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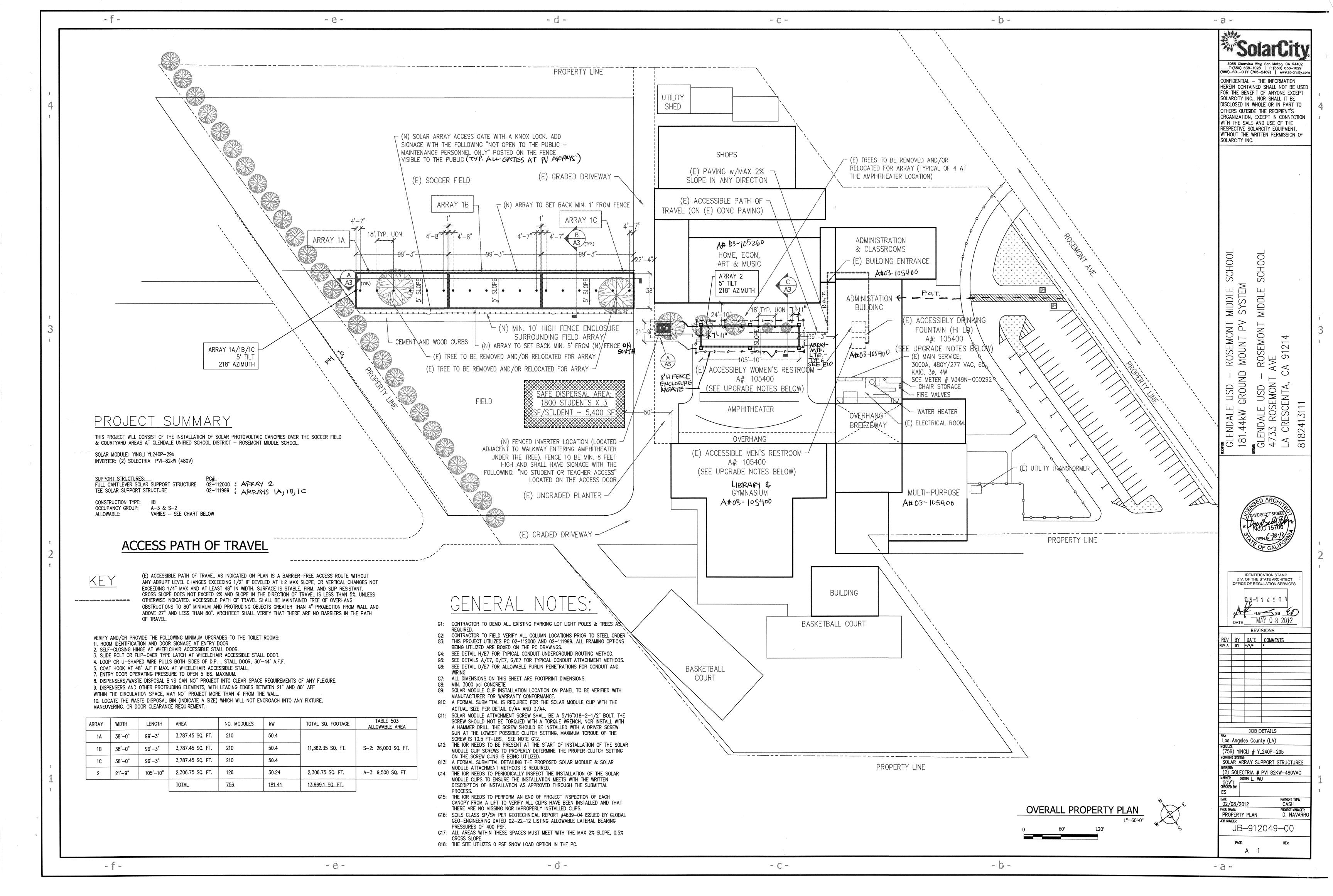
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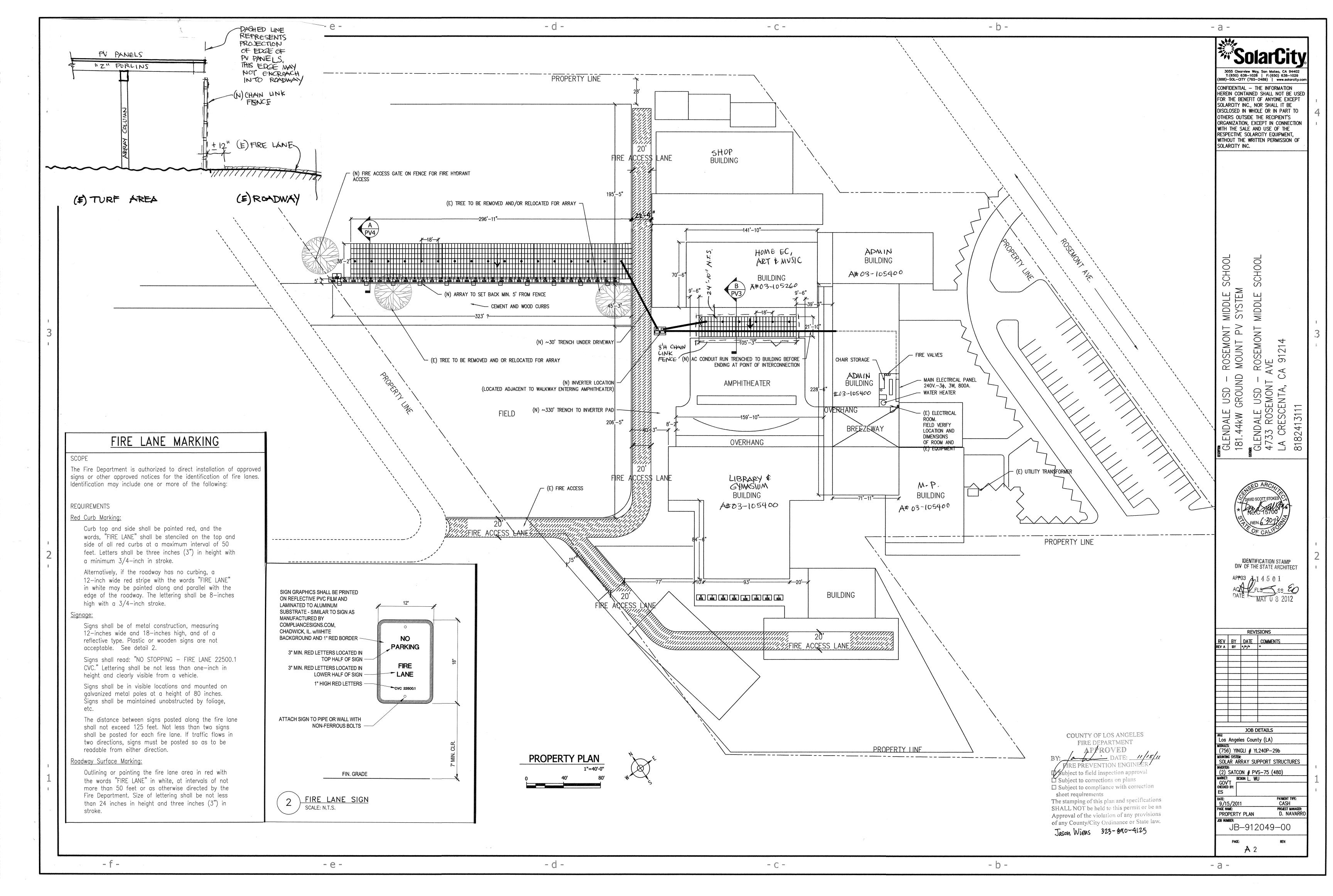
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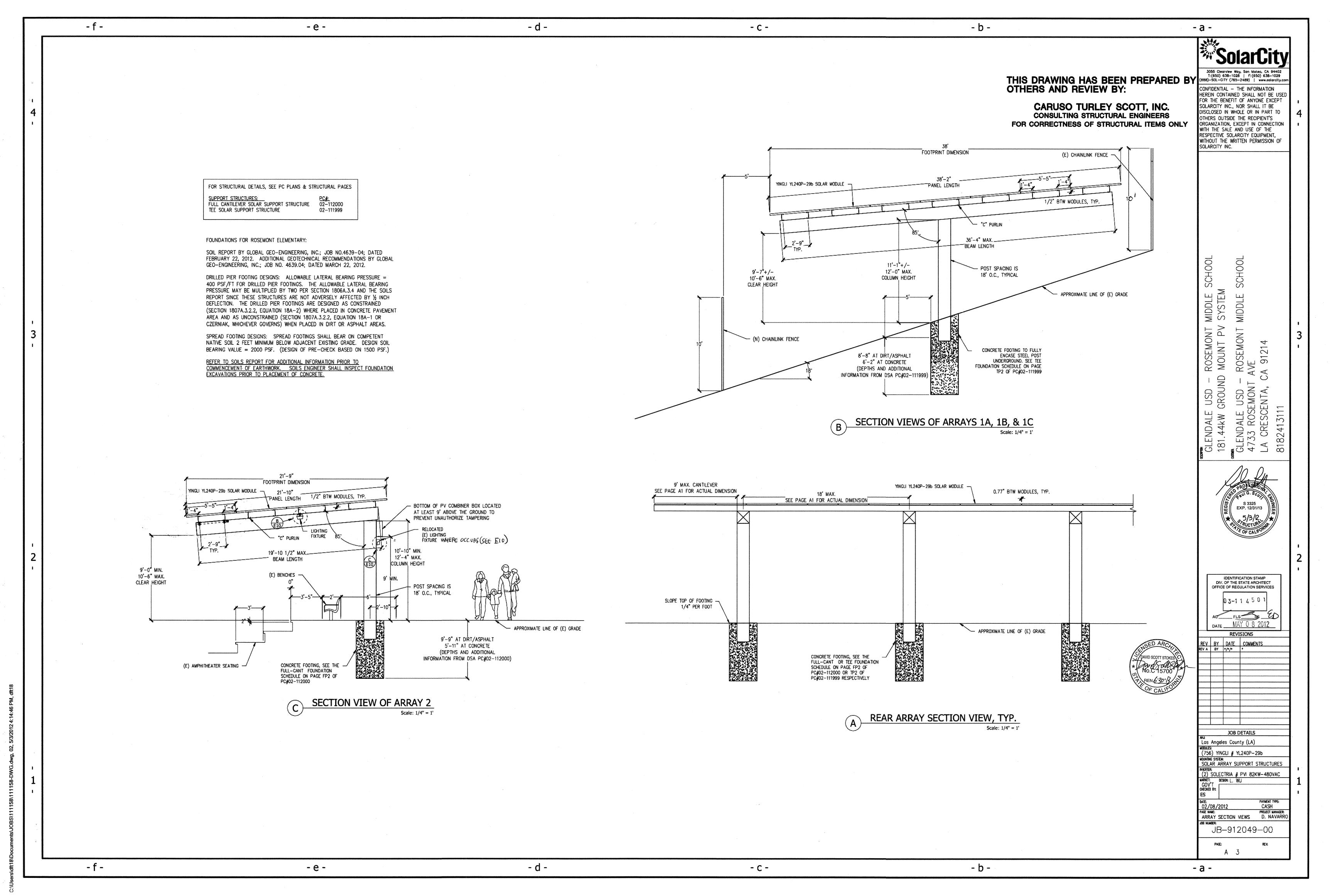
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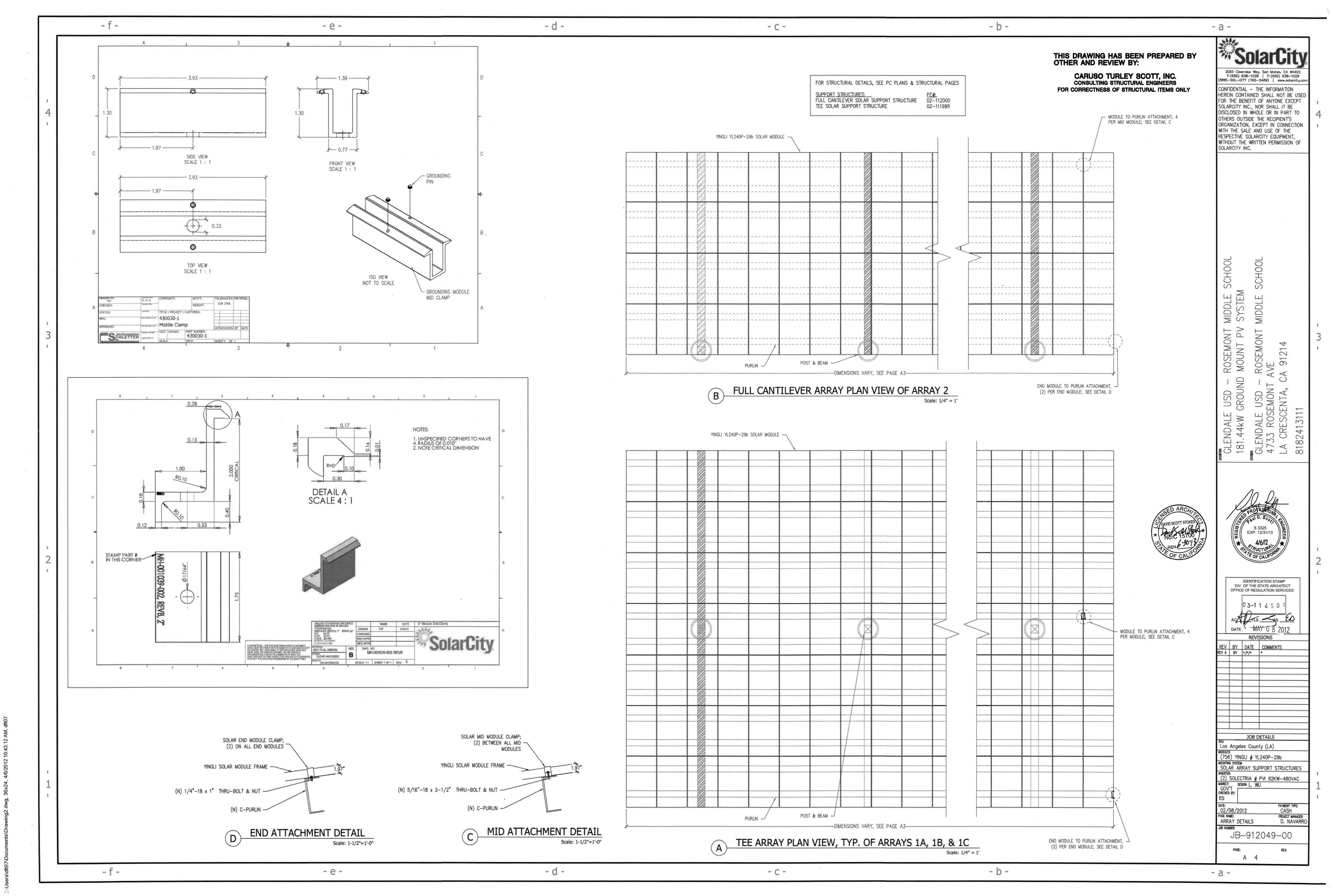
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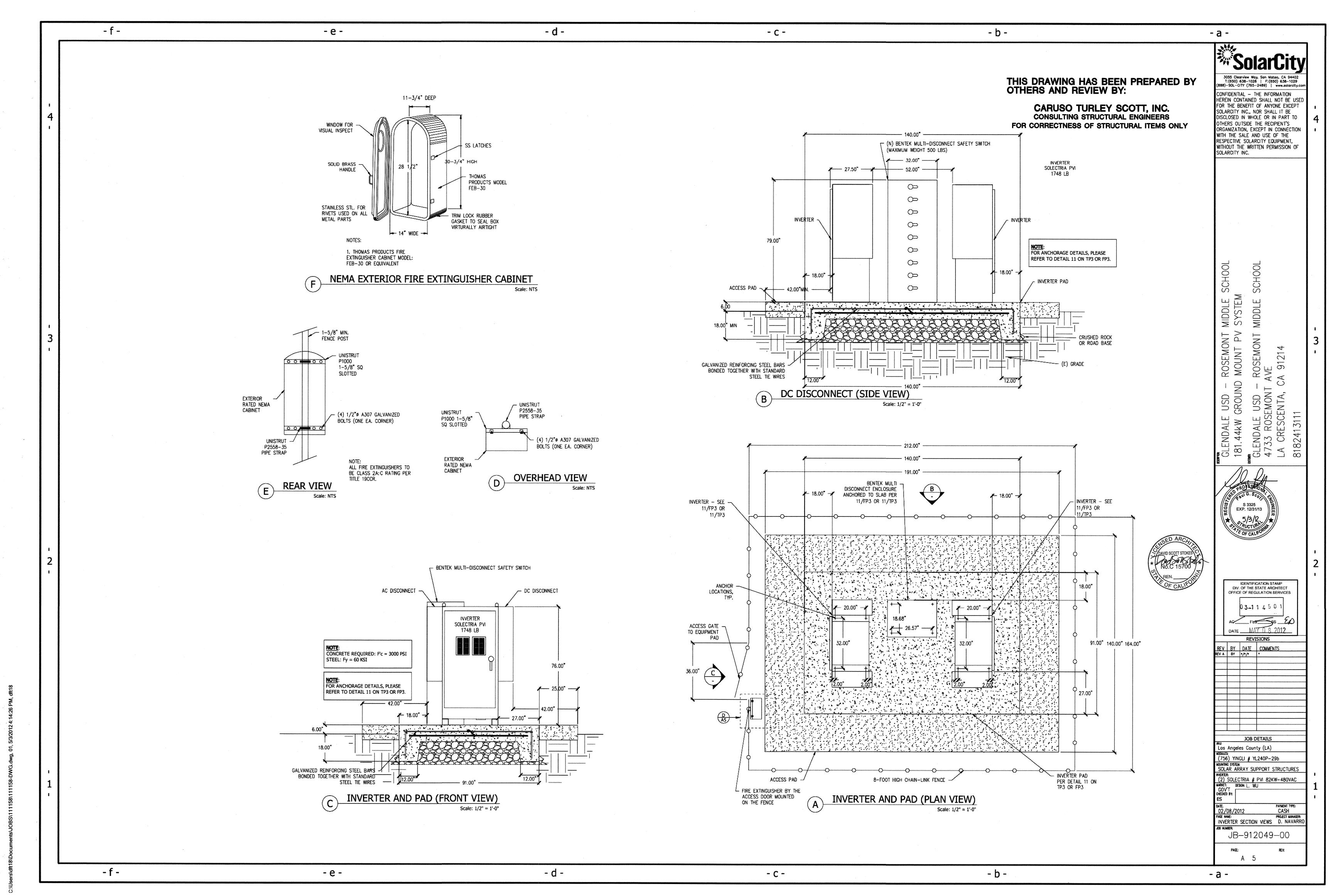
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CONFIDENTIAL - THE INFORMATION HEREIN CONTAINED SHALL NOT BE USED FOR THE BENEFIT OF ANYONE EXCEPT SOLARCITY INC., NOR SHALL IT BE DISCLOSED IN WHOLE OR IN PART TO OTHERS OUTSIDE THE RECIPIENT'S ORGANIZATION, EXCEPT IN CONNECTION WITH THE SALE AND USE OF THE RESPECTIVE SOLARCITY EQUIPMENT, WITHOUT THE WRITTEN PERMISSION OF SOLARCITY INC.

