - MAXIMUM WIDTH IS 14' (SEE 'ARCHITECTURAL VIEWS' SHEET FOR REFERENCE)
- THE 44', 64', AND 84' LENGTHS ARE SUGGESTED BECAUSE THEY ARE THE MOST COMMON (20' BAYS ARE MOST ECONOMICAL) - FRAME LENGTHS ASSUME 2' OVERHANGS (UNO BY ARCHITECT - 2' MAX DIMENSION)

STEP 2: SELECT ROOF DECK FOR YOUR PROJECT - "MR" REPRESENTS MCELROY METAL "MULTI-RIB" ROOF DECK

- "SS" REPRESENTS MCELROY METAL "MEDALLION-LOK" 16" STANDING SEAM ROOF DECK
- STEP 3: IDENTIFY THE SS ACCELERATION (g) FOR YOUR PROJECT - SS VALUE DETERMINES THE REQUIRED SEISMIC DESIGN FORCES
- SS VALUE DEPENDS ON THE PROJECT'S GEOGRAPHICAL LOCATION (VALUES RANGE FROM 0.00 TO 3.73) - FIND Ss VALUES FOR YOUR PROJECT ON THE USGS WEBSITE (SEARCH INTERNET FOR "USGS SEISMIC DESIGN MAPS")

STEP 4: IDENTIFY THE Ss REGION FOR YOUR PROJECT - THE REGIONS ARE DEPENDANT ON THE SS VALUE DETERMINED IN STEP 3

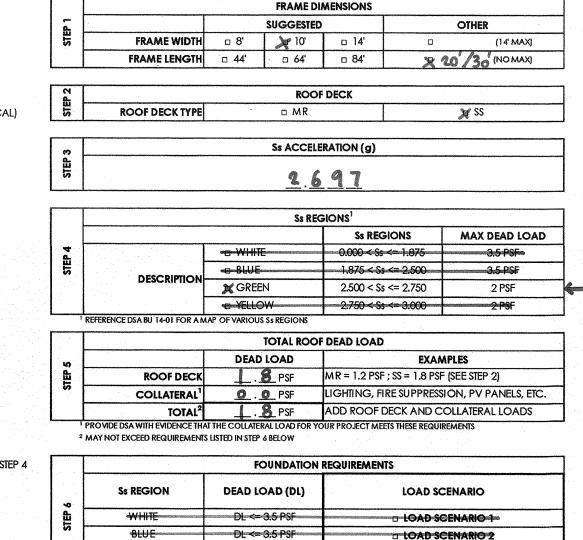
- REFERENCE DSA BU 14-01 FOR A MAP OF VARIOUS SS REGIONS - THE SS REGION DICTATES THE MAXIMUM DEAD LOAD PERMITTED ON THE FRAME (SEE TABLE TO THE RIGHT)

STEP 5: IDENTIFY THE ROOF DEAD LOAD FOR YOUR PROJECT

-THE ROOF DECK DEAD LOAD WILL ALWAYS BE INCLUDED - THE COLLATERAL LOAD REPRESENTS ADDITIONAL LOAD THAT CAN BE SUPPORTED BY THE FRAME

- BE SURE THE TOTAL ROOF DEAD LOAD FOR YOUR PROJECT IS LESS THAN OR EQUAL TO THE MAX DEAD LOAD SHOWN IN STEP 4 STEP 6: IDENTIFY THE FOUNDATION REQUIREMENTS FOR YOUR PROJECT

- REFERENCE THE SS REGION (STEP 4) AND THE TOTAL ROOF DEAD (STEP 5) - IDENTIFY A SINGLE LOAD SCENARIO - E.G. A PROJECT IN THE BLUE SS REGION WITH A 2 PSF ROOF DEAD LOAD IS LOAD SCENARIO 2
- LOAD SCENARIOS HAVE NO IMPACT ON FRAME DESIGN OR COST (BUT DO AFFECT FOUNDATION SIZE) - POLIGON CAN REVIEW THE SITE-SPECIFIC SOILS REPORT TO EVALUATE THE POSSIBILITY OF SMALLER FOUNDATIONS



DL <= 2.0 PSF

YELLOW DL <= 2.0 PSF

STEP 8: SELECT APPLICABLE SHEET INDEX FOR YOUR PROJECT - REFERENCE THE BASE FRAME (STEP 1) AND THE ROOF DECK TYPE (STEP 2) - IDENTIFY THE APPLICABLE SHEET INDEX STEP 9: INCLUDE APPLICABLE SHEETS WITH YOUR DSA SUBMITTAL - EXCLUDE 'MISC DESIGN OPTIONS' SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUTS OR GUTTERS STEP 10: IDENTIFY PROJECT NAME AND SCHOOL DISTRICT DUNSMORE ELEMENTARY SCHOOL KINDER SHAPE SHRUCTURES GLENPALE UNIFIED SCHOOL DISTRICT

STEP 7: SELECT MISCELLANEOUS OPTIONS FOR YOUR PROJECT

- MAXIMUM CLEAR HEIGHT IS 12-0"; (SEE 'ARCHITECTURAL VIEWS' SHEET FOR REFERENCE)

- MARK UP PC DRAWINGS WITH SIZE AND LOCATION OF CUTOUTS BEFORE SUBMITTING TO DSA

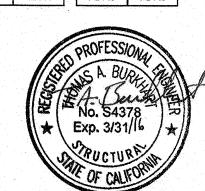
SHEET INDEX CWC 10 CWC 14 MR SS MR SS PD1.0 PD1.0 PD1.0 PD1.0 PD1.0 PD1.0 GENERAL NOTES PD1.1 PD1.1 PD1.1 PD1.1 PD1.1 PD1.1 PD3.1 FRAMING PLAN PD4.0 PD4.0 PD4.2 PD4.2 PD4.1 PD4.1 FRAME CONNECTION DETAILS NOTUSED NOTUSED SERIES PD5.X NOTUSED PD6.1 PD6.1 PD6.2 PD6.2 ARCHITECTURAL VIEWS PD7.0 PD7.0 PD7.1 PD7.1 PD7.2 PD7.2 PD8.0 PD8.1 PD8.0 PD8.1 PD8.0 PD8.1 ROOF CONNECTION DETAILS PD9.0 PD9.0 PD9.0 PD9.0 PD9.0 PD9.0 MISC DESIGN OPTIONS