ELECTRICAL NOTES

- 1. ALL ELECTRICAL WORK SHALL COMPLY WITH THE 2008 NATIONAL ELECTRIC CODE AS AMENDED BY THE 2010 CALIFORNIA ELECTRIC CODE.
- BRANCH CIRCUIT WILL BE IDENTIFIED BY PHASE AND SYSTEM PER ART 210.5. 3. A NATIONALLY-RECOGNIZED TESTING LABORATORY SHALL UST ALL EQUIPMENT IN COMPLIANCE WITH ART

2. EACH UNGROUNDED CONDUCTOR OF THE MULTIWIRE

- 4. CIRCUITS OVER 250V TO GROUND SHALL COMPLY WITH
- ART. 250.97, 250.92(B) 5. DC CONDUCTORS INSÍDÉ BUILDING SHALL BE IN
- METALLIC RACEWAY PER ART 690.31(E). 6. ALL ABOVE GROUND CONDUIT SHALL BE EMT WITH RAINTIGHT FITTINGS, ALL CONDUIT EXPOSED TO VEHICULAR DAMAGE SHALL BE RMC. ALL BELOW
- GROUND CONDUIT SHALL BE SCHEDUCLE 40 PVC ALL WRES SHALL BE PROVIDED WITH STRAIN RELIEF AT ALL ENTRY INTO BOXES AS REQUIRED BY UL
- 8. INSTALLATION SHALL COMPLY WITH ART. 250.52,
- 250.53 9. INSTALL PARALLEL CONDUCTORS PER ART 310.4 10. ALL VALUES FOR IMP AND ISC AND VMP ARE
- MANUFACTURER'S LISTED DATA UNCORRECTED BY NEC. 11. REFER TO CURRENT MANUFACTURER'S PLANNING AND INSTALLATION MANUAL FOR TORQUE SPECS FOR ALL
- BOLTS AND TERMINAL CONNECTIONS. 12. DC STRING CIRCUITS SHALL BE RUN IN OUTDOOR
- AMBIENT CONDITIONS. 13. PV INVERTER CONTAINS INTEGRATED AC AND DC
- DISCONNECTS AND GFDI. 14. BURIED CONDUCTORS SHALL BE BURIED TO THE
- MINIMUM DEPTH SPECIFIED IN ART. 300.50. 15. ALL CONDUCTORS ARE COPPER UNLESS NOTED OTHERWISE.

- 16. SINGLE-CONDUCTOR CABLE USED AS A GROUNDED CONDUCTOR IN PHOTOVOLTAIC POWER SYSTEMS SHALL BE IDENTIFIED AT THE TIME OF INSTALLATION BY DISTINCTIVE WHITE MARKING AT ALL TERMINATIONS.
- 17. THE DC GEC SHALL BE CONTINUOUS FROM THE INVERTER GROUND BUSBAR TO THE MAIN ELECTRICAL SERVICE GROUNDING ELECTRODE SYSTEM. THE DC GEC SHALL BE ATTACHED TO THE GROUND ELECTRODE USING AN IRREVERSIBLE MEANS AS CALLED OUT IN ART. 250.64 AND 690.47.
- DO NOT BOND THE GROUNDED DC CONDUCTOR TO GROUND EXCEPT THROUGH THE INVERTER GFDI. 19. ALL EXPOSED METAL PARTS (RAIL, PIPE, BOXES, ETC) SHALL BE GROUNDED USING PROPER GROUNDED

18. PV INVERTER CONTAINS AN INTEGRATED GFDI CIRCUIT.

- METHODS APPROVED BY A NATIONALLY RECOGNIZED TESTING LABORATORY. 20. #10 BARE COPPER EGC AT SOURCE CIRCUITS SHALL BE ROUTED SECURELY TO MOUNTING HARDWARE IN A
- 21. FERROUS METAL RACEWAYS ENCLOSING GEC CONDUCTORS SHALL BE ELECTRICALLY CONTINUOUS OR BONDED IN ACCORDANCE WITH ART. 250.64(E).

MANNER THAT PROTECTS FROM PHYSICAL HARM.

- 22. MODULE FRAMES, RAIL, AND POSTS SHALL BE BONDED WITH EQUIPMENT GROUND CONDUCTORS AND GROUNDED AT THE MAIN ELECTRIC PANEL.
- 23. BOTH ENDS OF ALL METALLIC CONDUIT CONTAINING GROUNDING ELECTRODE CONDUCTORS SHALL BE
- BONDED PER ART 250.64(E). 24. GROUNDING ELECTRODE CONDUCTOR TO BE BONDED
- TO (E) UFER PER ART 250.30(A)(4)(A). 25. DC GROUNDING ELECTRODE CONDUCTOR SIZED PER ART 250.166(D).

ABBREVIATIONS

... d ...

ALTERNATING CURRENT BLDG CONC CONCRETE COMBINER BOX DISTRIBUTION PANEL DIRECT CURRENT EGC EQUIPMENT GROUNDING CONDUCTOR

ELECTRICAL METALLIC TUBING SOLAR GUARD METER GALV GALVANIZED GEC GROUNDING ELECTRODE CONDUCTOR **GFDI** GROUND FAULT DETECTION & INTERRUPTION GND HDG HOT DIPPED GALVANIZED

CURRENT CURRENT AT MAX POWER INVERTERS SHORT CIRCUIT CURRENT kVA KILOVOLT AMPERE KILOWATT

LOAD BEARING WALL LBW MIN MINIMUM NATIONAL ELECTRIC CODE NOT IN CONTRACT

NTS NOT TO SCALE ON CENTER OC. PANEL BOARD PROPERTY LINES PHOTOVOLTAIC POLYVINYL CHLORIDE PVC SUBPANEL

SCHEDULE SCH STAINLESS STEEL SS SEE STRUCTURAL DRAWINGS SSD STC STANDARD TESTING CONDITIONS SWH SOLAR WATER HEATER TYP

UNLESS OTHERWISE NOTED UON UNINTERRUPTIBLE POWER SUPPLY VOLT VOLTAGE AT MAX POWER VOLTAGE AT OPEN CIRCUIT

NEMA 3R, RAINTIGHT

LEGEND

BREAKER, 2 POLE BREAKER, 3 POLE T世 COMBINER BOX, DC

(SEE BELOW FOR MORE INFO) CURRENT TRANSFORMER DISCONNECT, FUSED

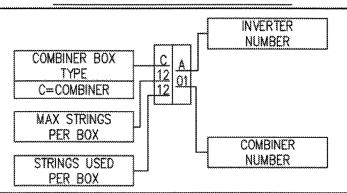
GROUND FUSE METER

DISCONNECT, NON FUSED

LINEAR FLUORESCENT FIXTURE

PHOTOVOLTAIC MODULE

COMBINER BOX NUMBERING SYSTEM



MODULE CHARACTERISTICS

YINGLI YL240P-29b Voc = 37.5 VVmp = 29.5 Vlsc = 8.65 AImp = 8.14 ATkvoc = $-0.14V/^{\circ}C$

Tlow = 1 °C (FROM ASHRAE TABLE)

INDEX

E1 ELECTRICAL NOTES E2 ELECTRICAL SITE PLAN E3 ELECTRICAL SECTION VIEWS E4 LINE DRAWING

E5 STRING DIAGRAMS E6 MONITORING E7 ELECTRICAL DETAILS

E8 SIGNAGE E9 LIGHTING DESIGNS

E10 TITLE 24 CONFORMANCE DOCUMENTS

SYSTEM COMPONENTS: • (756) YINGLI YL240P-29b PHOTOVOLTAIC MODULES CONFIGURED INTO (54) SERIES STRINGS OF (14) MODULES

• (2) SOLECTRIA PVI-82kW (480V) 3¢ CRID TIE INVERTER

ASHRAE EXTREME ANNUAL DRY BULB MEAN MINIMUM TEMPERATURE = 1° C ASHRAE 2% DRY BULB = 35° C

(BURBANK-GLENDALE-PASADENA AP, CA) MAX SYSTEM VOC CALCULATIONS

LOWEST EXPECTED AMBIENT TEMPERATURE FOR LA CRESCENTA, MAX VOLTAGE = # OF MODULES/STRING X (MODULE Voc -(Tstc-Trecord_low) X Tkvoc)

MAX VOC = 37.5 VDC - (25°C - 1°C)*-0.14 = 37.5 - -3.36 = 40.86 VDC MAX SYSTEM VOC = 40.86 VDC * 14 MODULES IN SERIES =

ENGINEER OF RECORD

CARL BURATTI BURATTI & ASSOCIATES, INC. 6345 BALBOA BLVO, STE 259 ENCINO, CA 91316 TEL: (818) 345-7130 FAX: (818) 345-7129 EMAIL: carl@buratti-pe.com

ROSEMONT MOUNT PV ROSEMONT AVE A 91214 GLENDALE USD –
181.44kW GROUND
GLENDALE USD –
4733 ROSEMONT /
LA CRESCENTA, C.

SCHOOL

MIDDLE SYSTEM



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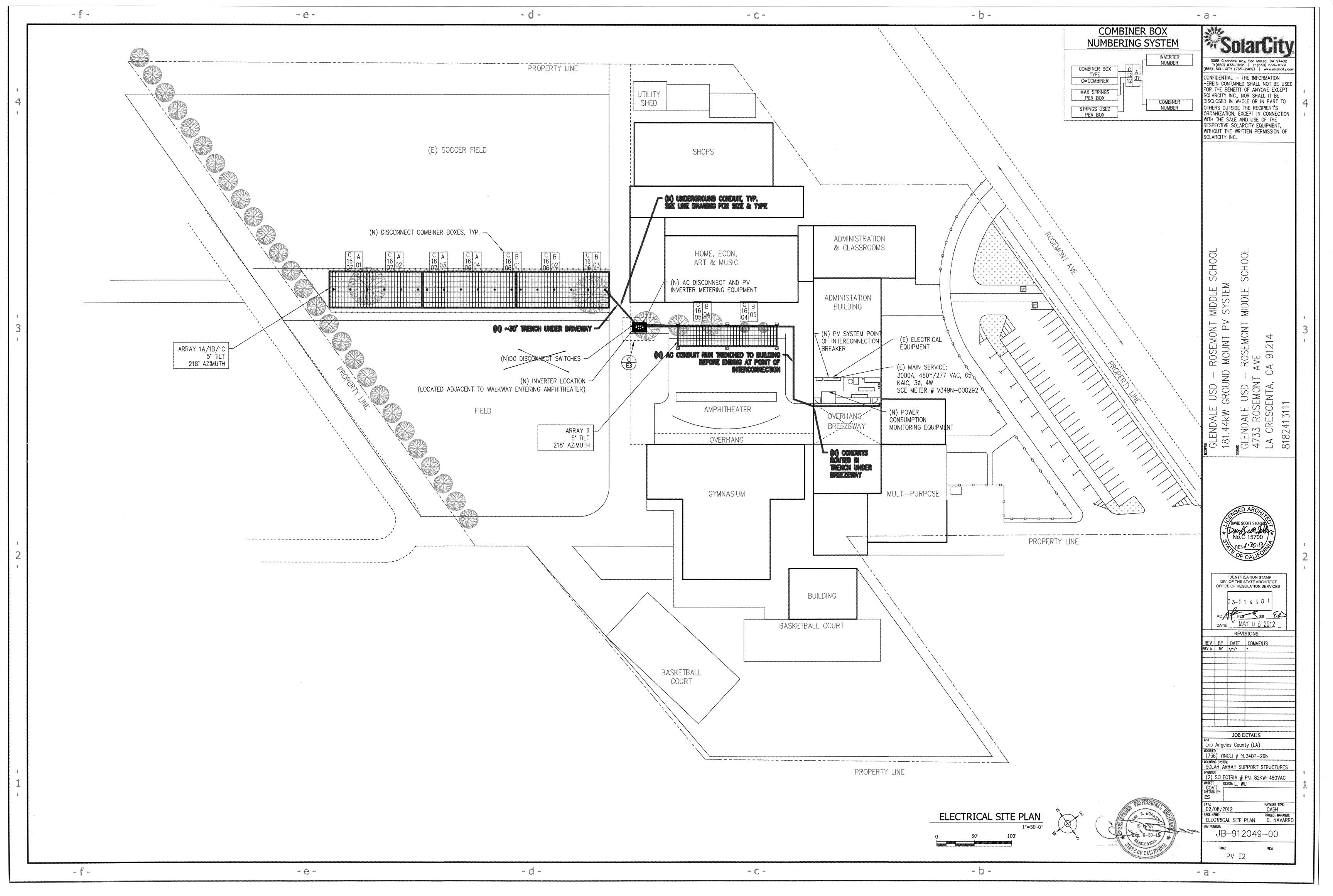
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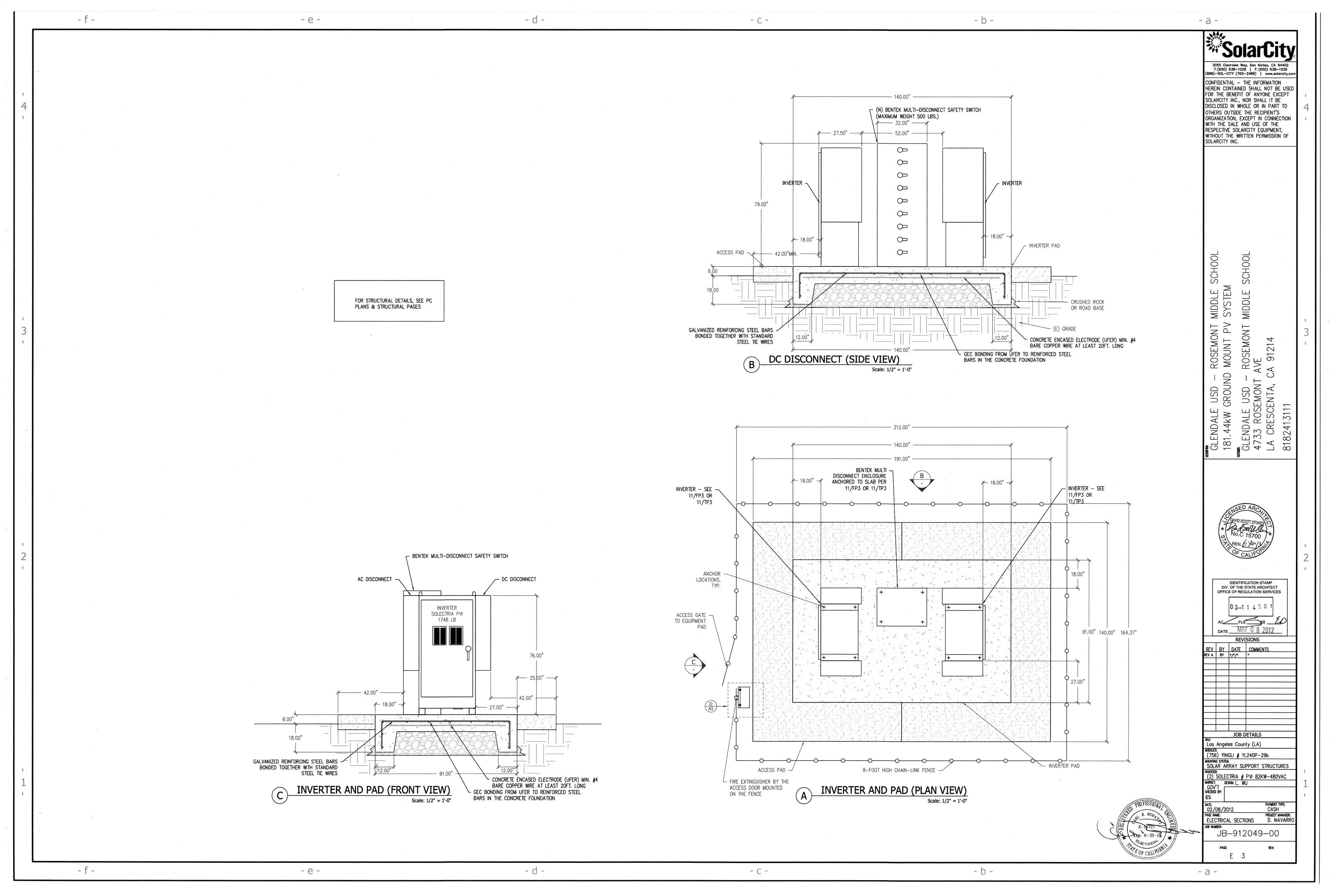
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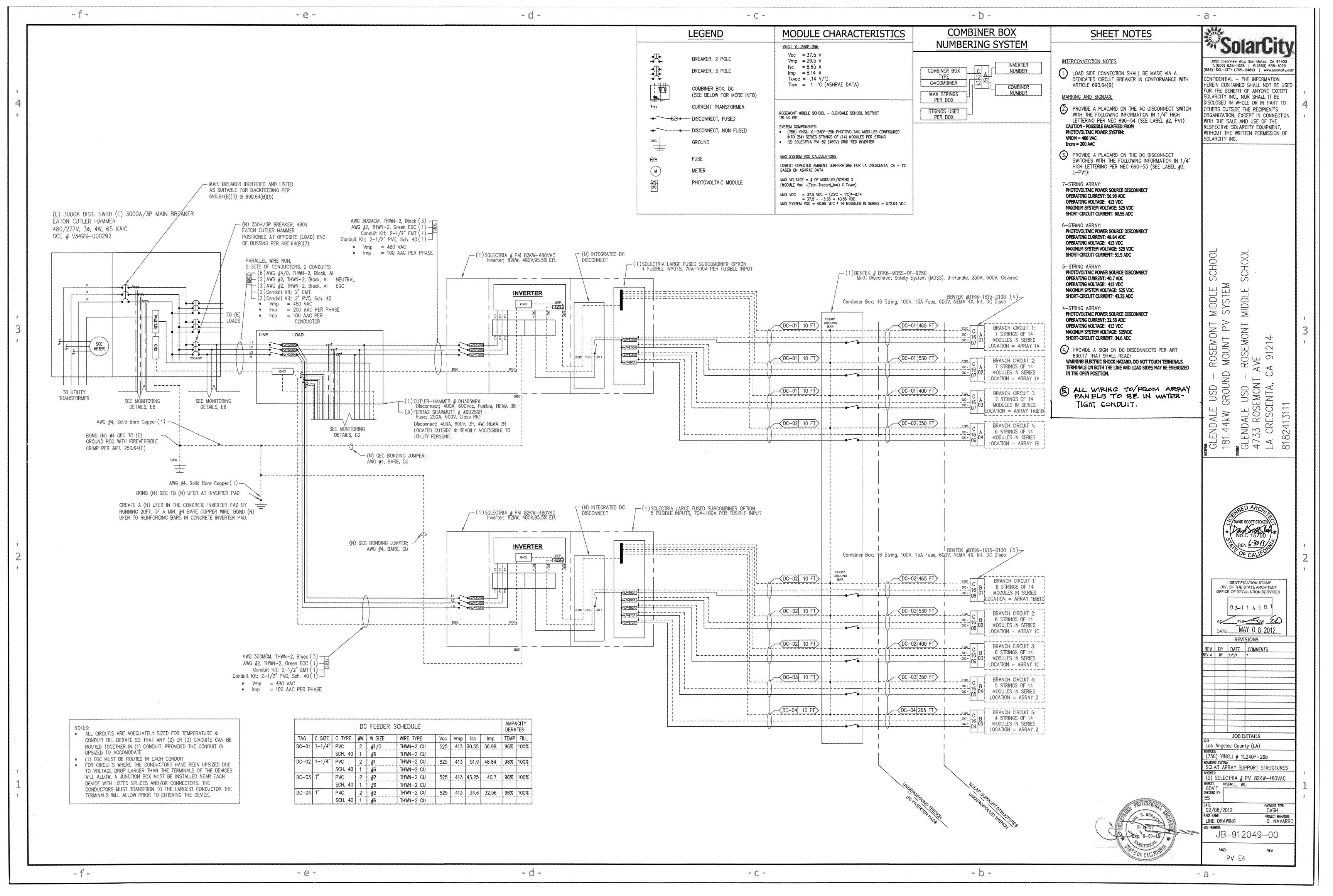
JOB DETAILS Los Angeles County (LA) (756) YINGLI # YL240P-29b SOLAR ARRAY SUPPORT STRUCTURES (2) SOLECTRIA # PVI 82KW-480VAC DESIGN: L. WIJ GOV'T CHECKED BY: CASH PROJECT MANAGER: ELECTRICAL NOTES D. NAVARRO

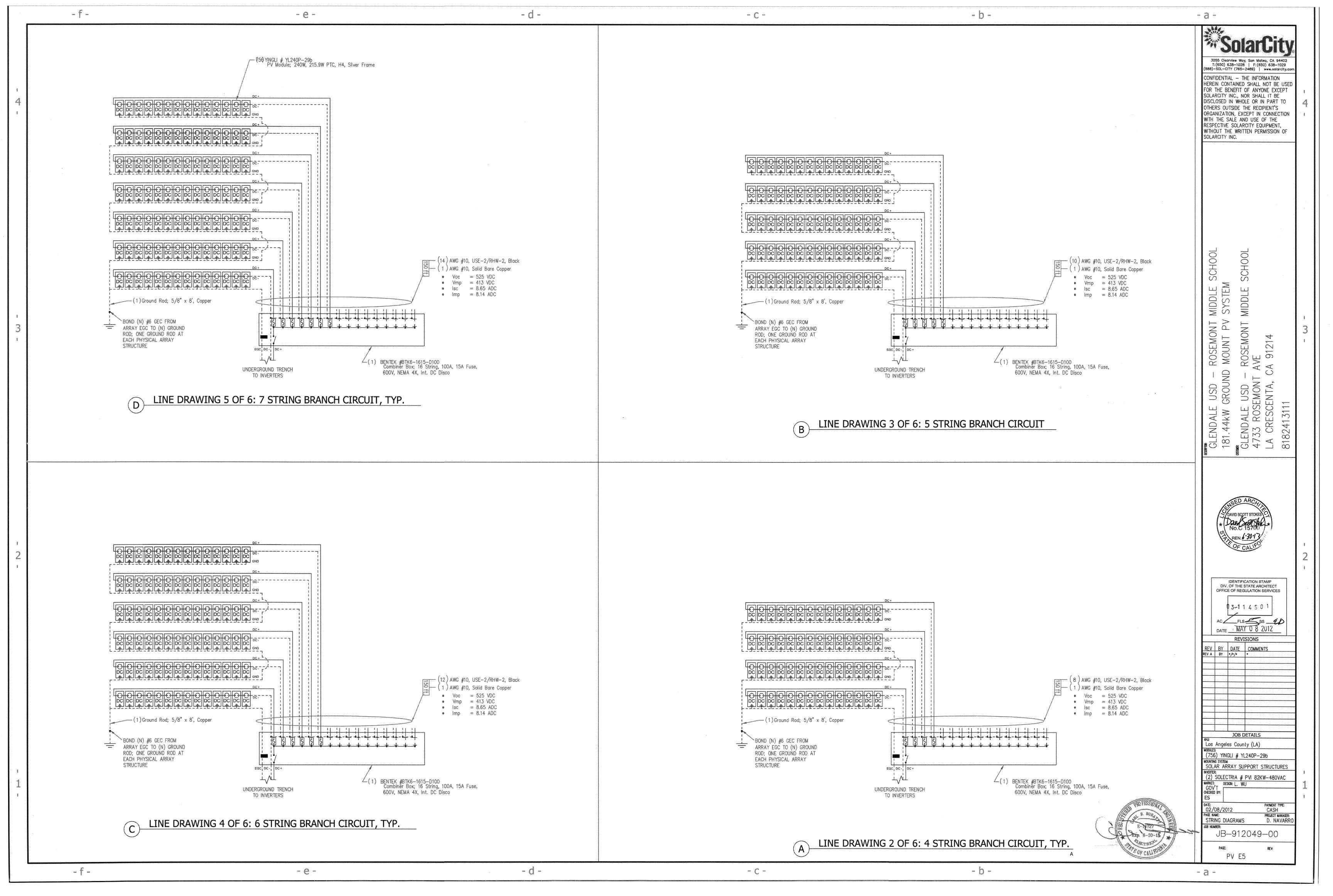
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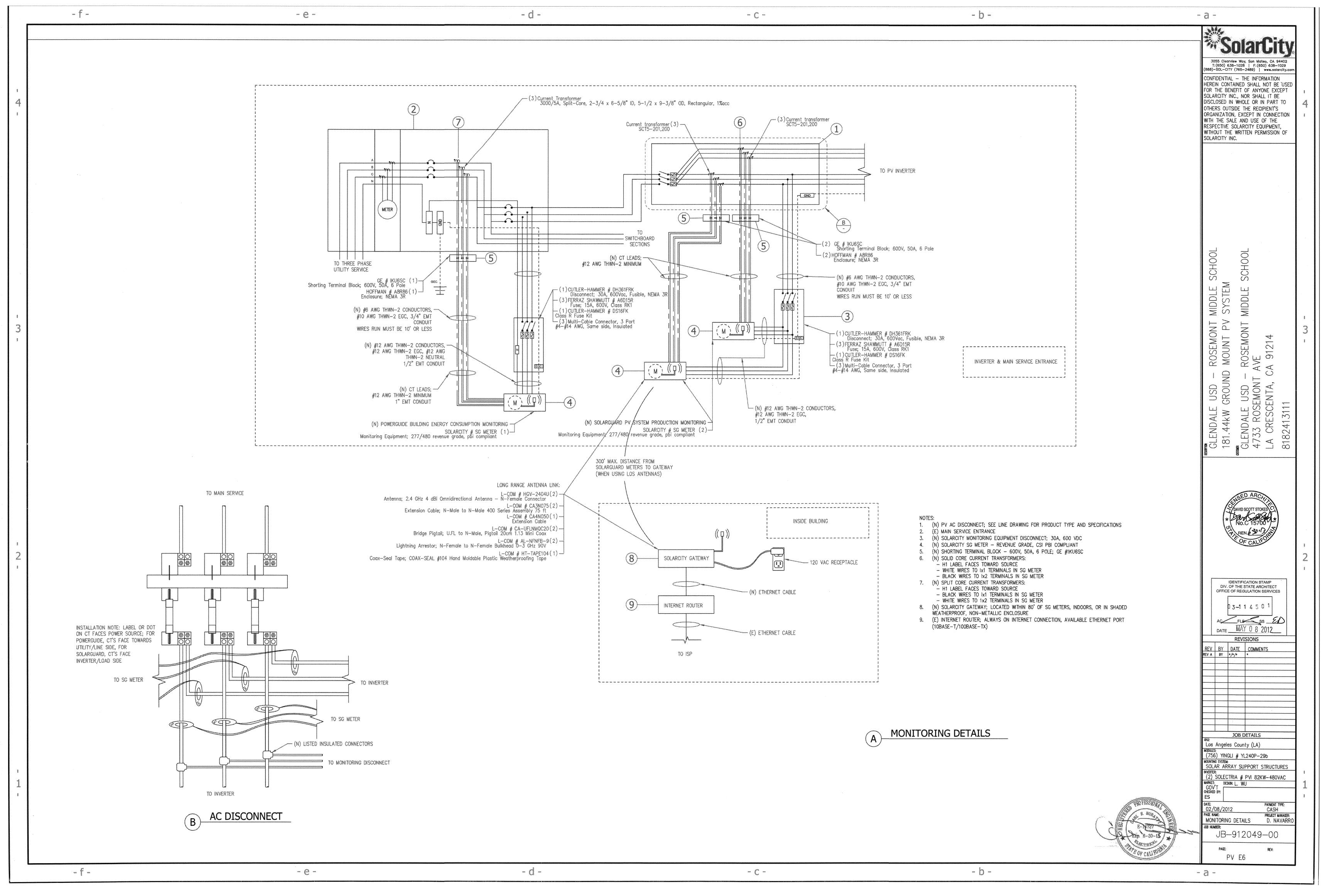
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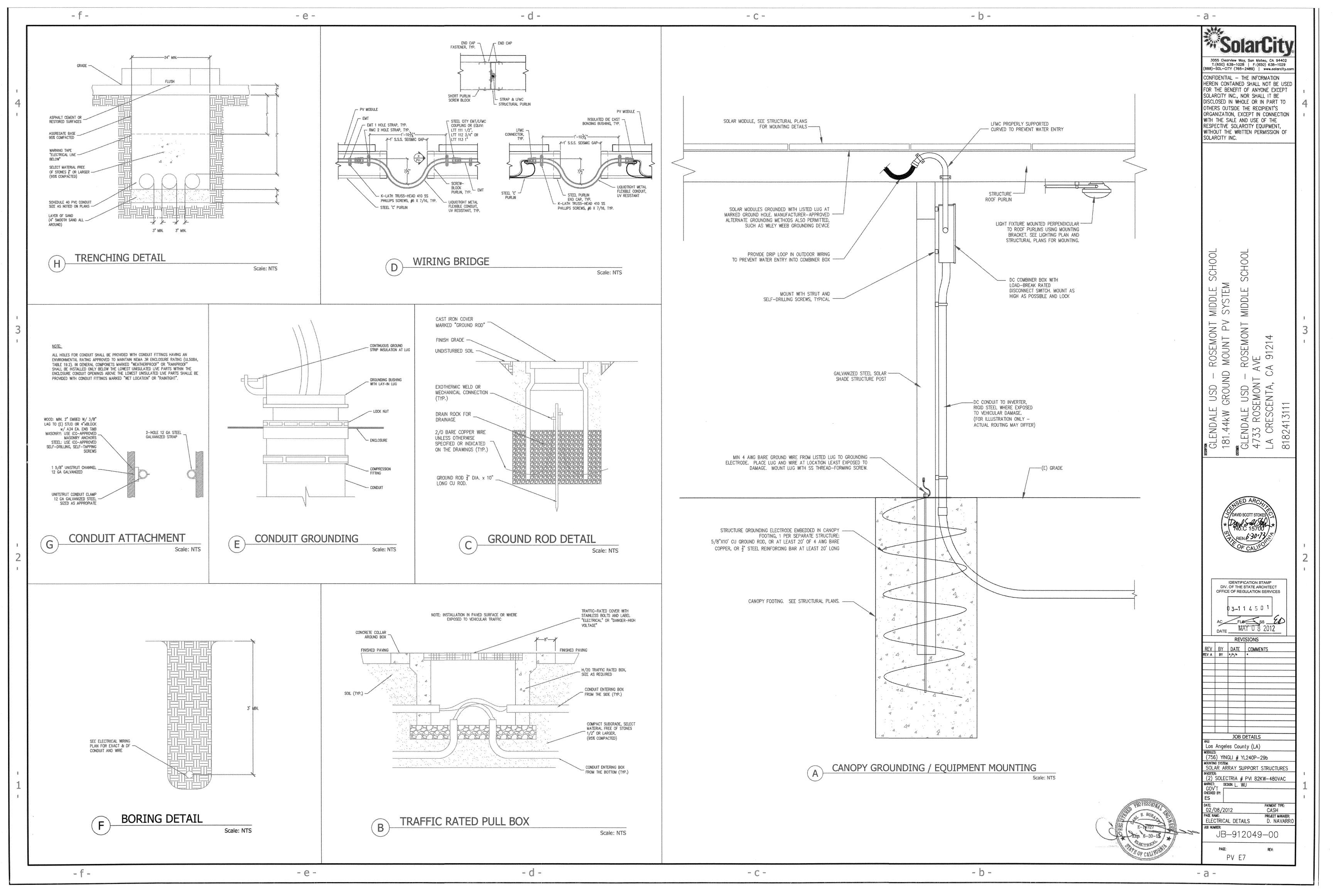


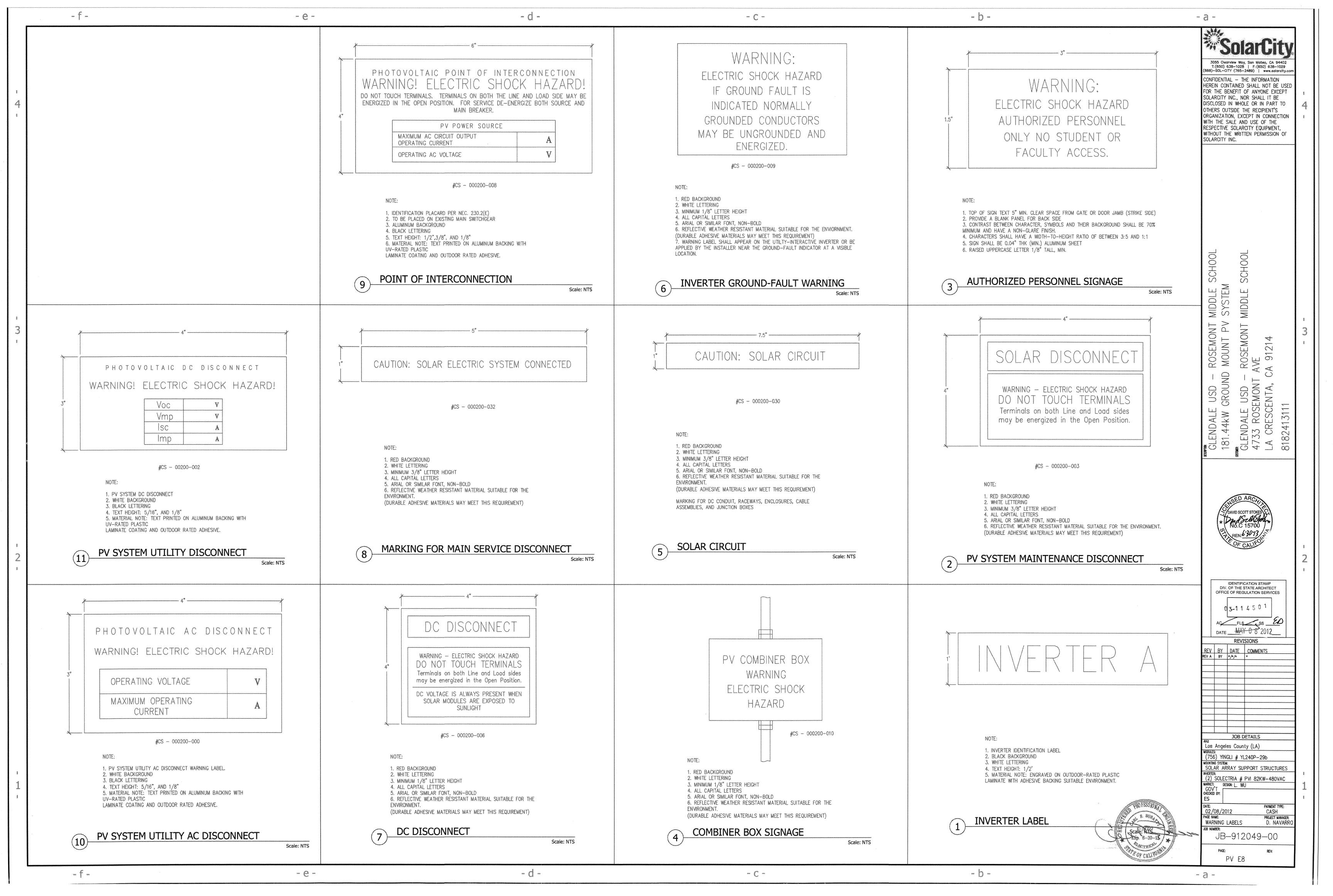












MANDATORY NOTES:

DETERMINING INSTALLED LIGHTING POWER

INSTALLED LIGHTING POWER HAS BEEN DETERMINED IN ACCORDANCE WITH \$130(C AND D).

CONTROLS FOR INEFFICIENT LIGHTING SYSTEMS
ALL OUTDOOR LUMINAIRES WITH LAMPS RATED OVER 100 W MUST EITHER: HAVE A LAMP EFFICACY OF AT LEAST 60 LUMENS PER WATT; OR BE CONTROLLED BY A MOTION SENSOR [§132(A)].

OUTDOOR LUMINAIRE CUTOFF

OUTDOOR LUMINAIRES THAT USE LAMPS RATED GREATER THAN 175 W (\$132(B)) IN THE HARDSCAPE AREAS, PARKING LOTS, BUILDING ENTRANCES, CANOPIES AND ALL OUTDOOR SALES AREAS WILL BE REQUIRED TO BE DESIGNATED CUTOFF IN A PHOTOMETRIC TEST REPORT THAT INCLUDES ANY TILT OR OTHER NON-LEVEL MOUNTING CONDITIONS.

CONTROLS TO TURN OFF THE LIGHTS DURING THE DAY

ALL PERMANENTLY INSTALLED OUTDOOR LIGHTING MUST BE CONTROLLED BY A PHOTOELECTRIC SWITCH OR ASTRONOMICAL TIME SWITCH THAT AUTOMATICALLY TURNS OFF THE OUTDOOR LIGHTING WHEN DAYLIGHT IS AVAILABLE (§132(C)1).

CONTROLS TO PROVIDE THE OPTION TO TURN OFF A PORTION OF THE LIGHTS

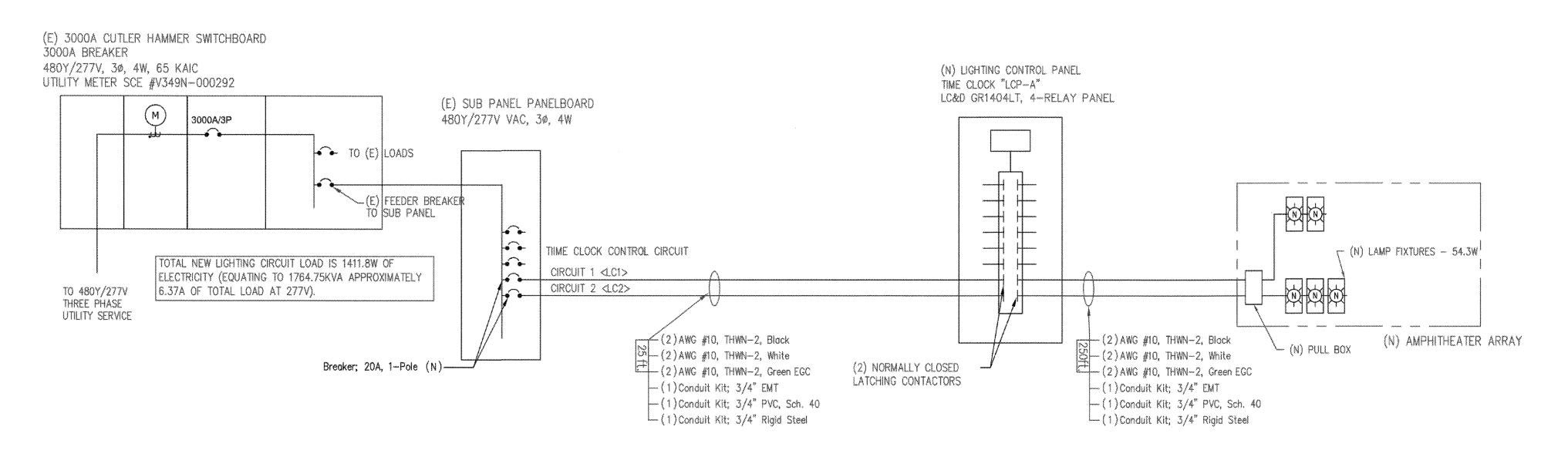
FOR LIGHTING OF BUILDING FACADES, PARKING LOTS, GARAGES, SALES AND NON-SALES CANOPIES, AND ALL OUTDOOR SALES AREAS, AUTOMATIC CONTROLS ARE REQUIRED TO PROVIDE THE OWNER WITH THE ABILITY TO TURN OFF THE LIGHTING OR TO REDUCE THE LIGHTING POWER BY AT LEAST 50 PERCENT BUT NOT EXCEEDING 80 PERCENT WHEN THE LIGHTING IS NOT NEEDED [\$132(C)2].