CLEAR HEIGH

ELECTRICAL CUTOUT

MISCELLANEOUS

DESIGN OPTIONS



(12' MAX)

By geotechnical engineer or his or her qualified representative.	special inspections or providing ital inspections not phragms, cold-phragms, cold-prize and information of the 2007 and information of the California Build and inform	roperly prior to placement of controlled foundations, re extended to proper depth and have and are adequate to achieve the design ACE DEEP FOUNDATIONS (PI and maintain complete and accurate lumbness, bell diameters (if embedment into bedrock (if applicable). volumes.
d special inspections. An "X" before a listed test or inspection indicates by requirement. A shaded box indicates a test or special inspection the copy requirement. A shaded box indicates a test or special inspection the depending on the scope of the construction and other issues. A shad indicating your selection of that test. Note: A minus (-) on a category indicates that it can be collapsed. However, any selections you may he click on the "COMPILE" button to show only the tests finally selected too on use of this form, see DSA-103.INSTR. Iding Code (CBC) unless otherwise noted. CODE REFERENCE AND NOTES By geotechnical engineer or his or her qualified representative. By geotechnical engineer or his or her qualified representative. By geotechnical engineer or his or her qualified representative. per CONCRETE section below. To be performed by batch-plant special inspector and project inspector. 913A.2 (1913.2.6*). ASTM A370. DSA IR 17-10 ISTM C172, ASTM C31. ICI 318 Section 5.6 and 1905A.1.2 (1913.3.1*). ASTM C39.	remed as detailed for providing mandator required, clicked in required,	tests and inspections must be performed. The project inspector is responsible for project, including but not limited to, special insial wood framing, high-load wood diaphraginon-structural components, etc., per Title for projects submitted for review under the Note: References are to the 2013 ed PECTION Toperly prior to placement of controlled foundations, reextended to proper depth and have and are adequate to achieve the design ACE DEEP FOUNDATIONS (Pland maintain complete and accurate lumbness, bell diameters (if embedment into bedrock (if applicable), volumes.
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	Continuous Pi* * TMS 402-11/ACI 530-11/ASC Table 1705A.2.1	on).
May be performed by a special inspector when specifically approved by DSA.	TMS 402-11/ACI 530-11/ASCI Table 1705A.2.1	
	Table 1705A.2.1	ect curing and form removal.
E 5-11 Table 1.19.3		
OR STRUCTURAL PURPOSES		AL STEEL AND COLD-FORME
By special inspector when performed off-site; by project inspector for steel shi roject site without welding or fabrication.		e appropriately marked and that: e material properties that comply with and grades comply with requirements.
203A.1 (2203.1*). ASTM A370. DSA IR 17-3.		s tructural tubes and pipes
		bracing and all details constructed in
	Continuous	connection tab locations and all
	Continuous PI	nated in the shop. NGTH BOLTS:
•	Continuous PI Periodic SI	
	Periodic SI	of High-Strength Bolts, Nuts, and Was
OSA IR 17-9	Periodic SI	of High-Strength Bolts, Nuts, and Was ngs and manufacturer's certificates of STM standards specified in the DSA
DSA IR 17-9 213A.1 (2212.6.1 ⁺). ASTM F606, A370. DSA IR 17-8	Periodic SI d Washers: Periodic SI Di	of High-Strength Bolts, Nuts, and Was
213A.1 (2212.6.1*). ASTM F606, A370. DSA IR 17-8 "Continuous" or "Periodic" depends on the tightening method used, DSA IR 17	Periodic SI d Washers: Periodic SI Di Test Lab 22	of High-Strength Bolts, Nuts, and Was ngs and manufacturer's certificates of STM standards specified in the DSA nuts and washers.
213A.1 (2212.6.1 ⁺). ASTM F606, A370. DSA IR 17-8	Periodic SI d Washers: Periodic SI Di Test Lab 22	of High-Strength Bolts, Nuts, and Was ngs and manufacturer's certificates of STM standards specified in the DSA nuts and washers.
213A.1 (2212.6.1*). ASTM F606, A370. DSA IR 17-8 "Continuous" or "Periodic" depends on the tightening method used, DSA IR 17	Periodic SI d Washers: Periodic SI Di Test Lab 22 * SI **	of High-Strength Bolts, Nuts, and Was ngs and manufacturer's certificates of STM standards specified in the DSA nuts and washers. Strength Bolt Installation: rials, Equipment, Welders, etc: dentification markings per AWS DSA approved documents and the WPS.
213A.1 (2212.6.1*). ASTM F606, A370. DSA IR 17-8 "Continuous" or "Periodic" depends on the tightening method used, DSA IR 17- SA IR 17-3, AWS D1.1 and AWS D1.8 (AWS D1.3 for cold formed steel).	Periodic SI d Washers: Periodic SI Di Test Lab 22 * SI ** Periodic SI Periodic SI Periodic SI	of High-Strength Bolts, Nuts, and Was ngs and manufacturer's certificates of STM standards specified in the DSA nuts and washers. Strength Bolt Installation: rials, Equipment, Welders, etc: dentification markings per AWS DSA approved documents and the WPS. manufacturer's certificate of
213A.1 (2212.6.1*). ASTM F606, A370. DSA IR 17-8 "Continuous" or "Periodic" depends on the tightening method used, DSA IR 17	Periodic SI d Washers: Periodic SI Di Test Lab 22 * SI ** Periodic SI Periodic SI Periodic SI	of High-Strength Bolts, Nuts, and Was ngs and manufacturer's certificates of STM standards specified in the DSA nuts and washers. Strength Bolt Installation: rials, Equipment, Welders, etc: dentification markings per AWS DSA approved documents and the WPS. manufacturer's certificate of ications and equipment.
213A.1 (2212.6.1*). ASTM F606, A370. DSA IR 17-8 "Continuous" or "Periodic" depends on the tightening method used, DSA IR 17- ISA IR 17-3, AWS D1.1 and AWS D1.8 (AWS D1.3 for cold formed steel). ISA IR 17-3. Per AISC 360 (and AISC 341 as applicable). DSA IR 17-3.	Periodic SI d Washers: Periodic SI Di Test Lab 22 * SI ** Periodic SI	of High-Strength Bolts, Nuts, and Was ngs and manufacturer's certificates of STM standards specified in the DSA nuts and washers. Strength Bolt Installation: rials, Equipment, Welders, etc: dentification markings per AWS DSA approved documents and the WPS. manufacturer's certificate of ications and equipment. ELDING: s, and fillet welds > 5/16"
213A.1 (2212.6.1*). ASTM F606, A370. DSA IR 17-8 "Continuous" or "Periodic" depends on the tightening method used, DSA IR 17- OSA IR 17-3, AWS D1.1 and AWS D1.8 (AWS D1.3 for cold formed steel).	Periodic SI d Washers: Periodic SI Di Test Lab 22 * SI ** Periodic SI	of High-Strength Bolts, Nuts, and Was ngs and manufacturer's certificates of STM standards specified in the DSA nuts and washers. Strength Bolt Installation: rials, Equipment, Welders, etc: dentification markings per AWS DSA approved documents and the WPS. manufacturer's certificate of ications and equipment. ELDING:
OR STRUCTURAL PURPOSES By special inspector when performed off-site; by project inspector roject site without welding or fabrication. 203A.1 (2203.1*). ASTM A370.	Test Lab 2:	TAL STEEL AND COLD-FORME: The appropriately marked and that: The material properties that comply with and grades comply with requirements. The interior of the properties and pipes The properties an