SYMBOL	LABEL	DESCRIPTION	LAMP TYPE	WATTAGE	MANUFACTURER	CATALOG NUMBER	VOLTAGE	QUANTITY
8	FCP	SURFACE MOUNTED 4 FOOT (2) LAMP T8 FLUORESCENT LUMINAIRE WITH SPECULAR REFLECTOR AND CLEAR WRAP LENS. POLYCARBONATE CONSTRUCTION WITH STAINLESS STEEL CLIPS AND TAMPER PROOF SCREWS.	TWO (2) 32W 4100K (MIN.) T8 FLUORESCENT LAMPS.	54.3W	GVA	FAD-PC-232-277-PH-SS-TRS	120-277V	5

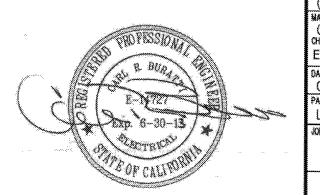
USE EMT WHEN ROUTING BETWEEN FIXTURES UNDER THE ARRAY. USE SCH. 40 PVC IN THE TRENCH. USE RIGID STEEL IN AREAS WHERE EXPOSED TO STRONG PHYSICAL DAMAGE (SUCH AS THE POST STUB UPS AS NECESSARY)

LIGHTING CIRCUIT (LC1) AND CIRCUIT (LC2) HAS 0.1629kW (0.2036kVA) AND 0.1086kW (0.1358kVA) RESPECTIVELY (EQUATING TO APPROXIMATELY 0.735A AND 0.392A RESPECTIVELY).

THE (N) ASTRONOMICAL TIME CLOCK SCHEDULED TO COMPLY WITH THE MULTI-LEVEL SWITCHING REQUIREMENT PER TITLE 24.



SINGLE LINE DIAGRAM FOR LIGHTING SYSTEM



3055 Clearview Way, San Mateo, CA 94402 T:(650) 638-1028 | F:(650) 638-1029 (888)-SOL-CITY (765-2489) | www.salaroity.coi HEREIN CONTAINED SHALL NOT BE USED FOR THE BENEFIT OF ANYONE EXCEPT SOLARCITY INC., NOR SHALL IT BE DISCLOSED IN WHOLE OR IN PART TO OTHERS OUTSIDE THE RECIPIENT'S ORGANIZATION, EXCEPT IN CONNECTION

WITH THE SALE AND USE OF THE

RESPECTIVE SOLARCITY EQUIPMENT, WITHOUT THE WRITTEN PERMISSION OF

SOLARCITY INC.

MIDDLE SYSTEM ROSEMONT MOUNT PV AVE 3A 91214 GLENDALE USD - F 181.44kW GROUND N GLENDALE USD - F 4733 ROSEMONT AN

IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT OFFICE OF REGULATION SERVICES 03-11450

REV BY DATE COMMENTS
REV A BY */*/* *

JOB DETAILS Los Angeles County (LA)

WOOULES: (756) YINGLI # YL240P-29b MOUNTING SYSTEM:
SOLAR ARRAY SUPPORT STRUCTURES (2) SOLECTRIA # PVI 82KW-480VAC
MARKET: DESIGN: L. WU

MARKET: GOV'T CHECKED BY:

02/08/2012 CASH
PAGE NAME: PROJECT MANAGER:
LIGHTING SINGLE LINE D. NAVARRO

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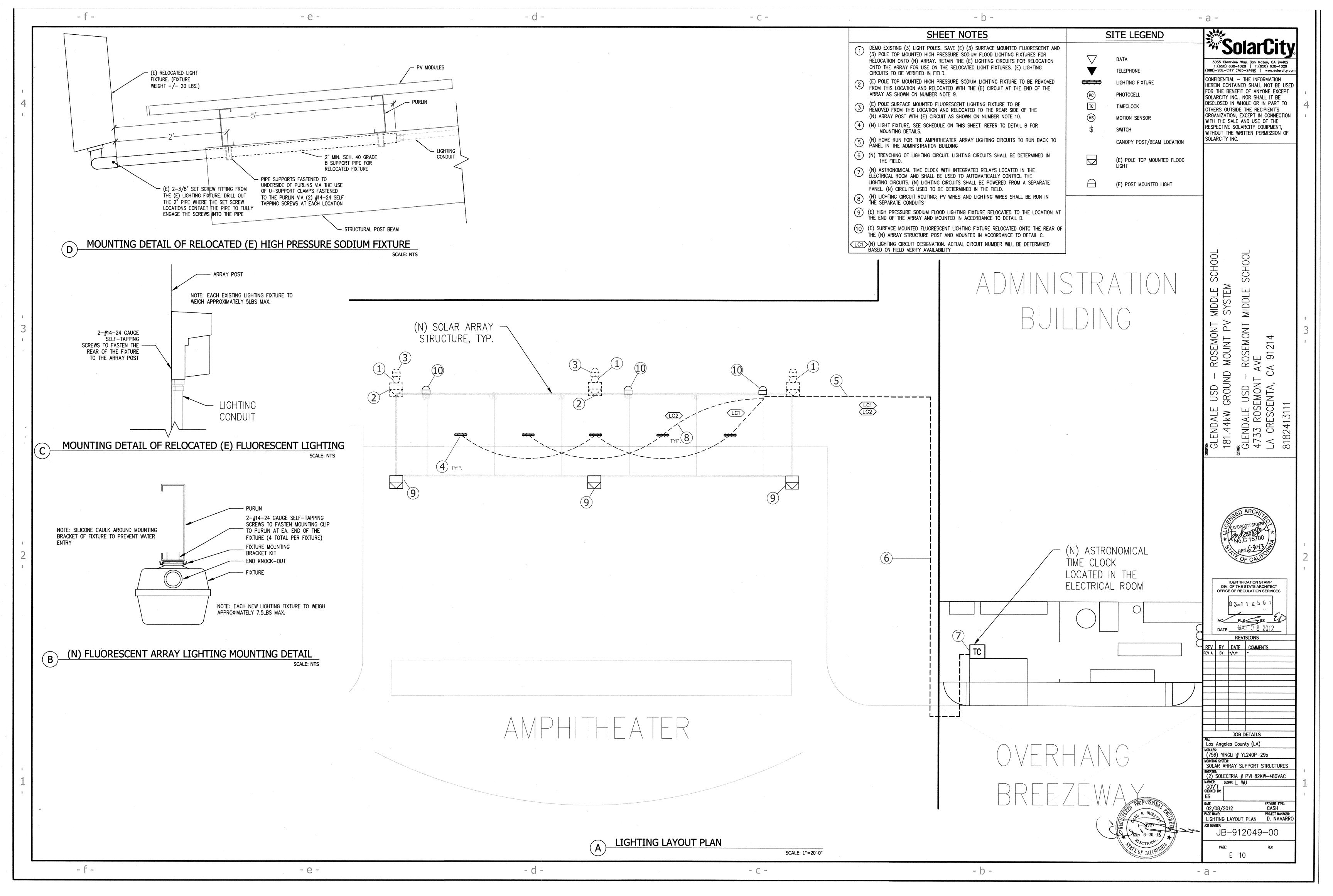
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PV E9

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Project Name:	(Page 1 of 4) OLTG-1
GLENDALE USD - ROSEMONT MIDDLE SCHOOL	Oale 03/08/2012
Project Address 4733 ROSEMONT AVE.	Total Hardscape Illiummated Area: 5,743
General Information	
	[Alteration]
Documentation Author's Declaration Statement • I certify that this Certificate of Compliance documentation is accurate and	Complete:
Nume LEO WU, P.E., LEED AP	
Company SOLAR CITY	Date: 03/08/2012
Address 3055 CLEARVIEW WAY	IP Applicables CLAS CEPES
City/State/Zip SAN MATEO, CA, 94402	Phone (650) 963-5820
Principal Lighting Designer's Declaration Statement	
◆I am eligible under Division 3 of the California Business and Professions Co	de to accept responsibility for the lighting desig
 This Certificate of Compliance identifies the lighting features and performan Title 24, Pages 1 and 6 of the California Code of Regulations 	and the second to the control of the second to
The design features represented on this Certificate of Compliance are consist	
this design reactes represented of this Continuate or Compliance are consist this design on the other applicable compliance forms, worksheets, calculate enforcement agency for approval with this building permit application	ions, plans and specifications submitted to the
Name: CARL BURATTI, P.E. Signature: (in the second se
Company BURATTI & ASSOCIATES, INC.	Phone; (818) 345-7130
Address 6345 BALBOA BLVD., STE 259	License # E14727
	Date: 03/08/2012
Ciny State Zip: ENCINO, CA 91316 Principal Lighting Designer's Declaration	I
CINCINO, OA SIDIO	implete, and accounts for all outdoor lighting ghting designed for the site, and that Additiona wer Allowances for Ordinance Requirements
Principal Lighting Designer's Declaration I certify that this Certificate of Compliance documentation is accurate and composer; including building mounted, pole mounted, as well as all other outdoor if Lighting Power Allowances for Specific Applications or Additional Lighting Power not been counted more than one time for the same area, in accordance with Outdoor Lighting Mandatory Measures Indicate location on building plans of Mandatory Measures Note Block:	implete, and accounts for all outdoor lighting ghting designed for the site, and that Additiona wer Allowances for Ordinance Requirements. Section 147 of the Standards. PV EG
Principal Lighting Designer's Declaration Teertify that this Certificate of Compliance documentation is accurate and compower, including building mounted, pole mounted, as well as all other outdoor librating Power Allowances for Specific Applications or Additional Lighting Pohave not been counted more than one time for the same area, in accordance with Outdoor Lighting Mandatory Measures Indicate location on building plans of Mandatory Measures Note Block: LIGHTING COMPLIANCE FORMS & WORKSHEETS (check box if worksheet is included) For detailed instructions on the use of this and all Energy Efficiency Standards compliance.	Implete, and accounts for all outdoor lighting ghting designed for the site, and that Additiona wer Allowances for Ordinance Requirements, Section 147 of the Standards.
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CERTIFICATE OF COMPLIANCE

Project Name: GLENDALE USD - ROSEMONT MIDDLE SCHOOL

ALLOWED AND INSTALLED OUTDOOR LIGHTING POWERS

Lighting power affowance for general hardscape (from OLTG-2C Page 1 of 3)

Specific application lighting wattage allowance per unit length (from OLTG-2C Page 1 of 3)

Specific application wattage allowance for ornamental lighting (from OLTG-2C Page 1 of 3)

Specific application wattage allowance per application (from OLTG-2C Page 2 of 3)

Specific application lighting wattage allowance per area (from OLTG-2C Page 2 of 3)

Total Allowed Wattage = Sum of rows A through F:

Total Installed Watts (from Luminaire Schedule, from OLTG-1C (Page 2 of 4)

Provided that the lighting wattage power allowances listed in rows A through F are identical to the lighting wattage power allowances taken from OLTG-2C Pages 1 through 3, complies if Installed Wattage in row H is less than or equal to the Total Installed Wattage in row G

Additional lighting power allowance for ordinance requirements (from OLTG-2C Page 3 of 3)

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Name TOF	Luminaire Description [†] See footnote below									
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	SolarGity
000000000000000000000000000000000000000	3055 Clearview Way, San Mateo, CA 94402 T:(650) 638–1026 F:(650) 638–1029 (888)–SOL–CITY (765–2469) www.salarcity.com
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GLENDALE USD — ROSEMONT MIDDLE SCHOOL

181.44kW GROUND MOUNT PV SYSTEM

CLENDALE USD — ROSEMONT MIDDLE SCHOOL

473 ROSEMONT AVE

LA CRESCENTA, CA 91214

8182413111

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MOULS
(756) YINGLI # YL240P-296 MOUNTING SYSTEM: SOLAR ARRAY SUPPORT STRUCTURES (2) SOLECTRIA # PVI 82KW-480VAC

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2008 Nonresidential Compliance Forms

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2008 Nonresidential Compliance Forms

NOTES:

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(Page 4 of 4) OLTG-1C

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03/08/2012

PERIODS, BUT SHALL BE READ AS SAME.

A.B. -----ANCHOR BOLT

ALT. — — — — — ALTERNATE

BM -----BEAM

BLK----BLOCK

BRG ----BEARING

C -----CAMBER

ARCH'L — — — — ARCHITECTURAL

A/C ---- AIR CONDITIONER

ABBREVIATIONS

CONSTRUCTION

CONSTRUCTION

AND MATERIALS

INSTITUTE

AITC---- AMERICAN INSTITUTE OF TIMBER

ANSI-----AMERICAN NATIONAL STANDARDS

APA ---- AMERICAN PLYWOOD ASSOCIATION

ASTM -----AMERICAN SOCIETY FOR TESTING

AWS ---- AMERICAN WELDING SOCIETY

B.F.F ----BELOW FINISHED FLOOR

B.O.B. ----BOTTOM OF BEAM

B.O.D. ----BOTTOM OF DECK

B.O.F. ----BOTTOM OF FOOTING

CFS ----COLD FORMED STEEL

C.L.B. -----CENTERLINE OF BEAM

C.L.C. -----CENTERLINE OF COLUMN

C.L.F. ----CENTERLINE OF FOOTING

CONC C.J. ---- CONCRETE CONTROL JOINT

CONC S.J. ---- CONCRETE SAWCUT JOINT

C.M.U. -----CONCRETE MASONRY UNIT

D.F. (D.F.L.) — — DOUGLAS FIR LARCH

E.C. ---- END TO CENTERLINE E.E. - - - - END TO END E.O.S. -----EDGE OF SLAB

EXP. BOLT (E.B.) — EXPANSION BOLT EXP. JT (E.J.) - - EXPANSION JOINT

F.O.W. ----FACE OF WALL

H.S. ----HEADED STUDS

GALV -----GALVANIZED

H.F. —————HEM FIR

IST -----JOIST

LBS (#) ---- POUNDS LGR -----LEDGER

LL -----LIVE LOAD

MAS ---- MASONRY

MAX ----- MAXIMUM

MIN ----- MINIMUM

MECH'L - - - MECHANICAL

MFR'D ---- MANUFACTURED

N/A ----NOT APPLICABLE

O.F.W.----OUTSIDE FACE OF WALL

P.C. — — — PRECAST CONCRETE

± ----PLUS OR MINUS

PT ----- POST-TENSIONED

SDI ----STEEL DECK INSTITUTE

SLV-----SHORT LEG VERTICAL

SJI ----STEEL JOIST INSTITUTE

SLH-----SHORT LEG HORIZONTAL

REINF - - - - REINFORCING

SIM -----SIMILAR

SQ. -----SQUARE

STL - - - - STEEL

STD----STANDARD

L ---- TOTAL LOAD

T.O.B. ———— TOP OF BEAM

T.O.D. - - - TOP OF DECK

T.O.F. ---- TOP OF FOOTING

T.O.L. - - - TOP OF LEDGER

T.O.P.————— TOP OF PLATE

T.O.S. ---- TOP OF STEEL

T.O.W.----TOP OF WALL

TYP ---- TYPICAL

W/ ----- WITH

W/O ---- WITHOUT

T.O.M. - - - TOP OF MASONRY

PREFAB ---- PREFABRICATED

PCF ---- POUNDS PER CUBIC FOOT

PLF ---- POUNDS PER LINEAR FOOT

PSF-----POUNDS PER SQUARE FOOT

PSI -----POUNDS PER SQUARE INCH

PTI -----POST-TENSIONING INSTITUTE

SSMA ----STEEL STUD MANUFACTURERS

T.O.C.T. ---- TOP OF CONCRETE TOPPING

T.O.P.C. ---- TOP OF PRECAST CONCRETE

TPI ---- TRUSS PLATE INSTITUTE

T&G ---- TONGUE AND GROOVE

UBC ---- UNIFORM BUILDING CODE

VERT ----- VERTICAL REINFORCING

W.W.F. ---- WELDED WIRE FABRIC

WWPA----- WESTERN WOOD PRODUCTS

W/C ---- WATER TO CEMENT RATIO

U.N.O. -- -- UNLESS NOTED OTHERWISE

WCLA ---- WEST COAST LUMBER ASSOCIATION

WCLIB---- WEST COAST LUMBER INSPECTION

ASSOCIATION

ASSOCIATION

OSHA - - - - OCCUPATIONAL SAFETY AND

PCI -----PRECAST/PRESTRESSED CONCRETE

HEALTH ADMINISTRATION

N.T.S. ----NOT TO SCALE

O.C. ----ON CENTER

OPP ----- OPPOSITE

MFR('S) ---- MANUFACTURER('S)

K(KIP) -----1000 POUNDS

E.W.----EACH WAY F.F. -----FINISHED FLOOR F.O.M. -----FACE OF MEMBER F.O.S. ----FACE OF STEEL

CRSI-----CONCRETE REINFORCING STEEL

DSA ---- DIVISION OF STATE ARCHITECT

GA -----GAGE (UNIT OF MEASUREMENT)

G.S.N. -----GENERAL STRUCTURAL NOTES GLB (GLULAM) — GLUED-LAMINATED BEAM

BC ----INTERNATIONAL BUILDING CODE

ICBO ---- INTERNATIONAL CONFERENCE OF

ICC -----INTERNATIONAL CODE COUNCIL

1.O.D.—————INTERPRETATION OF DRAWINGS

LGSEA ----LIGHT GAGE STEEL ENGINEERS

ASSOCIATION

BUILDING OFFICIALS

HORIZ -----HORIZONTAL REINFORCING

LE.W.-----INSIDE FACE OF WALL

KLF -----KIPS PER LINEAR FOOT

L.O.D. ----- LOCATION OF DETAILS

LLH -----LONG LEG HORIZONTAL

MAS C.J. -- -- MASONRY CONTROL JOINT

MBMA---- METAL BUILDING MANUFACTURERS

MWFRS ---- MAIN WIND FORCE RESISTANCE

ASSOCIATION

LLV -----LONG LEG VERTICAL

LGS -----LIGHT GAGE STEEL

C.L.W. -----CENTERLINE OF WALL

C.G.----CENTER OF GRAVITY

C.I.P. ———— CAST IN PLACE

C.L. - - - - CENTERLINE

CLR-----CLEAR

CONC -----CONCRETE

CONN -----CONNECTION

CONT -----CONTINUOUS

DL -----DEAD LOAD

DIA —————DIAMETER

DWG(S) ---- DRAWING(S)

DN ----DOWN

EQ -----EQUAL EQUIP -----EQUIPMENT

C.C.----CENTERLINE TO CENTERLINE

C & C ---- COMPONENTS & CLADDING

CBC -----CALIFORNIA BUILDING CODE

A.W.T.S. ----AUTOMATIC WELDED THREADED

NOTE: ABBREVIATIONS MAY OR MAY NOT HAVE

AISI ------AMERICAN IRON AND STEEL

2010 EDITION OF THE CALIFORNIA BUILDING CODE. OCCUPANCY GROUP PER SITE-SPECIFIC DOCUMENTS. ALLOWABLE

GENERAL STRUCTURAL NOTES

AREA AND MINIMUM SEPARATION BETWEEN STRUCTURES TO BE A.B.C. -----AGGREGATE BASE COURSE DETERMINED AT EACH SPECIFIC LOCATION PER CBC WHICH IS TO ACI -----AMERICAN CONCRETE INSTITUTE BE CHECKED AT BACKCHECK. A.F.F. ----ABOVE FINISHED FLOOR II-B CONSTRUCTION AISC-----AMERICAN INSTITUTE OF STEEL

BUILDING CODE:

ROOF DEAD LOAD = ACTUAL WEIGHT OF MEMBER: SOLAR PANEL = 3 PSF (MAX) PURLIN = 4 PLF

C&C WIND LOAD = 18.9 PSF (TOWARD THE SURFACE). C&C WIND LOAD = 20.8 PSF (AWAY FROM THE SURFACE). MWFRS WIND LOAD = 18.9 PSF / 4.4 PSF (TOWARD THE SURFACE). MWFRS WIND LOAD = 17.8 PSF / 0.0 PSF (AWAY FROM THE SURFACE). ROOF LIVE LOAD = 10 PSF. DESIGN FOR 300 POUND POINT LOAD LOCATED TO CAUSE MAXIMUM MOMENTS AND SHEAR. USE THE 300 POUND LOAD WITH WIND, BUT NOT WITH 10 PSF ROOF LIVE LOAD.

OCCUPANCY CATEGORY II 3 SECOND WIND GUST = 85 MPH

WIND IMPORTANCE FACTOR = 1.0.

DESIGN BASE SHEAR (6 PANEL) = 5250 LBS.

DESIGN BASE SHEAR (7 PANEL) = 6270 LBS.

FOR 10 DEGREE ROOF SLOPE:

THIS DESIGN CAN BE USED FOR ANY ROOF SLOPE FROM 0 DEGREES TO 10 DEGREES.

NO STEEL DECK IS TO BE PLACED ON THE STRUCTURE - NOW OR IN THE FUTURE.

SEISMIC IMPORTANCE FACTOR = 1.0. SHORT PERIOD SPECTRAL ACCELERATION Ss = 2.85 ONE SECOND SPECTRAL ACCELERATION S1 = 1.15. REDUNDANCY FACTOR p = 1.3. Sds = 1.005 (MAX.).Sd1 = 1.16 (MAX.).SEISMIC DESIGN CATEGORY D BASIC SEISMIC-FORCE RESISTING SYSTEM = CANTILEVERED COLUMN SYSTEMS DETAILED TO CONFORM TO THE REQUIREMENTS FOR ORDINARY STEEL MOMENT FRAMES. RESPONSE MODIFICATION FACTOR (R)= 1.25. ANALYSIS PROCEDURE USED = EQUIVALENT LATERAL FORCE PROCEDURE. Cs = 1.52.

FOUNDATIONS:

ALL FOOTINGS SHALL BE DESIGNED FOR THE SPECIFIC SITE. DRILLED PIER FOOTING DESIGNS ARE BASED ON THE ALLOWABLE LATERAL BEARING PRESSURES SHOWN IN DETAIL 2. THE ALLOWABLE LATERAL BEARING PRESSURE MAY BE MULTIPLIED BY 2.0 PER CBC SECTION 1806A.3.4. THE DRILLED PIER FOOTINGS ARE DESIGNED AS CONSTRAINED (SECTION 1807A.3.2.2, EQUATION 18A-2) WHERE PLACED IN A CONCRETE PAVEMENT AREA AND AS UNCONSTRAINED (SECTION 1807A.3.2.2, EQUATION 18A-1 OR CZERNIAK, WHICHEVER IS DEEPER) WHERE PLACED IN ASPHALT PAVEMENT AREAS OR DIRT AREAS.

SPREAD FOOTING DESIGNS ARE BASED ON CBC SECTION 1806A, CLASS 5 SOILS. SPREAD FOOTINGS SHALL BEAR ON FIRM, UNDISTURBED SOIL 2 FEET MINIMUM BELOW ADJACENT EXISTING GRADE. DESIGN SOIL BEARING VALUE = 1500 PSF. SOILS ENGINEER MUST VERIFY THAT 1500 PSF SOILS (MINIMUM) ARE PRESENT AT SITE.

CONCRETE:

SPECIFIED 28 DAY COMPRESSIVE STRENGTH F'c: FOUNDATIONS ----- 3,000 PSI

ALL CAST-IN-PLACE CONCRETE CONSTRUCTION SHALL CONFORM TO THE LATEST EDITION OF THE ACI. MECHANICALLY VIBRATE ALL CONCRETE WHEN PLACED UNLESS NOTED OTHERWISE. ADMIXTURES CONTAINING CHLORIDES SHALL NOT BE USED. NO OTHER ADMIXTURES PERMITTED WITHOUT APPROVAL. FOR CONCRETE WITHOUT PLASTICIZER, MAXIMUM SLUMP 4 1/2" AT POINT OF PLACEMENT U.N.O. IF PLASTICIZER IS USED, A HIGHER FINAL SLUMP MAY BE ALLOWED UPON STRUCTURAL ENGINEER'S APPROVAL.

FOR REINFORCING INFORMATION, SEE REINFORCING SECTION OF G.S.N., PLANS, SCHEDULES AND

FLY ASH - SHALL BE LIMITED TO 50% OF TOTAL CEMENTITIOUS MATERIALS BY WEIGHT. TEST DATA FOR EACH CONCRETE MIX SHALL BE SUBMITTED FOR REVIEW PER CHAPTER 5 OF ACI 318. REFERENCE FIGURE R5.3 FOR SUBMITTAL REQUIREMENTS AND OPTIONS. CONCRETE MIX

DESIGNS THAT ARE SUBMITTED WITHOUT THE APPROPRIATE TEST DATA CANNOT BE REVIEWED. IT IS ACCEPTABLE AND INTENDED TO USE EARTH CUTS FOR THE DRILLED PIER FOOTING AND SPREAD FOOTING. THE FOOTING DESIGNS INDICATED ON THIS SHEET DO NOT APPLY IF THE EARTH CUTS ARE UNSTABLE AND/OR DO NOT STAND ON THEIR OWN.

THE FOOTINGS INDICATED ON THIS SHEET DO NOT APPLY WHERE ORGANIC FILL MATERIALS EXIST. CONCRETE SHALL BE ADEQUATELY VIBRATED AROUND THE EMBEDDED STEEL COLUMNS TO ENSURE THE CONCRETE HAS COMPLETELY SURROUNDED THE STEEL COLUMN AND TO ENSURE THE CONCRETE AT THE INSIDE OF THE STEEL COLUMN HAS RISEN TO THE LEVEL OF THE CONCRETE IN THE

REMAINDER OF THE DRILLED PIER OR SPREAD FOOTING. CONCRETE SHALL SLOPE UP SLIGHTLY TOWARDS COLUMNS TO PREVENT WATER FROM PONDING AROUND COLUMNS,

IT IS ACCEPTABLE FOR CONCRETE TO FREE FALL INTO FOOTINGS. REINFORCING:

ALL REINFORCING PER CRSI SPECIFICATIONS AND HANDBOOK. ASTM A615 (Fy = 60 KSI / GRADE 60) DEFORMED BARS FOR ALL BARS. WHERE SHOWN ON DRAWINGS ALL GRADE 60 REINFORCING TO BE WELDED SHALL BE ASTM A706. NO TACK WELDING OF REINFORCING BARS ALLOWED WITHOUT PRIOR REVIEW OF PROCEDURE WITH THE STRUCTURAL ENGINEER. LATEST ACI CODE AND DETAILING MANUAL APPLY. CLEAR CONCRETE COVERAGES AS FOLLOWS:

EXPOSED TO EARTH OR WEATHER #6 OR LARGER ----\$5 AND SMALLER ----- 1 1/2"

ALL OTHER PER LATEST EDITION OF ACI 318 ALL REINFORCING SHALL BE CHAIRED TO ENSURE PROPER CLEARANCES. SUPPORT OF FOUNDATION REINFORCING MUST PROVIDE ISOLATION FROM MOISTURE/CORROSION BY USE OF A PLASTIC OR CONCRETE CHAIR. DUCT-TAPE COVERED REINFORCING IS NOT AN ACCEPTABLE CHAIR.

EDGE OF REINFORCING, AND SHALL NOT BE LESS THAN STATED, NOR GREATER THAN "CLEAR" DIMENSION PLUS 3/8". ALL OTHERS SHALL BE PLUS OR MINUS 1/4" TYPICAL UNLESS NOTED

ALL DIMENSIONS REFERENCED IN DRAWINGS AS "CLEAR" SHALL BE FROM FACE OF STRUCTURE TO

FIELD BENDING OR STRAIGHTENING OF DEFORMED BARS SHALL BE LIMITED TO #5 BARS AND SMALLER AND SHALL BE FIELD BENT OR STRAIGHTENED ONLY ONCE. ANY BEND SHALL BE LIMITED TO 90 DEGREES. IF FIELD BENDING OR STRAIGHTENING OF #6 BARS OR LARGER IS REQUIRED, OR II A SECOND BEND IS REQUIRED FOR #5 BARS AND SMALLER, HEAT SHALL BE APPLIED FOR BENDING OR STRAIGHTENING. CONTRACTOR SHALL SUBMIT PROCEDURE FOR APPLYING HEAT TO ENGINEER FOR REVIEW AND APPROVAL PRIOR TO BENDING OR STRAIGHTENING BARS.

STRUCTURAL STEEL:

GENERAL:

INSPECTION) IS REQUIRED.

ALL CONSTRUCTION PER LATEST AISC STEEL CONSTRUCTION ALL CONSTRUCTION PER LATEST AISC STEEL CONSTRUCTION MANUAL. ALL WIDE FLANGE STEEL SHALL BE ASTM A992 (Fy = 50 KSI). ALL PIPE STEEL SHALL BE ASTM A500 (Fy = 42 KSI) OR

STEEL SHALL BE ASTM A529 OR A572. ALL STRUCTURAL ROLLED STEEL MEMBERS WITH FY GREATER THAN 36 KSI ARE TO BE IDENTIFIED WITH AN ASTM SPECIFICATION MARK OR TAG PER IBC SEC. 2203.1.

ASTM A53, TYPE E OR S, GRADE B (Fy = 35 KSI). ALL MISCELLANEOUS STEEL UNLESS NOTED

OTHERWISE SHALL BE ASTM A36 (Fy = 36 KSI). IF CALLED OUT ON PLANS, Fy = 50 KSI PLATE

HOLLOW STRUCTURAL SHAPE (HSS):

PATENTS PENDING

HSS COLUMNS ARE CALLED OUT ON THE DRAWINGS AS EITHER ASTM A500 (Fy = 46 KSI) OR ASTM A572 (Fy = 65 KSI).

ASTM A500 (Fy = 46 KSI) HSS SECTIONS ARE TO BE PRODUCED PER THE SPECIFICATIONS SET ASTM A572 (Fy = 65 KSI) HSS SECTIONS ARE TO BE PRODUCED BY DIRECT-FORMING OR FOLDING OF THE PLATE FOLLOWED BY AN ELECTRIC RESISTANCE WELD ALONG THE SEAM. INLINE INSPECTION OF THE WELD ZONE DURING PRODUCTION BY NON-DESTRUCTIVE TESTING (NDT) (ULTRASONIC

THE TERMS PIPE AND ROUND HOLLOW STRUCTURAL SHAPE (HSS) ARE USED SYNONYMOUSLY THROUGHOUT THESE DOCUMENTS ALONG WITH THE TERMS TUBE STEEL AND RECTANGULAR OR

ALL BOLTS SHALL BE ASTM A325 AND SHALL BE INSTALLED AS SLIP CRITICAL CONNECTIONS WITH THREADS INCLUDED IN SHEAR PLANE. TIGHTEN BOLTS PER AISC SPECIFICATIONS. IT IS ACCEPTABLE TO USE OVERSIZE HOLES OR SLOTTED HOLES PER AISC SPECIFICATIONS.

UNLESS NOTED OTHERWISE, ALL WELDS PER LATEST EDITION OF THE AWS STANDARDS. ALL WELDING SHALL BE PERFORMED BY WELDERS HOLDING VALID CERTIFICATES AND HAVING CURRENT EXPERIENCE IN THE TYPE OF WELD SHOWN ON THE DRAWINGS OR NOTES. CERTIFICATES SHALL BE THOSE ISSUED BY AN ACCEPTED TESTING AGENCY. ALL WELDING DONE BY E70 SERIES LOW HYDROGEN RODS UNLESS NOTED OTHERWISE. FOR GRADE 60 REINFORCING BARS, USE E90 SERIES. THESE DRAWINGS DO NOT DISTINGUISH BETWEEN SHOP AND FIELD WELDS; THE CONTRACTOR MAY SHOP WELD OR FIELD WELD AT THEIR DISCRETION. SHOP WELDS AND FIELD WELDS SHALL BE SHOWN ON THE SHOP DRAWINGS SUBMITTED FOR REVIEW.

ALL FULL (COMPLETE) PENETRATION WELDS SHALL BE TESTED AND CERTIFIED BY AN INDEPENDENT

ALL SPOT WELDS SHALL BE PER LATEST AISI AND AWS STANDARDS.

STEEL CONNECTORS:

SCREW FASTENERS:

ALL STEEL SCREWS SHALL BE IN ACCORDANCE WITH AISI-GENERAL AND AISI-NAS. Fy = 50 ksi AND Ft = 70 ksi FOR ALL SCREWS.

MINIMUM SPACING OF SCREWS SHALL NOT BE LESS THAN 3 TIMES THE NOMINAL DIAMETER. MINIMUM EDGE DISTANCE FOR SCREWS SHALL NOT BE LESS THAN 1.5 TIMES THE NOMINAL SCREW 2. THE HEAD OF THE SCREW OR WASHER SHALL HAVE A DIAMETER, DW, OF NOT LESS THAN 5/16". WASHERS SHALL BE AT LEAST 0.05" THICK.

SCREW NUMBER DESIGNATION	8	10	12 (12–14)	14
NOMINAL DIAMETER	0.164"	0.190*	0.216"	0.250"

COLD FORMED STRUCTURAL STEEL FRAMING:

all cold formed steel components indicated on the structural drawings shall be FABRICATED AND ERECTED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND IN ACCORDANCE WITH THE LATEST EDITION OF "SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" BY THE AISI.

ALL STRUCTURAL STEEL FRAMING MATERIAL AND ITS ERECTION SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE AMERICAN IRON AND STEEL INSTITUTE "SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBER".

ALL WELDING TO BE PERFORMED BY WELDERS HOLDING A VALID CERTIFICATE AND HAVING CURRENT EXPERIENCE IN LIGHT GAUGE STEEL. CERTIFICATES SHALL BE ISSUED BY AN ACCEPTED TESTING agency. Do not notch flanges of members without expressed approval of the engineer OF RECORD. ALL WELDING TO BE PERFORMED IN AN APPROVED FABRICATORS SHOP

STRUCTURAL STEEL MEMBERS ARE FURNISHED TO A SPECIFIED MINIMUM Fy = 55,000 PSI. U.N.O. the grade and the astm specification number or other specification designation shall BE INDICATED BY PAINTING, DECAL, TAGGING OR OTHER SUITABLE MEANS ON EACH BUNDLE OF FABRICATED ELEMENTS. IT IS ACCEPTABLE TO USE THE FY SHOWN ON THE MILL CERTIFICATION IN LIEU OF THE "ORDERED" Fy. IT IS ACCEPTABLE TO USE STEEL WITH FY = 70 KSI IF THE STEEL USED IS IN THE AISI AND/OR AISC SPECIFICATION, THE ELONGATION IN A 2" COUPON IS A MINIMUM OF 10% AND THE RATIO OF Ft OVER FY IS AT LEAST 1.08.

MILS	GAGE NO.	MIN DELIVERED THICKNESS	DESIGN THICKNESS
12	30	0.0120	0.0126*
14	29	0.0132"	0.0139"
16	26	0.0174"	0.0183"
33	20	0.0336"	0.0354"
43	18	0.0447"	0.0470"
54	16	0.0561"	0.0590"
68	14	0.0713"	0.0750"
97	12	0.0998"	0.1050"
118	10	0.1283*	0.1350"

GENERAL NOTES:

THE STRUCTURAL CONSTRUCTION DOCUMENTS REPRESENT THE FINISHED STRUCTURE. EXCEPT WHERE NOTED, THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. THE STRUCTURAL ENGINEER OF RECORD SHALL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S MEANS, METHODS, TECHNIQUES, SEQUENCES FOR PROCEDURE OF CONSTRUCTION, OR THE SAFETY PRECAUTIONS AND THE PROGRAMS INCIDENT THERETO (NOR SHALL OBSERVATION VISITS TO THE SITE INCLUDE INSPECTION OF THESE ITEMS).

WHERE REFERENCE IS MADE TO VARIOUS TEST STANDARDS FOR MATERIALS, SUCH STANDARDS SHALL BE THE LATEST EDITION AND/OR ADDENDA. ANY ENGINEERING DESIGN, PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW, SHALL BEAR THE SEAL OF A REGISTERED ENGINEER RECOGNIZED BY THE BUILDING CODE JURISDICTION OF THIS PROJECT.

NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT, AND/OR AS PROVIDED FOR IN THE CONTRACT DOCUMENTS. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL STRUCTURAL NOTES AND SPECIFICATIONS, THE GREATER REQUIREMENTS SHALL GOVERN.

ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR ARCHITECTURAL. CIVIL. MECHANICAL. PLUMBING AND ELECTRICAL ITEMS WITH THE APPROPRIATE TRADE DRAWINGS AND SUBCONTRACTORS PRIOR TO CONSTRUCTION. TYPICAL DETAILS MAY NOT NECESSARILY BE CUT ON PLANS, BUT APPLY UNLESS NOTED OTHERWISE.

CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION OF ALL DIMENSIONS WITH ARCHITECTURAL

DRAWINGS PRIOR TO START OF CONSTRUCTION. RESOLVE ANY DISCREPANCY WITH THE ARCHITECT.

CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED CONSTRUCTION. LOAD SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT. OPTIONS ARE FOR CONTRACTOR'S CONVENIENCE. IF AN OPTION IS CHOSEN, CONTRACTOR SHALL

BE RESPONSIBLE FOR ALL NECESSARY CHANGES, APPROVALS AND THE COORDINATION OF THE WORK WITH ALL RELATED TRADES AND SUPPLIERS.