1. THE PROJECT INSPECTOR AND TESTING AGENCY SHALL BE SELECTED BY THE SCHOOL DISTRICT AND APPROVED BY DSA AND THE ARCHITECT OF RECORD.

2. COSTS OF THE PROJECT INSPECTOR AND THE TESTING AGENCY SHALL BE BORN BY THE SCHOOL DISTRICT.

SPECIAL INSPECTION:

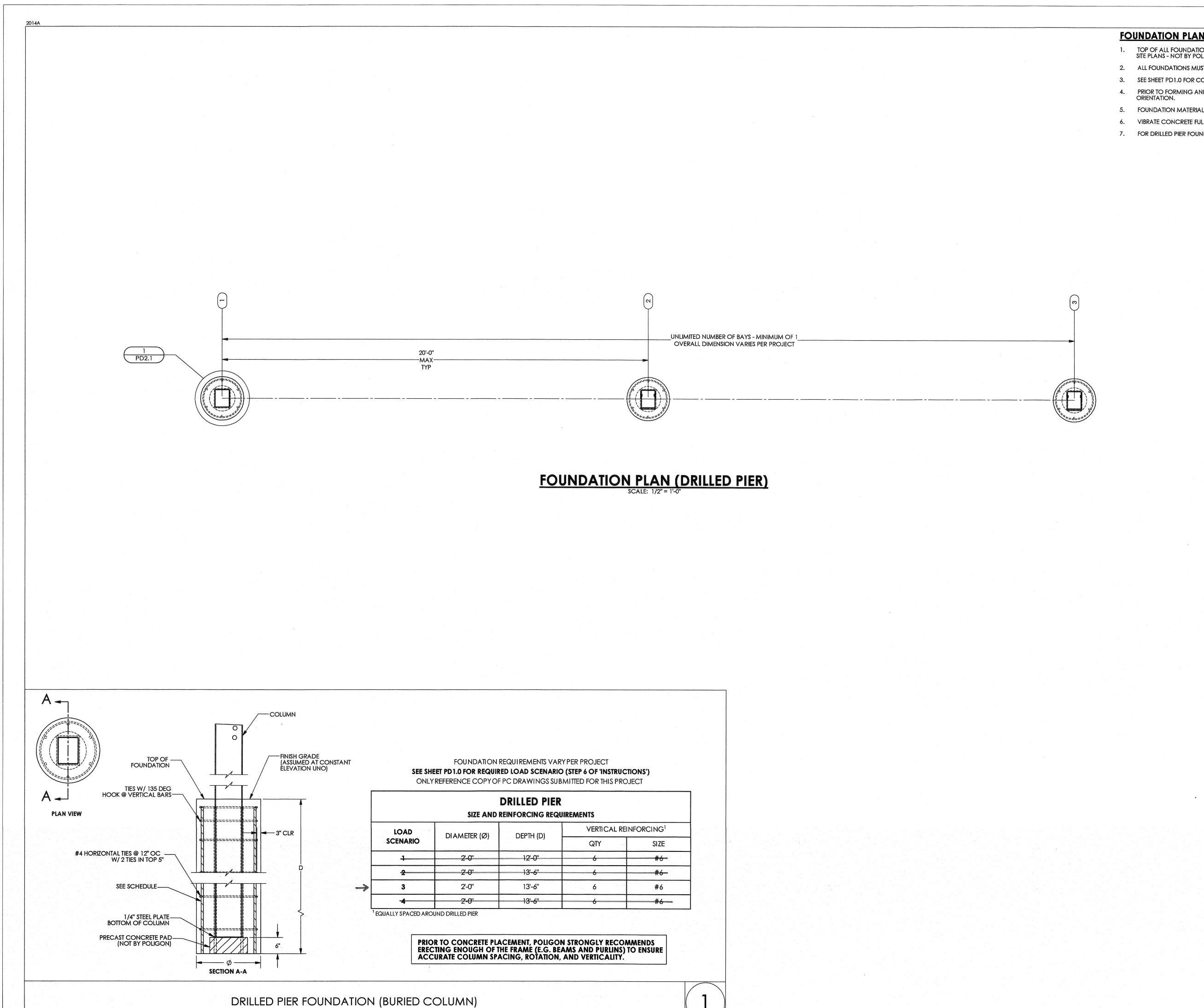
IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT

10 03-119278

AC / FLS / SS V C
Date AUG 29 2018

PRE-CHECK (PC) DOCUMENT

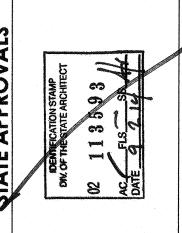
ECIAL INSPECTIONS SPI



FOUNDATION PLAN NOTES:

- 1. TOP OF ALL FOUNDATIONS MUST BE CONSTRUCTED AT ONE COMMON ELEVATION (COORDINATE WITH SITE PLANS NOT BY POLIGON)
- 2. ALL FOUNDATIONS MUST BE CENTERED UNDER COLUMNS (UNO).
- 3. SEE SHEET PD1.0 FOR CONCRETE REQUIREMENTS.
- 4. PRIOR TO FORMING AND CASTING FOUNDATIONS, REVIEW FOUNDATION PLAN FOR REQUIRED ORIENTATION.
- 5. FOUNDATION MATERIAL AND INSTALLATION NOT BY POLIGON.
- 6. VIBRATE CONCRETE FULL DEPTH OF FOUNDATION.
- 7. FOR DRILLED PIER FOUNDATIONS, PREVENT SOIL FROM ENTERING EXCAVATED HOLE (FORM, ETC).





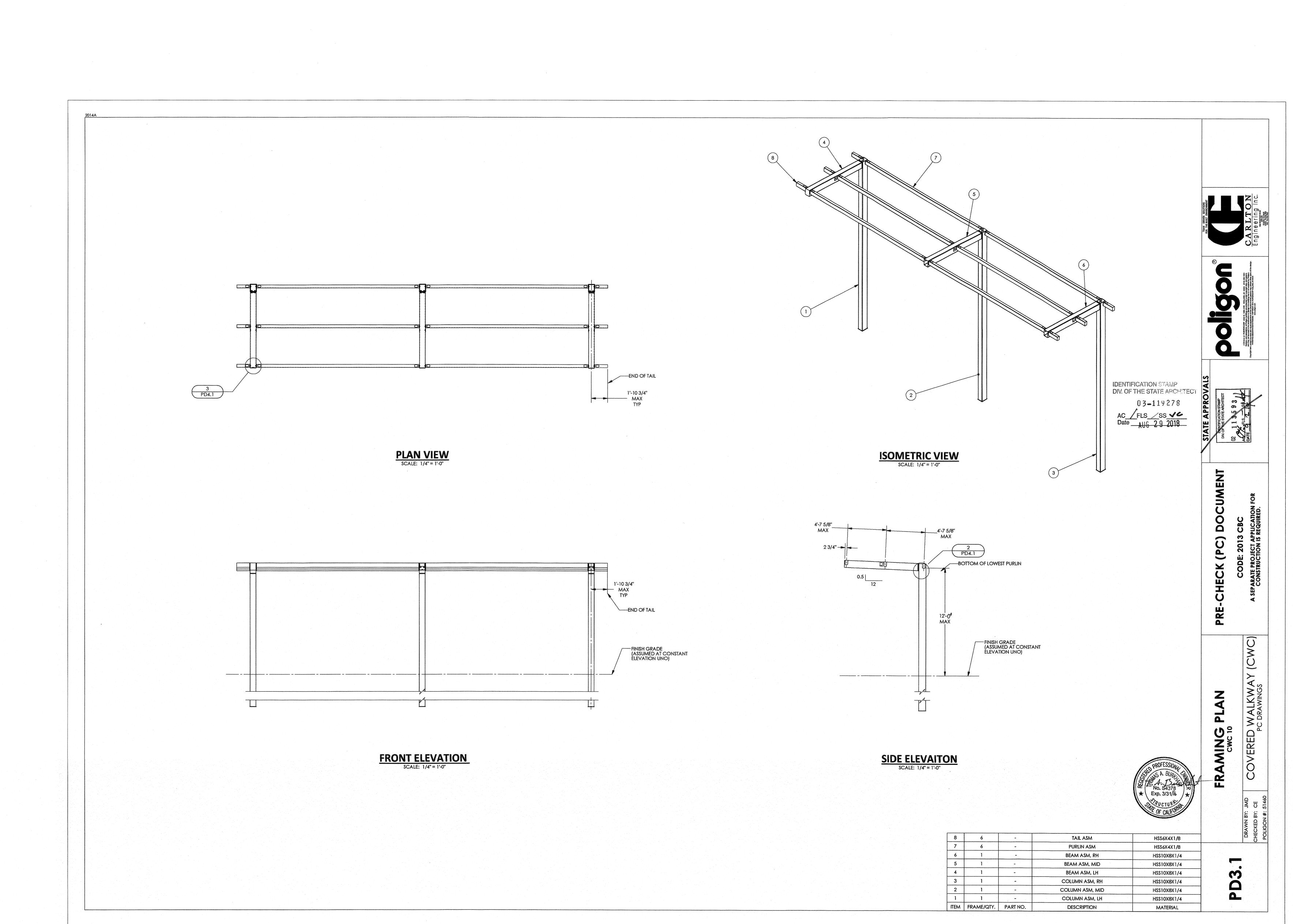
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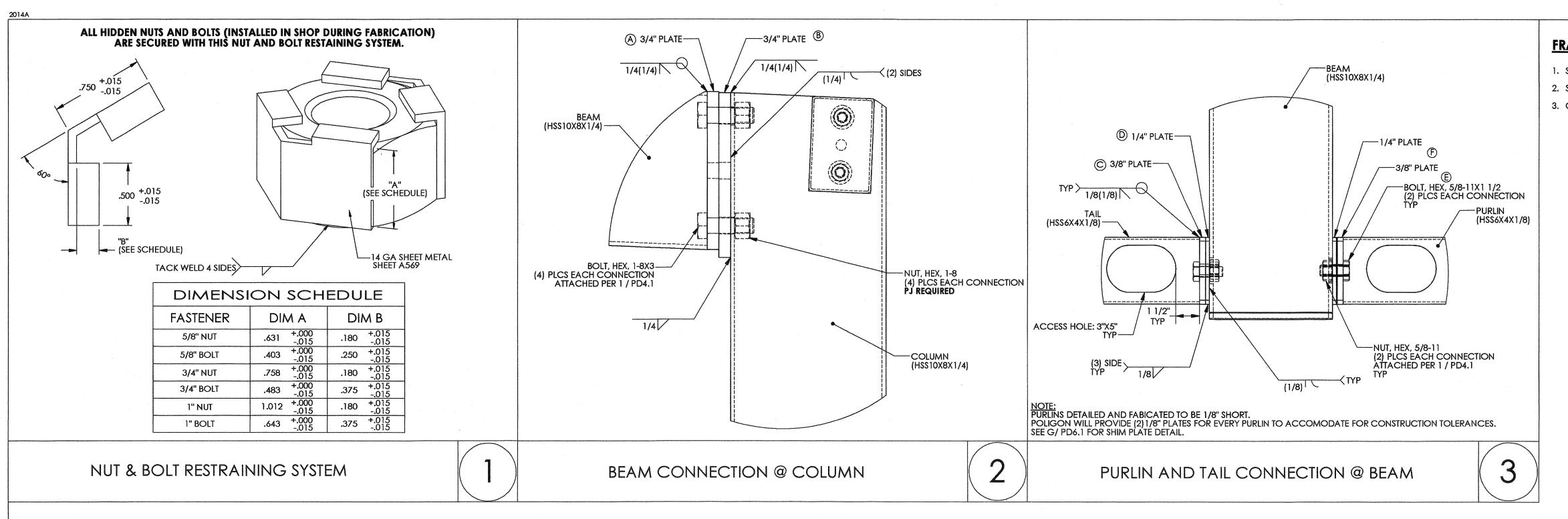
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PRE-