SPECIAL INSPECTION - STRUCTURAL ONLY:

SPECIAL INSPECTION IS REQUIRED PER CHAPTER 17A OF THE CBC FOR THE FOLLOWING:

CONCRETE CONSTRUCTION:

. DURING THE TAKING OF TEST SPECIMENS. B. THE PLACEMENT OF ALL FOUNDATION CONCRETE.

2. REINFORCING STEEL: INSPECTION OF IN-PLACE REINFORCING FOR CONFORMANCE PRIOR TO THE CLOSING OF FORMS OR THE DELIVERY OF CONCRETE TO THE JOBSITE FOR THE FOLLOWING: A. REINFORCING FOR SPREAD FOOTING AND DRILLED PIER CONCRETE FOUNDATIONS. B. REINFORCING FOR INVERTER SLABS ON THE GROUND.

STEEL CONSTRUCTION:

. PERIODIC VISUAL INSPECTION OF ALL FIELD WELDS. B. CONTINUOUS INSPECTION OF ALL MULTIPASS FILLET WELDS OR SINGLE PASS FILLET WELDS LARGER THAN 5/16". C. NON-DESTRUCTIVE TESTING OF ALL COMPLETE PENETRATION WELDS BY AN AWS CERTIFIED INDEPENDENT TESTING LABORATORY AT THE CONTRACTORS EXPENSE. . VERIFICATION OF VALID WELDER'S CERTIFICATES.

E. ALL STRUCTURAL STEEL FABRICATORS SHALL EMPLOY AN AWS CERTIFIED INDEPENDENT TESTING LAB TO PROVIDE SHOP WELD INSPECTIONS PER CODE. INSPECTION REPORTS SHALL BE SUBMITTED TO ENGINEER OF RECORD PRIOR TO STEEL INSTALLATION.

2. STEEL FRAMES: VERIFICATION OF BRACING, STIFFENING, MEMBER LOCATIONS, AND PROPER JOINT DETAIL APPLICATION AT ALL STEEL FRAME CONNECTIONS.

A. VERIFICATION OF SLIP CRITICAL BOLT INSTALLATION FOR ASTM A325 BOLTS.

WORKMANSHIP PROVISIONS OF THE CODE.

DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:

A. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED TO BE CERTAIN IT CONFORMS TO THE APPROVED DESIGN DRAWINGS AND SPECIFICATION. THE SPECIAL INSPECTOR IS NOT AUTHORIZED TO APPROVE DEVIATIONS FROM THE DESIGN Drawings or specifications, and all deviations must be approved by the structural ENGINEER OF RECORD AND/OR DSA PRIOR TO PROCEEDING WITH THE WORK. ALL REQUESTS FOR DEVIATIONS SHALL BE INITIATED BY THE CONTRACTOR VIA WRITTEN REQUEST FOR INFORMATION (RFI). C. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE DSA AND TO THE ENGINEER OR ARCHITECT OF RECORD. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF UNCORRECTED, TO THE DSA AND/OR THE ARCHITECT OR ENGINEER OF RECORD. CONTRACTOR SHALL PROVIDE THE SPECIAL INSPECTOR ACCESS TO ALL ITEMS REQUIRING SPECIAL INSPECTION. ACCESS SHALL BE PROVIDED BY IN-PLACE LADDERS. SCAFFOLDS. LIFTS AND/OR OTHER EQUIPMENT OPERATED BY THE CONTRACTOR'S PERSONNEL AS REQUIRED FOR SAFE OBSÉRVATION. INSPECTOR IS NOT RESPONSIBLE OR AUTHORIZED TO OPERATE CONTRACTOR'S E. UPON COMPLETION OF THE ASSIGNED WORK THE ENGINEER OR ARCHITECT SHALL COMPLETE

AND SIGN THE APPROPRIATE FORMS CERTIFYING THAT TO THE BEST OF THEIR KNOWLEDGE THE

WORK IS IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS, AND THE APPLICABLE

THE SOLAR PANELS AND THEIR ANCHORAGE SYSTEMS ARE DEFERRED ITEMS. PER TITLE 24, PART 1, SECTION 4-317 (g), THEIR DESIGNS SHALL BE REVIEWED AND APPROVED BY DSA PRIOR TO INSTALLATION. THE DEFERRED SUBMITTAL DOCUMENTS SHALL BE STAMPED AND SIGNED BY EITHER AN ARCHITECT OR REGISTERED ENGINEER WITH A VALID CALIFORNIA LICENSE. PLEASE NOTE THAT ADDITIONAL CANOPY FRAMING AND BEARING BLOCKS MAY BE REQUIRED FOR CONNECTING THE SOLAR PANEL ANCHORAGE SYSTEM TO THE CANOPY.

NOTES FOR SITE SPECIFIC PHOTOVOLTAIC (PV) INSTALLATION:

THESE DRAWINGS ARE FOR THE STEEL STRUCTURES SUPPORTING PV PANELS. NO PROVISIONS ARE INCLUDED IN THESE DRAWINGS FOR THE PV PANELS OR THE PV

PANEL INSTALLATION. THE PV PANELS AND THE PV PANEL INSTALLATION SHALL BE SUBMITTED AS A SITE SPECIFIC APPLICATION. (REFER TO THE BOX NOTE REGARDING THE SOLAR PANELS and their anchorage being a deferred item)

PV PANELS SHALL BE INSTALLED PER DRAWINGS THAT HAVE BEEN SUBMITTED TO AND REVIEWED/PERMITTED BY DSA. THE PV DRAWINGS SHALL PROVIDE THE MINIMUM . LOCATION ALL ELECTRICAL EQUIPMENT

WIRING DIAGRAMS TO AND FROM ALL PV PANELS AND ELECTRICAL EQUIPMENT.

EQUIPMENT WARNING LABELS FOR INVERTER OVER VOLTAGE. SINGLE 120 VOLT SUPPLY WITHOUT MULTI BRANCH CIRCUITS AND ELECTRICAL SHOCK HAZARD.

ALL GROUNDING DETAILS FOR STRUCTURES AND EQUIPMENT.

ALL DISCONNECTION LOCATIONS AND DETAILS.

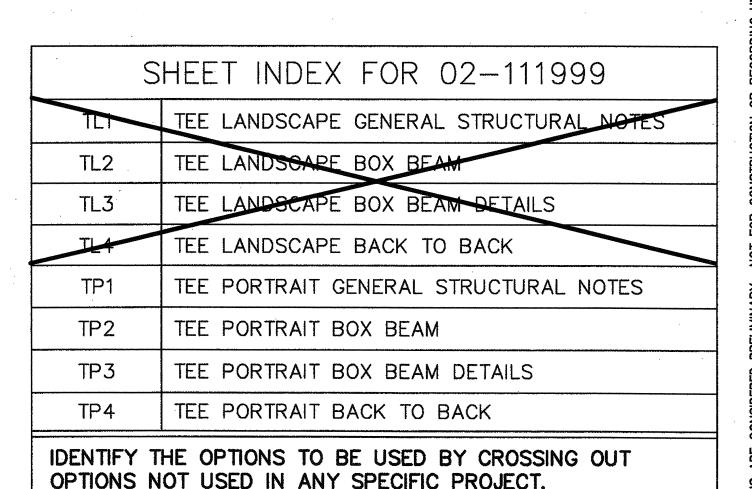
REFER TO CEC ARTICLE 690 FOR ADDITIONAL REQUIREMENTS AND DETAILS.

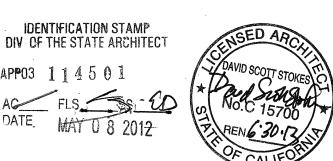
NOTE: PV SYSTEM SHALL BE MARKED. MARKING IS NEEDED TO PROVIDE EMERGENCY RESPONDERS WITH APPROPRIATE WARNING AND GUIDANCE WITH RESPECT TO ISOLATING THE SOLAR ELECTRIC SYSTEM. THIS CAN FACILITATE IDENTIFYING ENERGIZED ELECTRICAL LINES THAT CONNECT THE SOLAR PANELS TO THE INVERTER AND MAIN SERVICE DISCONNECT. THE LABEL SHALL BE OF A WEATHER-RESISTANT MATERIAL SUITABLE FOR THE ENVIRONMENT. MARKING CONTENT SHALL READ: "CAUTION: SOLAR ELECTRIC SYSTEM CONNECTED". THIS LABEL SHALL BE PLACED ADJACENT TO THE MAIN SERVICE DISCONNECT IN A LOCATION CLEARLY VISIBLE FROM THE LOCATION WHERE THE LEVER IS

ADDITIONAL MARKING IS REQUIRED OF THE DC CIRCUIT. MARKING IS REQUIRED ON ALL INTERIOR AND EXTERIOR DC CONDUIT, RACEWAYS, ENCLOSURES, CABLE ASSEMBLIES AND JUNCTION BOXES TO ALERT FIRE SERVICE TO AVOID CUTTING THEM. MARKING SHALL BE PLACED EVERY 10 FEET, AT TURNS AND ABOVE AND/OR BELOW PENETRATIONS AND AT ALL DC COMBINER AND JUNCTION BOXES. MARKING FOR CIRCUIT SHALL READ: "CAUTION: PATENTS PENDING

	GOVERNING LO	M MAX(K')	V MAX(K)	
	PURLIN	DL + 0.75W + 0.75Lr	4.05	0.68
	BEAM OF	DL + 0.73W + 0.73Lr	07.12	6.17
	BEAM 7P	DL + 0.75W + 0.75Lr	91.88	9.59
	COLUMN AND FOOTING STRONG AXIS 6P	(1 + .14 SDS) DL + 0.7pE	68.11	5.18
10.5' CLR.	COLUMN AND FOOTING STRONG AXIS 7P	(1 + .14 SDS) DL + 0.7pE	84.41	6.19
CLK.	COLUMN AND FOOTING WEAK AXIS 6P	(1 + .14 SDS) DL + 0.7pE	68.88	5.24
	COLUMN AND FOOTING WEAK AXIS 7P	(1 + .14 SDS) DL + 0.7pE	85.47	6.27
	COLUMN AND FOOTING STRONG AXIS 6P	(1 + .14 SDS) DL + 0.7pE	75.02	5.19
12'	COLUMN AND FOOTING STRONG AXIS 7P	(1 + .14 SDO) DL + 0.7pE	93.70	6.19
CLR.	COLUMN AND FOOTING WEAK AXIS OF	(1 + .14 SOS) DL + 0.7pE	76.86	5.25
	COLUMN AND FOOTING WEAK AXIS 7P	(1 + .14 SDS) DL + 0.7pE	94.80	6.27

6P = 6 PANELS, 7P = 7 PANELS





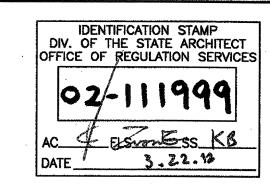


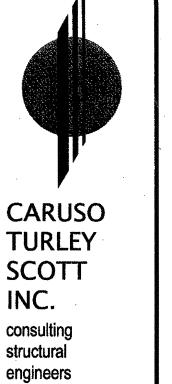
PATENTS PENDING

DOCUMENT CODE: 2010 CBC A SEPARATE PROJECT

PRE-CHECK (PC)

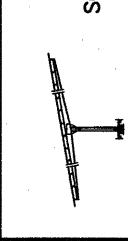
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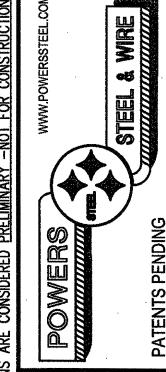




1215 W. Rio Salado Pkwy Suite 200 Tempe, Arizona 85281 (480) 774-1700 (480) 774-1701 FAX www.ctsaz.com

S S



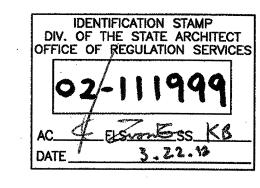


DRAWING EDITION/REF JOB :

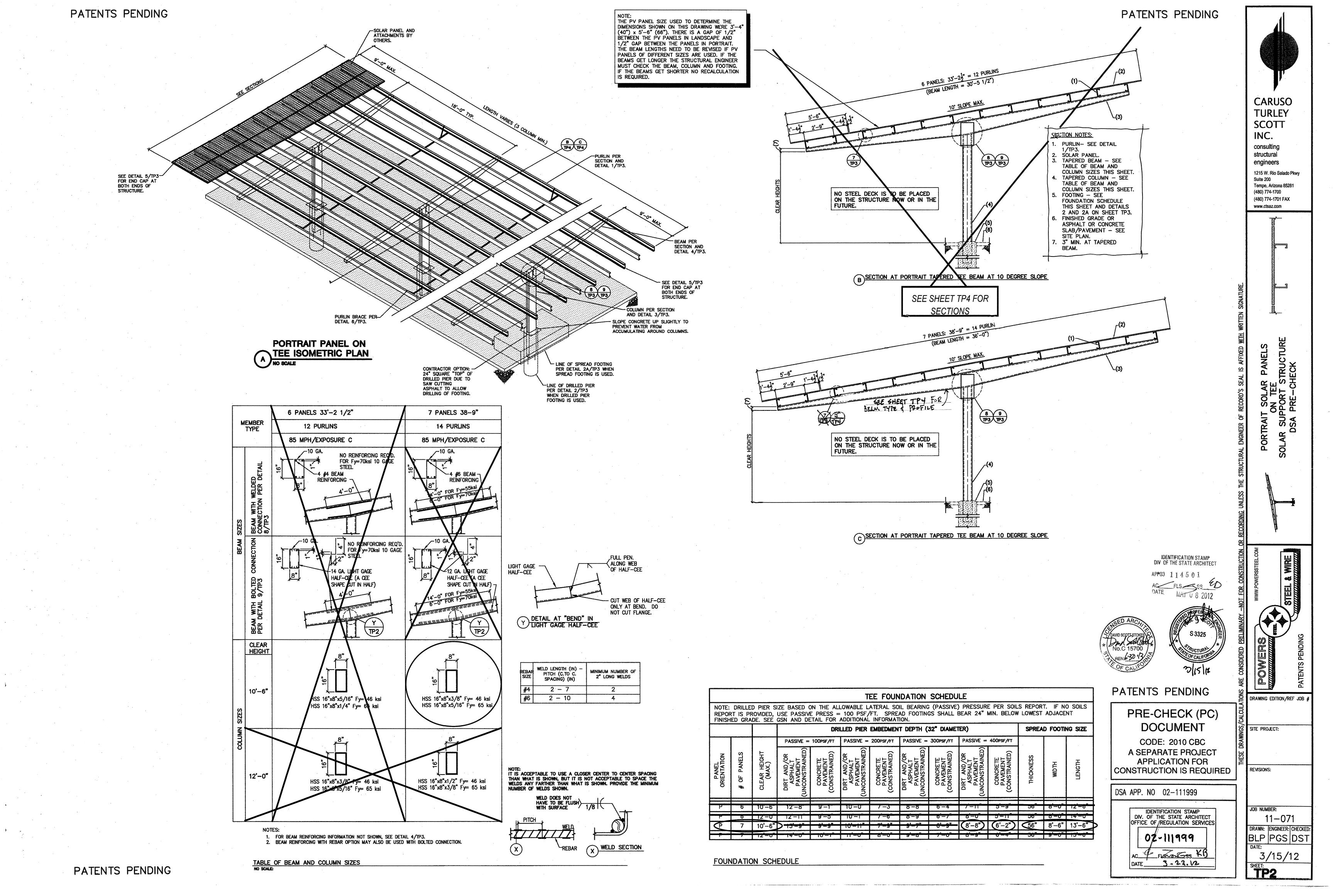
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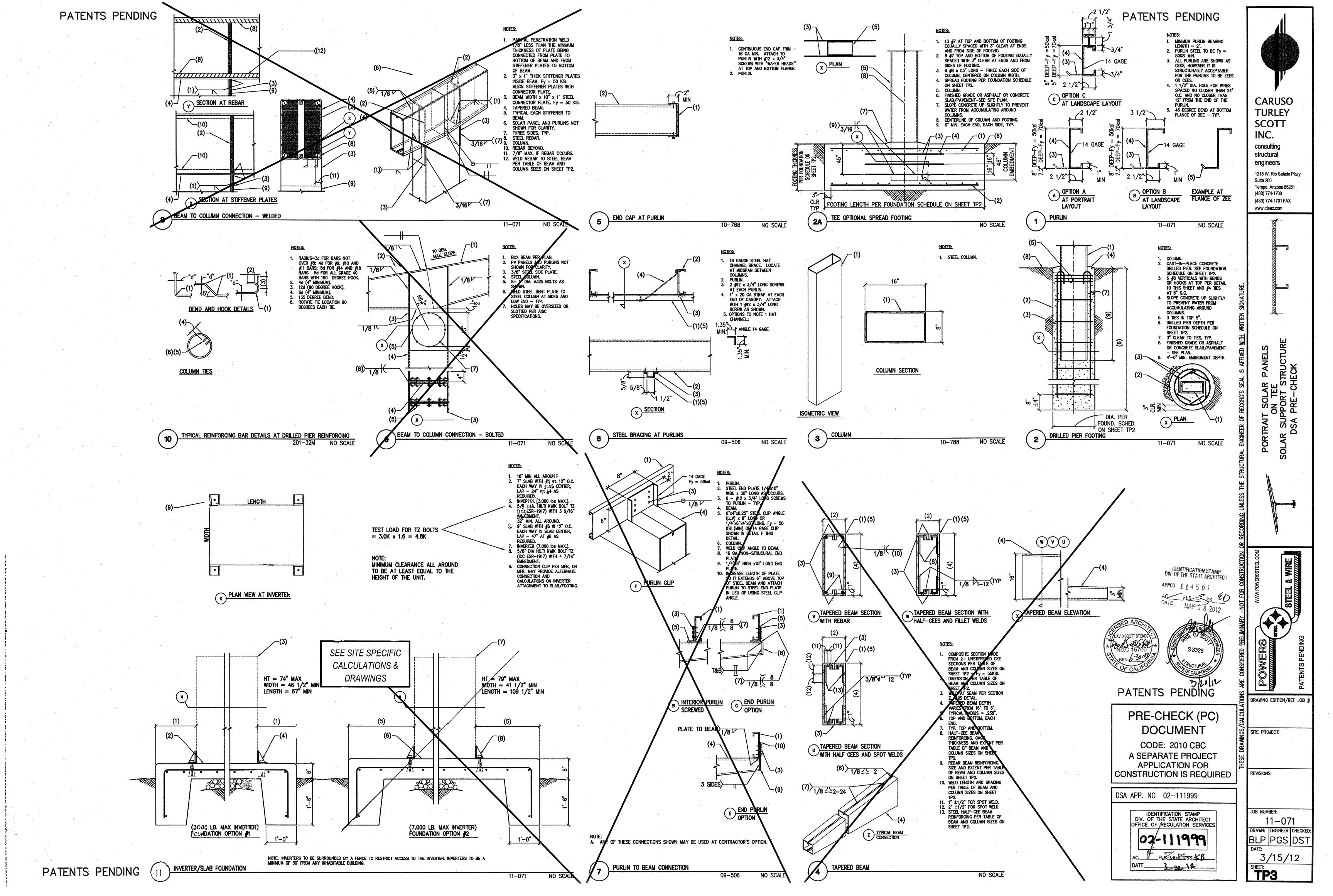
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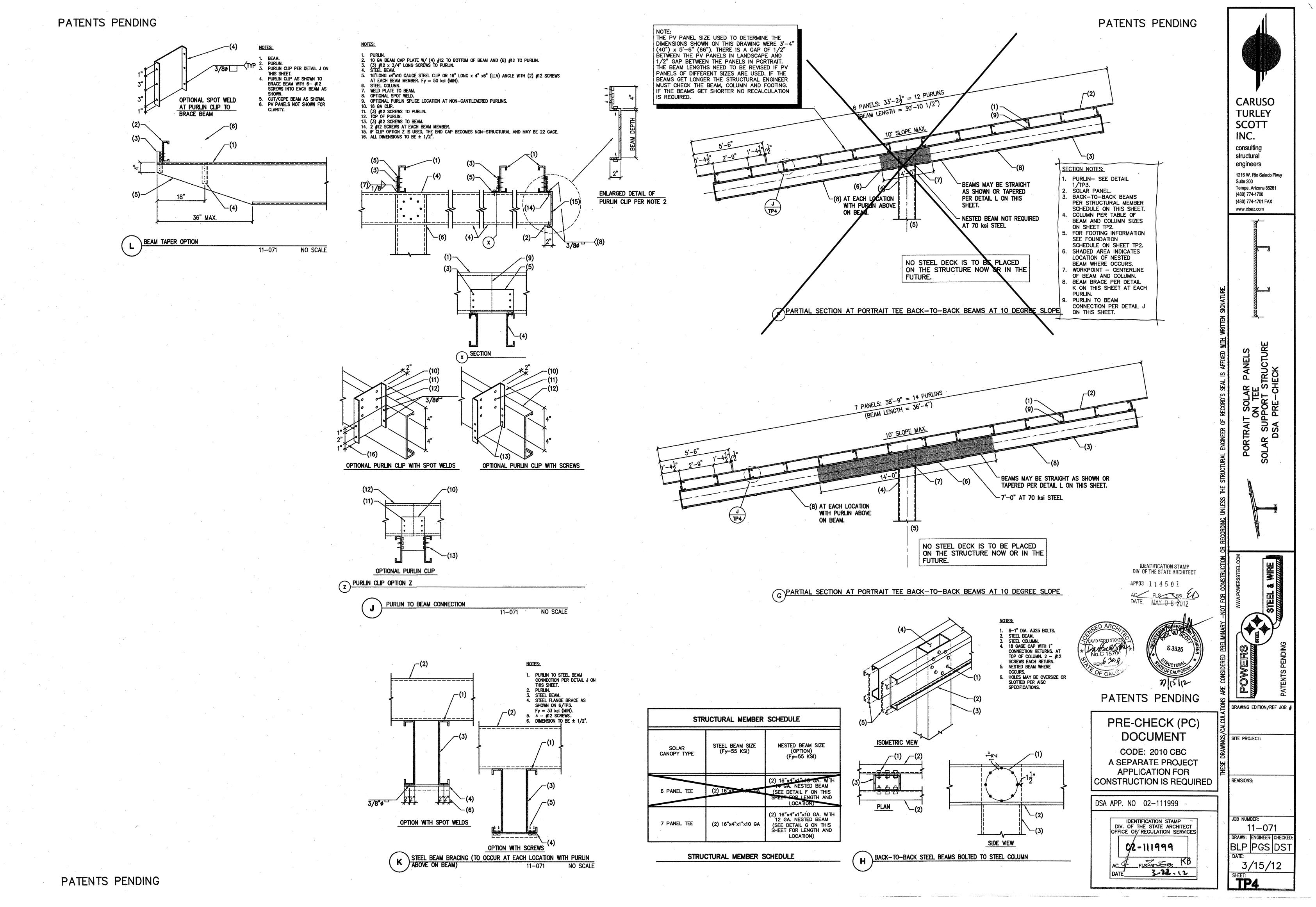
APPLICATION FOR CONSTRUCTION IS REQUIRED