OVERED WALKWAY (CWC)



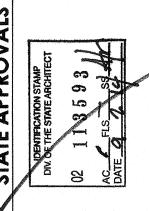


FRAME CONNECTION DETAIL NOTES:

- 1. SEE SECTIONS ON SHEET PD5.1.
- 2. SEE PLATE igotimes DETAILS ON SHEET PD6.1
- 3. COVER ACCESS HOLES WITH GRACE ICE AND WATER SHIELD BEFORE ATTACHING ROOF DECK.





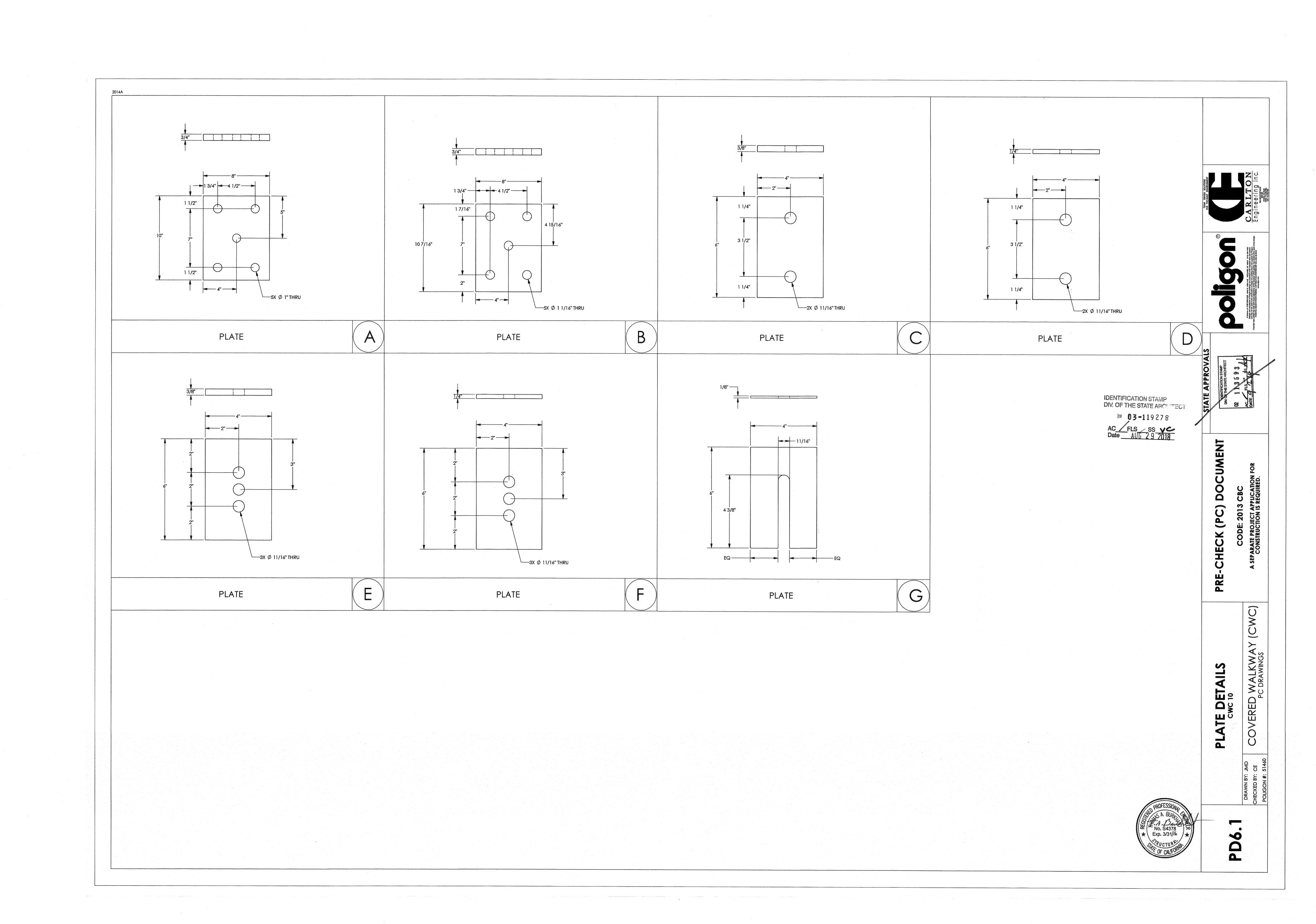


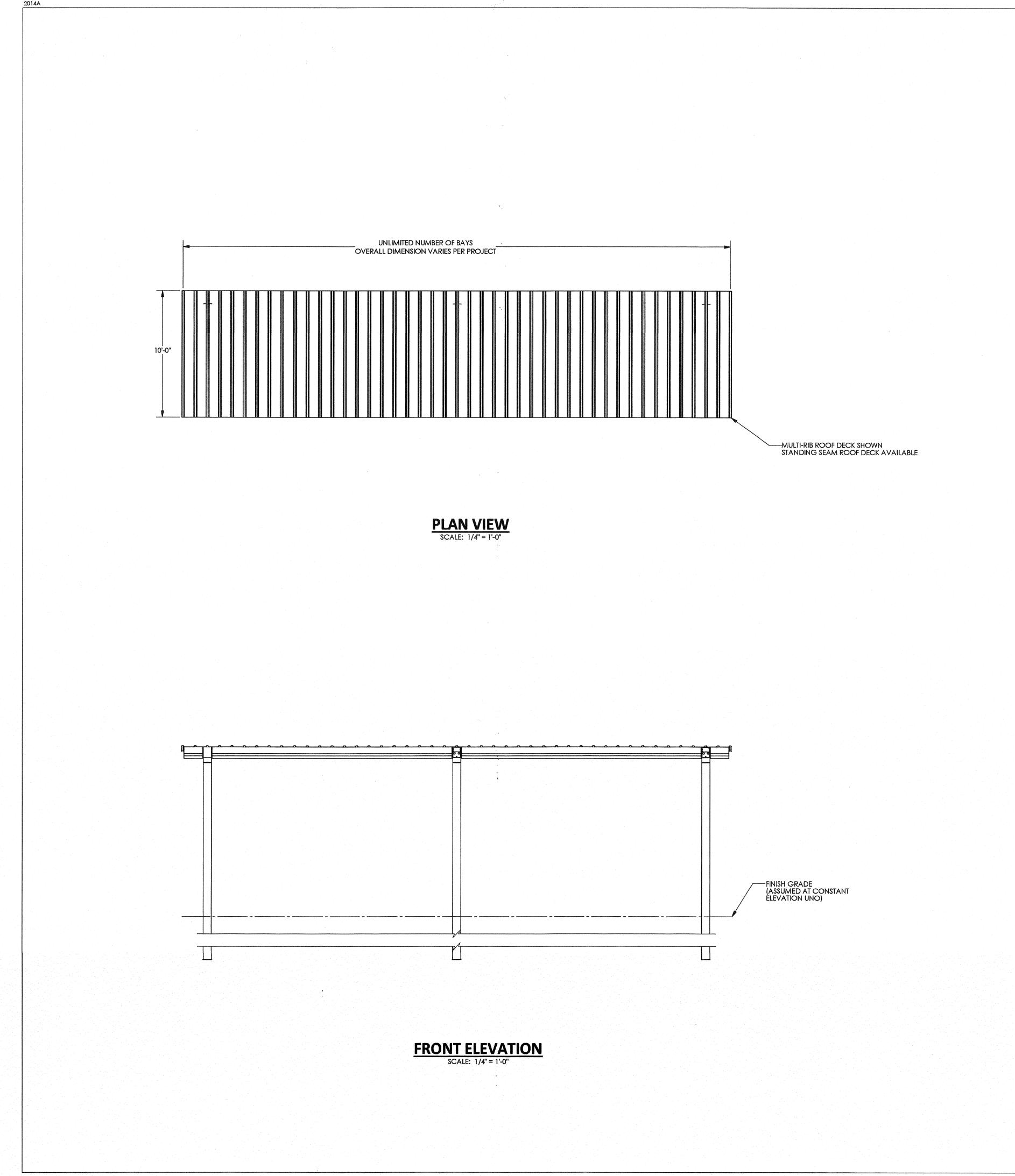
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DIV. OF THE STATE ARCHITECT