JOB NUMBER: 11-071 DRAWN: ENGINEER: CHECKE BLP PGS DST 3/15/12 TP







2010 EDITION OF THE CALIFORNIA BUILDING CODE.

ROOF DEAD LOAD = ACTUAL WEIGHT OF MEMBER:

C&C WIND LOAD = 18.9 PSF (TOWARD THE SURFACE).

C&C WIND LOAD = 20.8 PSF (AWAY FROM THE SURFACE).

MWFRS WIND LOAD = 18.9 PSF / 4.4 PSF (TOWARD THE SURFACE).

MWFRS WIND LOAD = 17.8 PSF / 0.0 PSF (AWAY FROM THE SURFACE).

SOLAR PANEL = 3 PSF (MAX)

BE CHECKED AT BACKCHECK

II-B CONSTRUCTION

FOR 10 DEGREE ROOF SLOPE:

OCCUPANCY CATEGORY II

3 SECOND WIND GUST = 85 MPH.

WIND IMPORTANCE FACTOR = 1.0.

SEISMIC IMPORTANCE FACTOR = 1.0.

REDUNDANCY FACTOR p = 1.3.

SEISMIC DESIGN CATEGORY

Sds = 1.005 (MAX.).

Sd1 = 1.16 (MAX.)

FOUNDATIONS:

CONCRETE:

GENERAL:

SHORT PERIOD SPECTRAL ACCELERATION Ss = 2.85.

ONE SECOND SPECTRAL ACCELERATION S1 = 1.15.

RESPONSE MODIFICATION FACTOR (R)= 1.25.

DESIGN BASE SHEAR (3 PANEL) = 2690 LBS.

DESIGN BASE SHEAR (4 PANEL) = 3680 LBS.

1500 PSF SOILS (MINIMUM) ARE PRESENT AT SITE.

SPECIFIED 28 DAY COMPRESSIVE STRENGTH F'c:

LOADS:

OCCUPANCY GROUP PER SITE-SPECIFIC DOCUMENTS. ALLOWABLE

AREA AND MINIMUM SEPARATION BETWEEN STRUCTURES TO BE

DETERMINED AT EACH SPECIFIC LOCATION PER CBC WHICH IS TO

ROOF LIVE LOAD = 10 PSF. DESIGN FOR 300 POUND POINT LOAD LOCATED TO CAUSE MAXIMUM

NO STEEL DECK IS TO BE PLACED ON THE STRUCTURE - NOW OR IN THE FUTURE.

THIS DESIGN CAN BE USED FOR ANY ROOF SLOPE FROM 0 DEGREES TO 10 DEGREES.

BASIC SEISMIC-FORCE RESISTING SYSTEM = CANTILEVERED COLUMN SYSTEMS DETAILED TO

ALL FOOTINGS SHALL BE DESIGNED FOR THE SPECIFIC SITE. DRILLED PIER FOOTING DESIGNS ARE

LATERAL BEARING PRESSURE MAY BE MULTIPLIED BY 2.0 PER CBC SECTION 1806A.3.4. THE DRILLED

CZERNIAK, WHICHEVER IS DEEPER) WHERE PLACED IN ASPHALT PAVEMENT AREAS OR DIRT AREAS.

EXISTING GRADE. DESIGN SOIL BEARING VALUE = 1500 PSF. SOILS ENGINEER MUST VERIFY THAT

FOUNDATIONS ----- 3,000 PSI

ALL CAST-IN-PLACE CONCRETE CONSTRUCTION SHALL CONFORM TO THE LATEST EDITION OF THE

ADMIXTURES CONTAINING CHLORIDES SHALL NOT BE USED. NO OTHER ADMIXTURES PERMITTED

PLACEMENT U.N.O. IF PLASTICIZER IS USED, A HIGHER FINAL SLUMP MAY BE ALLOWED UPON

FOR REINFORCING INFORMATION, SEE REINFORCING SECTION OF G.S.N., PLANS, SCHEDULES AND

TEST DATA FOR EACH CONCRETE MIX SHALL BE SUBMITTED FOR REVIEW PER CHAPTER 5 OF ACI

THE FOOTINGS INDICATED ON THIS SHEET DO NOT APPLY WHERE ORGANIC FILL MATERIALS EXIST.

CONCRETE SHALL BE ADEQUATELY VIBRATED AROUND THE EMBEDDED STEEL COLUMNS TO ENSURE

THE CONCRETE HAS COMPLETELY SURROUNDED THE STEEL COLUMN AND TO ENSURE THE CONCRETE

318. REFERENCE FIGURE R5.3 FOR SUBMITTAL REQUIREMENTS AND OPTIONS. CONCRETE MIX

DESIGNS THAT ARE SUBMITTED WITHOUT THE APPROPRIATE TEST DATA CANNOT BE REVIEWED.

IT IS ACCEPTABLE AND INTENDED TO USE EARTH CUTS FOR THE DRILLED PIER FOOTING AND SPREAD FOOTING. THE FOOTING DESIGNS INDICATED ON THIS SHEET DO NOT APPLY IF THE EARTH

AT THE INSIDE OF THE STEEL COLUMN HAS RISEN TO THE LEVEL OF THE CONCRETE IN THE

REMAINDER OF THE DRILLED PIER OR SPREAD FOOTING. CONCRETE SHALL SLOPE UP SLIGHTLY

ALL REINFORCING PER CRSI SPECIFICATIONS AND HANDBOOK. ASTM A615 (Fy = 60 KSI / GRADE

REINFORCING TO BE WELDED SHALL BE ASTM A706. NO TACK WELDING OF REINFORCING BARS

ALLOWED WITHOUT PRIOR REVIEW OF PROCEDURE WITH THE STRUCTURAL ENGINEER. LATEST ACI

60) DEFORMED BARS FOR ALL BARS. WHERE SHOWN ON DRAWINGS ALL GRADE 60

CODE AND DETAILING MANUAL APPLY. CLEAR CONCRETE COVERAGES AS FOLLOWS:

CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH ----- 3"

CONCRETE CHAIR. DUCT-TAPE COVERED REINFORCING IS NOT AN ACCEPTABLE CHAIR.

#6 OR LARGER ----- 2

5 AND SMALLER ----- 1 1/2"

ALL REINFORCING SHALL BE CHAIRED TO ENSURE PROPER CLEARANCES. SUPPORT OF FOUNDATION

ALL DIMENSIONS REFERENCED IN DRAWINGS AS "CLEAR" SHALL BE FROM FACE OF STRUCTURE TO

SMALLER AND SHALL BE FIELD BENT OR STRAIGHTENED ONLY ONCE. ANY BEND SHALL BE LIMITED

TO 90 DEGREES. IF FIELD BENDING OR STRAIGHTENING OF #6 BARS OR LARGER IS REQUIRED, OR IF

A SECOND BEND IS REQUIRED FOR #5 BARS AND SMALLER, HEAT SHALL BE APPLIED FOR BENDING

OR STRAIGHTENING. CONTRACTOR SHALL SUBMIT PROCEDURE FOR APPLYING HEAT TO ENGINEER

REINFORCING MUST PROVIDE ISOLATION FROM MOISTURE/CORROSION BY USE OF A PLASTIC OR

EDGE OF REINFORCING, AND SHALL NOT BE LESS THAN STATED, NOR GREATER THAN "CLEAR" DIMENSION PLUS 3/8". ALL OTHERS SHALL BE PLUS OR MINUS 1/4" TYPICAL UNLESS NOTED

FIELD BENDING OR STRAIGHTENING OF DEFORMED BARS SHALL BE LIMITED TO #5 BARS AND

WITHOUT APPROVAL. FOR CONCRETE WITHOUT PLASTICIZER, MAXIMUM SLUMP 4 1/2" AT POINT OF

ACI. MECHANICALLY VIBRATE ALL CONCRETE WHEN PLACED UNLESS NOTED OTHERWISE.

FLY ASH - SHALL BE LIMITED TO 50% OF TOTAL CEMENTITIOUS MATERIALS BY WEIGHT.

CUTS ARE UNSTABLE AND/OR DO NOT STAND ON THEIR OWN.

TOWARDS COLUMNS TO PREVENT WATER FROM PONDING AROUND COLUMNS.

IT IS ACCEPTABLE FOR CONCRETE TO FREE FALL INTO FOOTINGS.

SPREAD FOOTING DESIGNS ARE BASED ON CBC SECTION 1806A, CLASS 5 SOILS. SPREAD

FOOTINGS SHALL BEAR ON FIRM, UNDISTURBED SOIL 2 FEET MINIMUM BELOW ADJACENT

PIER FOOTINGS ARE DESIGNED AS CONSTRAINED (SECTION 1807A.3.2.2, EQUATION 18A-2) WHERE PLACED

IN A CONCRETE PAVEMENT AREA AND AS UNCONSTRAINED (SECTION 1807A.3.2.2, EQUATION 18A-1 OR

BASED ON THE ALLOWABLE LATERAL BEARING PRESSURES SHOWN IN DETAIL 2. THE ALLOWABLE

CONFORM TO THE REQUIREMENTS FOR ORDINARY STEEL MOMENT FRAMES.

ANALYSIS PROCEDURE USED = EQUIVALENT LATERAL FORCE PROCEDURE

MOMENTS AND SHEAR. USE THE 300 POUND LOAD WITH WND, BUT NOT WITH 10 PSF ROOF LIVE LOAD.

ANSI-----AMERICAN NATIONAL STANDARDS APA ---- AMERICAN PLYWOOD ASSOCIATION ARCH'L --- -- ARCHITECTURAL ASTM -----AMERICAN SOCIETY FOR TESTING AND MATERIALS

AWS ---- AMERICAN WELDING SOCIETY A.W.T.S. ----AUTOMATIC WELDED THREADED BM ----BEAM B.F.F ----BELOW FINISHED FLOOR BLK----BLOCK B.O.B. ----BOTTOM OF BEAM B.O.D. ----BOTTOM OF DECK B.O.F. ----BOTTOM OF FOOTING BRG ----BEARING

C -----CAMBER C.C. -----CENTERLINE TO CENTERLINE C & C ---- COMPONENTS & CLADDING CBC -----CALIFORNIA BUILDING CODE CFS -----COLD FORMED STEEL C.G. -----CENTER OF GRAVITY C.I.P. ---- CAST IN PLACE C.L.----CENTERLINE C.I.B. -----CENTERLINE OF BEAM C.L.C. -----CENTERLINE OF COLUMN

C.L.F. -----CENTERLINE OF FOOTING C.L.W. -----CENTERLINE OF WALL CLR-----CLEAR CONC -----CONCRETE CONC C.J. ---- CONCRETE CONTROL JOINT CONC S.J. ----CONCRETE SAWCUT JOINT C.M.U. - - - - - CONCRETE MASONRY UNIT CONN -----CONNECTION

CONT -----CONTINUOUS CRSI-----CONCRETE REINFORCING STEEL INSTITUTE D.F. (D.F.L.) — — DOUGLAS FIR LARCH DL -----DEAD LOAD DIA ---- DIAMETER DN -----DOWN DSA ---- DIVISION OF STATE ARCHITECT

DWG(S) - - - - DRAWING(S)E.C. -----END TO CENTERLINE E.E. -----END TO END E.O.S. -----EDGE OF SLAB EQ ----EQUAL EQUIP - - - - EQUIPMENT EXP. BOLT (E.B.) — EXPANSION BOLT EXP. JT (E.J.) — EXPANSION JOINT E.W.----EACH WAY

F.F. - - - - - FINISHED FLOOR F.O.M. ----FACE OF MEMBER F.O.S. ----FACE OF STEEL F.O.W. ----FACE OF WALL GA -----GAGE (UNIT OF MEASUREMENT) GALV -----GALVANIZED G.S.N. ----GENERAL STRUCTURAL NOTES GLB (GLULAM) — GLUED-LAMINATED BEAM H.F. ----HEM FIR

HORIZ - - - - - HORIZONTAL REINFORCING H.S. ----HEADED STUDS IBC ---- INTERNATIONAL BUILDING CODE ICBO ---- INTERNATIONAL CONFERENCE OF BUILDING OFFICIALS ICC -----INTERNATIONAL CODE COUNCIL I.F.W. -----INSIDE FACE OF WALL I.O.D.——————INTERPRETATION OF DRAWING

JST ---- JOIST K(KIP) -----1000 POUNDS KLF ---- KIPS PER LINEAR FOOT LBS (#) ---- POUNDS LGR ————— LEDGER LGS -----LIGHT GAGE STEEL LGSEA -----LIGHT GAGE STEEL ENGINEERS ASSOCIATION

LO.D. ----- LOCATION OF DETAILS LL ----LIVE LOAD LLH -----LONG LEG HORIZONTAL LLV -----LONG LEG VERTICAL MAS ---- MASONRY MAS C.J. -- -- MASONRY CONTROL JOINT MAX ---- MAXIMUM MBMA---- METAL BUILDING MANUFACTURERS ASSOCIATION

MECH'L -- -- -- MECHANICAL MFR'D ---- MANUFACTURED MFR('S) ---- MANUFACTURER('S) MIN '----- MINIMUM MWFRS ---- MAIN WIND FORCE RESISTANCE SYSTEM

N/A ----NOT APPLICABLE N.T.S. -----NOT TO SCALE Q.C. ----ON CENTER O.F.W.----OUTSIDE FACE OF WALL OPP ---- OPPOSITE OSHA ---- OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION

PCI -----PRECAST/PRESTRESSED CONCRETE INSTITUTE P.C. — — — PRECAST CONCRETE PCF ---- POUNDS PER CUBIC FOOT PLF ---- POUNDS PER LINEAR FOOT ± ----PLUS OR MINUS PREFAB — — — — PREFABRICATED PSF-----POUNDS PER SQUARE FOOT PSI -----POUNDS PER SQUARE INCH PT ----POST-TENSIONED

SSMA -----STEEL STUD MANUFACTURERS

T.O.C.T. ---- TOP OF CONCRETE TOPPING

T.O.P.C. ---- TOP OF PRECAST CONCRETE

TPI ---- TRUSS PLATE INSTITUTE

T&G ---- TONGUE AND GROOVE

UBC ---- UNIFORM BUILDING CODE

VERT - - - - VERTICAL REINFORCING

W.W.F.---- WELDED WIRE FABRIC

U.N.O.---- UNLESS NOTED OTHERWISE

WWPA---- WESTERN WOOD PRODUCTS

W/C ---- WATER TO CEMENT RATIO

WCLA ---- WEST COAST LUMBER ASSOCIATION

WCLIB----- WEST COAST LUMBER INSPECTION

BUREAU

ASSOCIATION

ASSOCIATION

REINF ----- REINFORCING

SIM ---- SIMILAR

SQ. -----SQUARE

STL ---- STEEL

STD-----STANDARD

TL ----TOTAL LOAD

T.O.B. ---- TOP OF BEAM

T.O.D. — — — TOP OF DECK

T.O.S.————— TOP OF STEEL

T.O.W.----TOP OF WALL

TYP ---- TYPICAL

W/ ————— WITH

W/0 ---- WITHOUT

T.O.F. — — — TOP OF FOOTING

T.O.I. - - - TOP OF LEDGER

T.O.M.---- TOP OF MASONRY T.O.P. - - - TOP OF PLATE

PTI -----POST-TENSIONING INSTITUTE SDI -----STEEL DECK INSTITUTE SLH - - - - - SHORT LEG HORIZONTAL SLV-----SHORT LEG VERTICAL SJI ----STEEL JOIST INSTITUTE

GENERAL: ALL CONSTRUCTION PER LATEST AISC STEEL CONSTRUCTION

FOR REVIEW AND APPROVAL PRIOR TO BENDING OR STRAIGHTENING BARS.