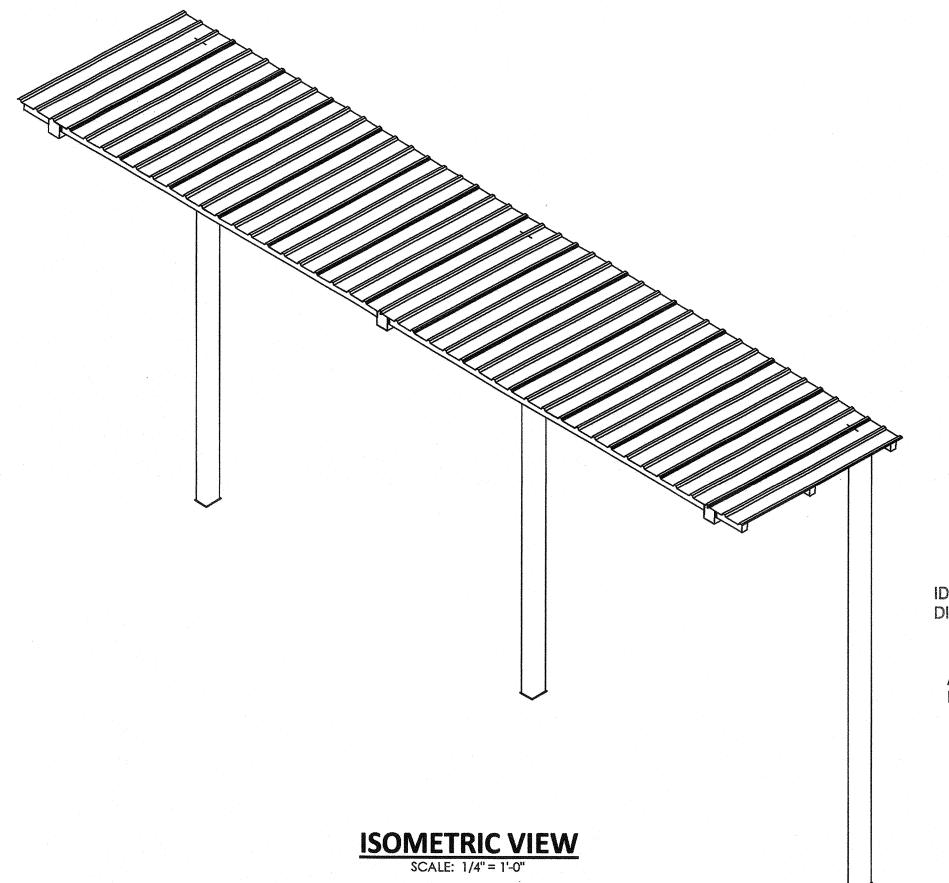
03-119278

PRE-CHECK (PC) DOCUMENT

ONNECTION DETAILS

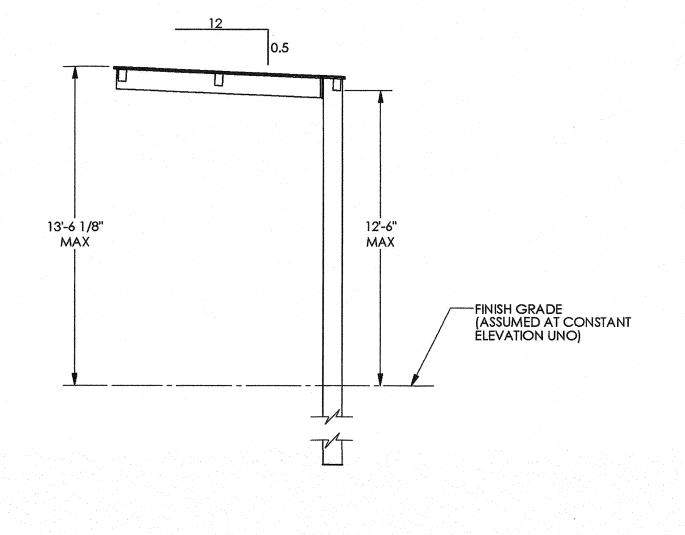






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Date AUG 29 2018

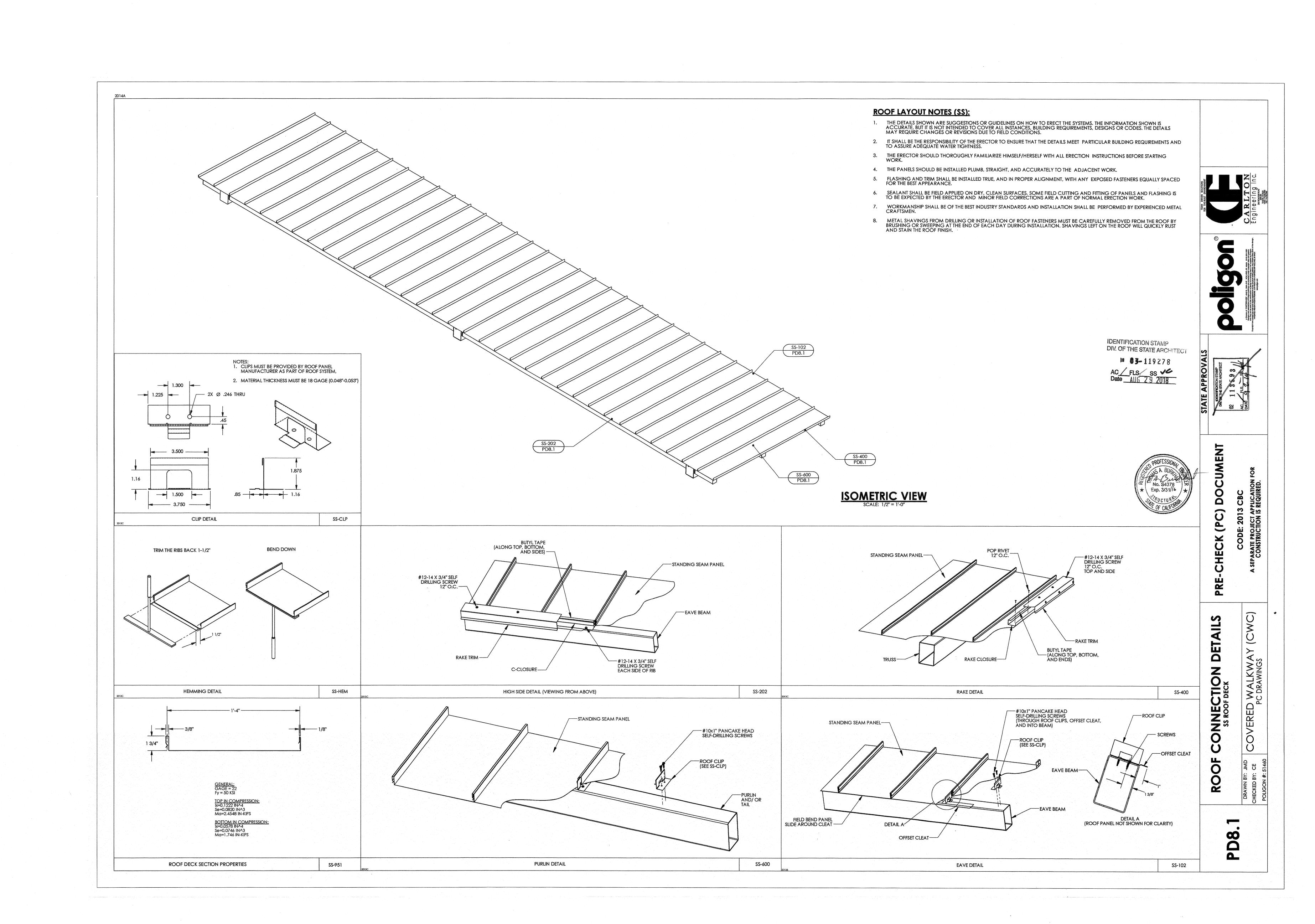


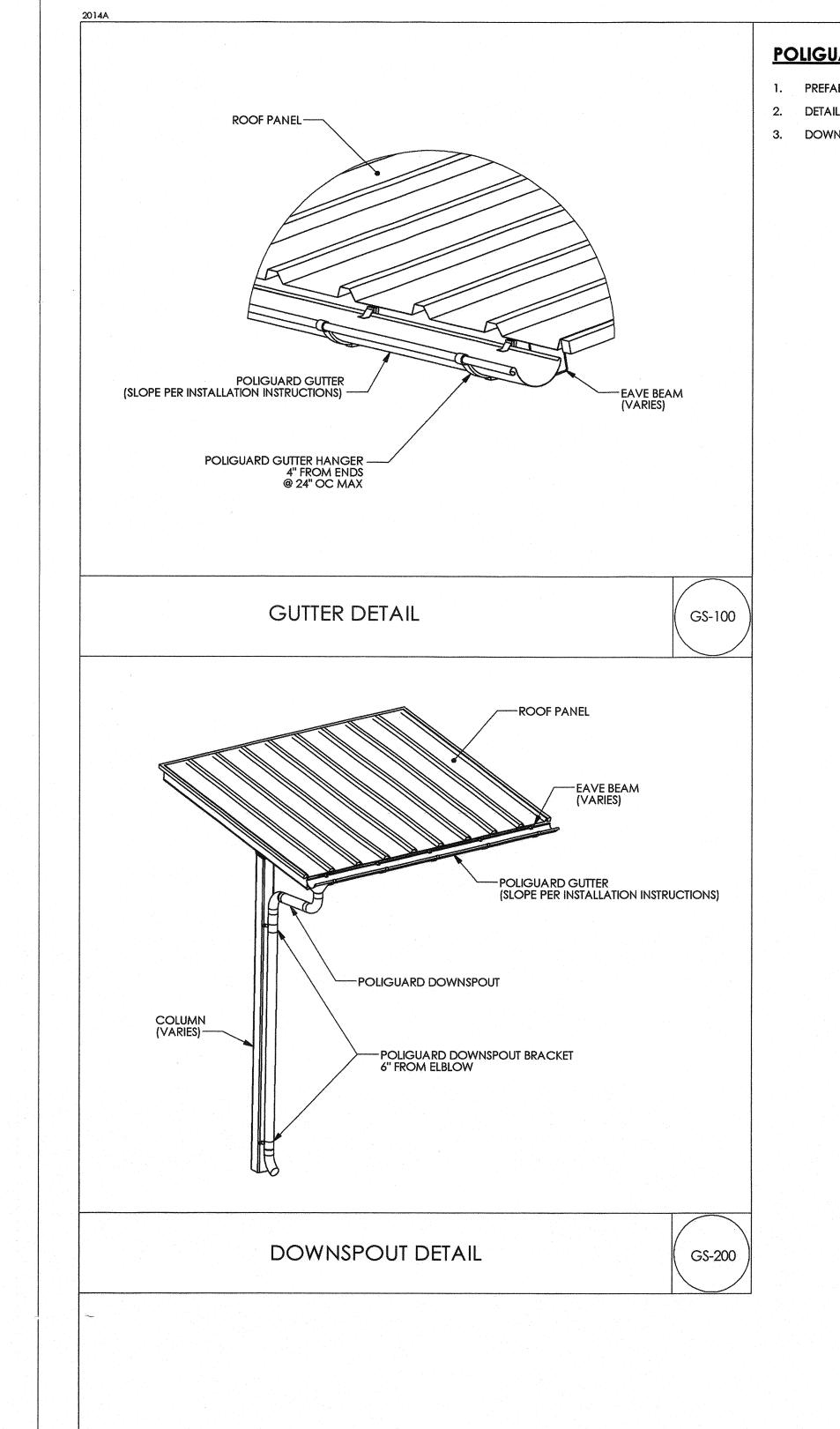
SCALE: 1/4" = 1'-0"

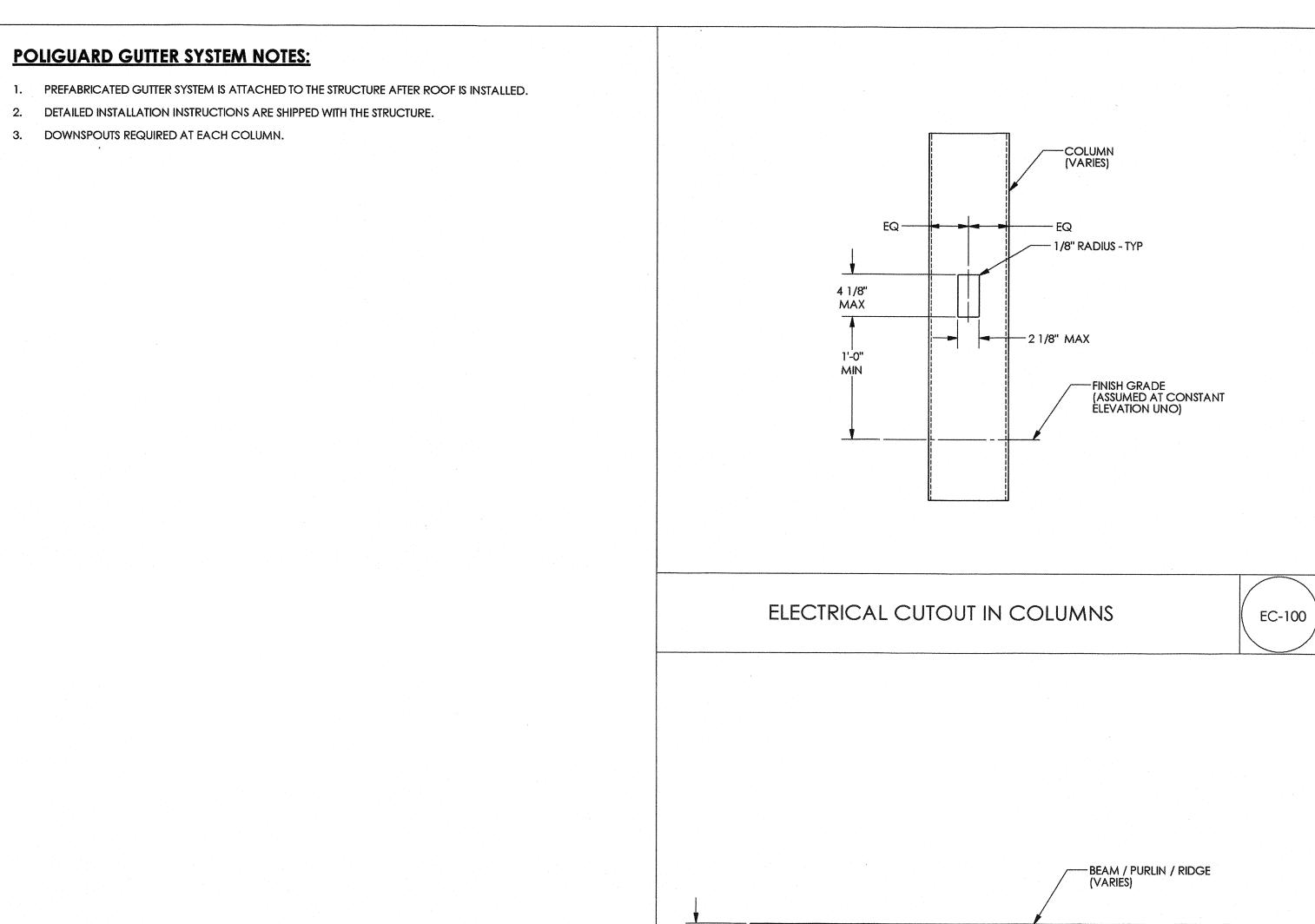


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PD7







── Ø 1 1/16"THRU MAX

EC-200

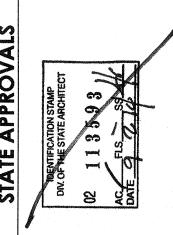
ELECTRICAL CUTOUT IN BEAMS / PURLINS / RIDGES



- 1. MAXIMUM ONE CUTOUT PERMITTED IN EACH MEMBER.
- 2. CUTOUTS CAN BE PLACED ON ANY SIDE OF A MEMBER.
- 3. CUTOUTS CAN BE PLACED ALONG MEMBERS AS INDICATED IN THE DETAILS.
- 4. ARCHITECTS REQUESTING CUTOUTS MUST MARKUP APPROVED PC DRAWINGS TO LOCATE CUTOUTS FOR APPROVAL AND FABRICATION.



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WALKWAY (CWC) C DRAWINGS DESIGN OPTIONS