ALL CONSTRUCTION PER LATEST AISC STEEL CONSTRUCTION MANUAL. ALL WIDE FLANGE STEEL SHALL BE ASTM A992 (Fy = 50 KSI). ALL PIPE STEEL SHALL BE ASTM A500 (Fy = 42 KSI) OR ASTM A53, TYPE E OR S, GRADE B (Fy = 35 KSI). ALL MISCELLANEOUS STEEL UNLESS NOTED OTHERWISE SHALL BE ASTM A36 (Fy = 36 KSI). IF CALLED OUT ON PLANS, Fy = 50 KSI PLATE STEEL SHALL BE ASTM A529 OR A572.

ALL STRUCTURAL ROLLED STEEL MEMBERS WITH FY GREATER THAN 36 KSI ARE TO BE IDENTIFIED WITH AN ASTM SPECIFICATION MARK OR TAG PER IBC SEC. 2203.1.

HOLLOW STRUCTURAL SHAPE (HSS):

INSPECTION) IS REQUIRED.

STRUCTURAL STEEL:

EXPOSED TO EARTH OR WEATHER

ALL OTHER PER LATEST EDITION OF ACI 318

HSS COLUMNS ARE CALLED OUT ON THE DRAWINGS AS EITHER ASTM A500 (Fy = 46 KSI) OR ASTM A572 (Fy = 65 KSI).

ASTM A500 (Fy = 46 KSI) HSS SECTIONS ARE TO BE PRODUCED PER THE SPECIFICATIONS SET ASTM A572 (Fy = 65 KSI) HSS SECTIONS ARE TO BE PRODUCED BY DIRECT-FORMING OR FOLDING OF THE PLATE FOLLOWED BY AN ELECTRIC RESISTANCE WELD ALONG THE SEAM. INLINE INSPECTION

OF THE WELD ZONE DURING PRODUCTION BY NON-DESTRUCTIVE TESTING (NDT) (ULTRASONIC

THE TERMS PIPE AND ROUND HOLLOW STRUCTURAL SHAPE (HSS) ARE USED SYNONYMOUSLY THROUGHOUT THESE DOCUMENTS ALONG WITH THE TERMS TUBE STEEL AND RECTANGULAR OR

BOLTS:

ALL BOLTS SHALL BE ASTM A325 AND SHALL BE INSTALLED AS SLIP CRITICAL CONNECTIONS WITH THREADS INCLUDED IN SHEAR PLANE. TIGHTEN BOLTS PER AISC SPECIFICATIONS. IT IS ACCEPTABLE TO USE OVERSIZE HOLES OR SLOTTED HOLES PER AISC SPECIFICATIONS. PATENTS PENDING

UNLESS NOTED OTHERWISE. ALL WELDS PER LATEST EDITION OF THE AWS STANDARDS. ALL WELDING SHALL BE PERFORMED BY WELDERS HOLDING VALID CERTIFICATES AND HAVING CURRENT EXPERIENCE IN THE TYPE OF WELD SHOWN ON THE DRAWINGS OR NOTES. CERTIFICATES SHALL BE THOSE ISSUED BY AN ACCEPTED TESTING AGENCY. ALL WELDING DONE BY E70 SERIES LOW HYDROGEN RODS UNLESS NOTED OTHERWISE. FOR GRADE 60 REINFORCING BARS, USE E90 SERIES. THESE DRAWINGS DO NOT DISTINGUISH BETWEEN SHOP AND FIELD WELDS: THE CONTRACTOR MAY SHOP WELD OR FIELD WELD AT THEIR DISCRETION. SHOP WELDS AND FIELD WELDS SHALL BE SHOWN ON THE SHOP DRAWINGS SUBMITTED FOR REVIEW.

ALL FULL (COMPLETE) PENETRATION WELDS SHALL BE TESTED AND CERTIFIED BY AN INDEPENDENT TESTING LABORATORY.

ALL SPOT WELDS SHALL BE PER LATEST AISI AND AWS STANDARDS.

STEEL CONNECTORS:

FRAMING:

SCREW FASTENERS:

ALL STEEL SCREWS SHALL BE IN ACCORDANCE WITH AISI-GENERAL AND AISI-NAS. Fy = 50 ksi AND Ft = 70 ksi FOR ALL SCREWS.

1. MINIMUM SPACING OF SCREWS SHALL NOT BE LESS THAN 3 TIMES THE NOMINAL DIAMETER. MINIMUM EDGE DISTANCE FOR SCREWS SHALL NOT BE LESS THAN 1.5 TIMES THE NOMINAL SCREW 2. THE HEAD OF THE SCREW OR WASHER SHALL HAVE A DIAMETER, DW, OF NOT LESS THAN 5/16". WASHERS SHALL BE AT LEAST 0.05" THICK.

SCREW NUMBER DESIGNATION	8	10	12 (12–14)	14
NOMINAL DIAMETER	0.164"	0.190"	0.216"	0.250*

COLD FORMED STRUCTURAL STEEL FRAMING: GENERAL:

ALL COLD FORMED STEEL COMPONENTS INDICATED ON THE STRUCTURAL DRAWINGS SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND IN ACCORDANCE WITH THE LATEST EDITION OF "SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" BY THE AISI.

ALL STRUCTURAL STEEL FRAMING MATERIAL AND ITS ERECTION SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE AMERICAN IRON AND STEEL INSTITUTE "SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBER".

ALL WELDING TO BE PERFORMED BY WELDERS HOLDING A VALID CERTIFICATE AND HAVING CURRENT EXPERIENCE IN LIGHT GAUGE STEEL. CERTIFICATES SHALL BE ISSUED BY AN ACCEPTED TESTING AGENCY. DO NOT NOTCH FLANGES OF MEMBERS WITHOUT EXPRESSED APPROVAL OF THE ENGINEER OF RECORD. ALL WELDING TO BE PERFORMED IN AN APPROVED FABRICATORS SHOP.

STRUCTURAL STEEL MEMBERS ARE FURNISHED TO A SPECIFIED MINIMUM Fy = 55,000 PSI. U.N.O. THE GRADE AND THE ASTM SPECIFICATION NUMBER OR OTHER SPECIFICATION DESIGNATION SHALL BE INDICATED BY PAINTING, DECAL, TAGGING OR OTHER SUITABLE MEANS ON EACH BUNDLE OF FABRICATED ELEMENTS. IT IS ACCEPTABLE TO USE THE FY SHOWN ON THE MILL CERTIFICATION IN LIEU OF THE "ORDERED" Fy. IT IS ACCEPTABLE TO USE STEEL WITH Fy = 70 KSI IF THE STEEL USED IS IN THE AISI AND/OR AISC SPECIFICATION, THE ELONGATION IN A 2" COUPON IS A MINIMUM OF 10% AND THE RATIO OF Ft OVER FY IS AT LEAST 1.08.

	MILS	GAGE NO.	MIN DELIVERED THICKNESS	DESIGN THICKNESS
	12	30	0.0120*	0.0126"
	14	29	0.0132*	0.0139"
	16	26	0.0174"	0.0183"
	33	20	0.0336"	0.0354*
	43	18	0.0447"	0.0470*
	54	16	0.0561"	0.0590"
	- 68	14	0.0713"	0.0750"
	97	12	0.0998"	0.1050"
•	118	10	0.1283"	0.1350*

GENERAL NOTES:

THE STRUCTURAL CONSTRUCTION DOCUMENTS REPRESENT THE FINISHED STRUCTURE. EXCEPT WHERE NOTED, THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. THE STRUCTURAL ENGINEER OF RECORD SHALL NOT E responsible for the contractor's means, methods, techniques, sequences for procedure OF CONSTRUCTION, OR THE SAFETY PRECAUTIONS AND THE PROGRAMS INCIDENT THERETO (NOR SHALL OBSERVATION VISITS TO THE SITE INCLUDE INSPECTION OF THESE ITEMS).

WHERE REFERENCE IS MADE TO VARIOUS TEST STANDARDS FOR MATERIALS, SUCH STANDARDS SHALL BE THE LATEST EDITION AND/OR ADDENDA. ANY ENGINEERING DESIGN, PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW, SHALL BEAR THE SEAL OF A REGISTERED ENGINEER RECOGNIZED BY THE BUILDING CODE JURISDICTION OF THIS PROJECT.

NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT, AND/OR AS PROVIDED FOR IN THE CONTRACT DOCUMENTS. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL STRUCTURAL NOTES AND SPECIFICATIONS, THE GREATER REQUIREMENTS SHALL GOVERN.

CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION OF ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION. RESOLVE ANY DISCREPANCY WITH THE ARCHITECT. ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING AND ELECTRICAL ITEMS WITH THE APPROPRIATE TRADE DRAWINGS AND SUBCONTRACTORS

TYPICAL DETAILS MAY NOT NECESSARILY BE CUT ON PLANS, BUT APPLY UNLESS NOTED OTHERWISE. CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED CONSTRUCTION. LOAD SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT.

OPTIONS ARE FOR CONTRACTOR'S CONVENIENCE. IF AN OPTION IS CHOSEN, CONTRACTOR SHALL BE RESPONSIBLE FOR ALL NECESSARY CHANGES, APPROVALS AND THE COORDINATION OF THE WORK WITH ALL RELATED TRADES AND SUPPLIERS.

SPECIAL INSPECTION - STRUCTURAL ONLY:

SPECIAL INSPECTION IS REQUIRED PER CHAPTER 17A OF THE CBC FOR THE FOLLOWING:

CONCRETE CONSTRUCTION:

DURING THE TAKING OF TEST SPECIMENS. B. THE PLACEMENT OF ALL FOUNDATION CONCRETE.

2. REINFORCING STEEL: INSPECTION OF IN-PLACE REINFORCING FOR CONFORMANCE PRIOR TO THE CLOSING OF FORMS OR THE DELIVERY OF CONCRETE TO THE JOBSITE FOR THE FOLLOWING: A. REINFORCING FOR SPREAD FOOTING AND DRILLED PIER CONCRETE FOUNDATIONS. B. REINFORCING FOR INVERTER SLABS ON THE GROUND.

STEEL CONSTRUCTION:

. WELDING: A. PERIODIC VISUAL INSPECTION OF ALL FIELD WELDS. . CONTINUOUS INSPECTION OF ALL MULTIPASS FILLET WELDS OR SINGLE PASS FILLET WELDS Larger than 5/16". C. NON-DESTRUCTIVE TESTING OF ALL COMPLETE PENETRATION WELDS BY AN AWS CERTIFIED INDEPENDENT TESTING LABORATORY AT THE CONTRACTORS EXPENSE.

VERIFICATION OF VALID WELDER'S CERTIFICATES. . ALL STRUCTURAL STEEL FABRICATORS SHALL EMPLOY AN AWS CERTIFIED INDEPENDENT TESTING LAB TO PROVIDE SHOP WELD INSPECTIONS PER CODE. INSPECTION REPORTS SHALL BE SUBMITTED TO ENGINEER OF RECORD PRIOR TO STEEL INSTALLATION.

2. STEEL FRAMES: VERIFICATION OF BRACING, STIFFENING, MEMBER LOCATIONS, AND PROPER JOINT DETAIL APPLICATION AT ALL STEEL FRAME CONNECTIONS.

A. VERIFICATION OF SLIP CRITICAL BOLT INSTALLATION FOR ASTM A325 BOLTS. DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:

A. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED TO BE CERTAIN IT CONFORMS TO THE APPROVED DESIGN DRAWINGS AND SPECIFICATION. B. THE SPECIAL INSPECTOR IS NOT AUTHORIZED TO APPROVE DEVIATIONS FROM THE DESIGN DRAWINGS OR SPECIFICATIONS, AND ALL DEVIATIONS MUST BE APPROVED BY THE STRUCTURAL ENGINEER OF RECORD AND/OR DSA PRIOR TO PROCEEDING WITH THE WORK. ALL REQUESTS FOR DEVIATIONS SHALL BE INITIATED BY THE CONTRACTOR VIA WRITTEN REQUEST FOR INFORMATION (RFI). C. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE DSA AND TO THE ENGINEER OR ARCHITECT OF RECORD. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF UNCORRECTED, TO THE DSA AND/OR THE ARCHITECT OR ENGINEER OF RECORD. . CONTRACTOR SHALL PROVIDE THE SPECIAL INSPECTOR ACCESS TO ALL ITEMS REQUIRING SPECIAL INSPECTION. ACCESS SHALL BE PROVIDED BY IN-PLACE LADDERS, SCAFFOLDS, LIFTS AND/OR OTHER EQUIPMENT OPERATED BY THE CONTRACTOR'S PERSONNEL AS REQUIRED FOR SAFE OBSERVATION. INSPECTOR IS NOT RESPONSIBLE OR AUTHORIZED TO OPERATE CONTRACTOR'S FOUIPMENT. E. UPON COMPLETION OF THE ASSIGNED WORK THE ENGINEER OR ARCHITECT SHALL COMPLETE
AND SIGN THE APPROPRIATE FORMS CERTIFYING THAT TO THE BEST OF THEIR KNOWLEDGE THE
WORK IS IN CONFORMACE WITH APPROVED PLANS AND SPECIFICATIONS, AND THE APPLICABLE

THE SOLAR PANELS AND THEIR ANCHORAGE SYSTEMS ARE DEFERRED ITEMS. PER TITLE 24, PART 1, SECTION 4-317 (g), THEIR DESIGNS SHALL BE REVIEWED AND APPROVED BY DSA PRIOR TO INSTALLATION. THE DEFERRED SUBMITTAL DOCUMENTS SHALL BE STAMPED AND SIGNED BY EITHER AN ARCHITECT OR REGISTERED ENGINEER WITH A VALID CALIFORNIA LICENSE. PLEASE NOTE THAT ADDITIONAL CANOPY FRAMING AND BEARING BLOCKS MAY BE REQUIRED FOR CONNECTING THE SOLAR PANEL ANCHORAGE SYSTEM TO THE CANOPY.

NOTES FOR SITE SPECIFIC PHOTOVOLTAIC (PV) INSTALLATION:

THESE DRAWINGS ARE FOR THE STEEL STRUCTURES SUPPORTING PV PANELS. NO PROVISIONS ARE INCLUDED IN THESE DRAWINGS FOR THE PV PANELS OR THE PV PANEL INSTALLATION. THE PV PANELS AND THE PV PANEL INSTALLATION SHALL BE SUBMITTED AS A SITE

SPECIFIC APPLICATION. (REFER TO THE BOX NOTE REGARDING THE SOLAR PANELS AND THEIR ANCHORAGE BEING A DEFERRED ITEM). PV PANELS SHALL BE INSTALLED PER DRAWINGS THAT HAVE BEEN SUBMITTED TO AND REVIEWED/PERMITTED BY DSA. THE PV DRAWINGS SHALL PROVIDE THE MINIMUM

LOCATION ALL ELECTRICAL EQUIPMENT.
WRING DIAGRAMS TO AND FROM ALL PV PANELS AND ELECTRICAL EQUIPMENT. ALL GROUNDING DETAILS FOR STRUCTURES AND EQUIPMENT. ALL DISCONNECTION LOCATIONS AND DETAILS.
EQUIPMENT WARNING LABELS FOR INVERTER OVER VOLTAGE. SINGLE 120 VOLT

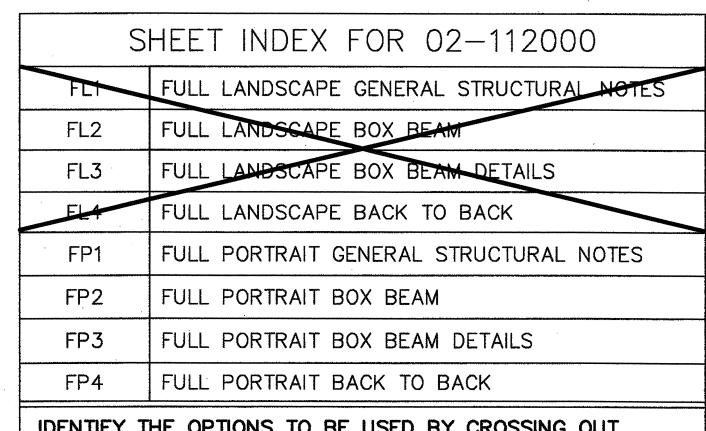
SUPPLY WITHOUT MULTI BRANCH CIRCUITS AND ELECTRICAL SHOCK HAZARD. REFER TO CEC ARTICLE 690 FOR ADDITIONAL REQUIREMENTS AND DETAILS.

NOTE: PV SYSTEM SHALL BE MARKED. MARKING IS NEEDED TO PROVIDE EMERGENCY RESPONDERS WITH APPROPRIATE WARNING AND GUIDANCE WITH RESPECT TO ISOLATING HE SOLAR ELECTRIC SYSTEM. THIS CAN FACILITATE IDENTIFYING ENERGIZED ELECTRICAL LINES THAT CONNECT THE SOLAR PANELS TO THE INVERTER AND MAIN SERVICE DISCONNECT. THE LABEL SHALL BE OF A WEATHER-RESISTANT MATERIAL SUITABLE FOR THE ENVIRONMENT. MARKING CONTENT SHALL READ: "CAUTION: SOLAR ELECTRIC SYSTEM CONNECTED". THIS LABEL SHALL BE PLACED ADJACENT TO THE MAIN SERVICE DISCONNECT IN A LOCATION CLEARLY VISIBLE FROM THE LOCATION WHERE THE LEVER IS

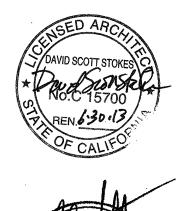
ADDITIONAL MARKING IS REQUIRED OF THE DC CIRCUIT. MARKING IS REQUIRED ON ALL INTERIOR AND EXTERIOR DC CONDUIT, RACEWAYS, ENCLOSURES, CABLE ASSEMBLIES AND JUNCTION BOXES TO ALERT FIRE SERVICE TO AVOID CUTTING THEM. MARKING SHALL BE PLACED EVERY 10 FEET, AT TURNS AND ABOVE AND/OR BELOW PENETRATIONS AND AT ALL DC COMBINER AND JUNCTION BOXES. MARKING FOR CIRCUIT SHALL READ: "CAUTION: SOLAR CIRCUIT".

1				
	GOVERNING LO	M MAX(K')	V MAX(K)	
PURLIN		DL + 0.75W + 0.75Lr	4.05	0.68
	BEAM 3P	DL + 0.75W + 0.75Li	50.53	0.73
	BEAM 4P	DL + 0.75W + 0.75Lr	99.07	9.65
	COLUMN AND FOOTING STRONG AXIS 3P	DL + 0.75W (MWFRS) + 0.75Lr	56.75	2.68
10.5'	COLUMN AND FOOTING STRONG AXIS 4P	DL + 0.75W (MWFRS) + 0.75Lr	104.46	3.64
CLR.	COLUMN AND FOOTING WEAK AXIS 3P	(1 + .14 SDS) DL + 0.7pE	37.80	2.69
	COLUMN AND FOOTING WEAK AXIS 4P	(1 + .14 SDS) DL + 0.7pE	56.71	3.67
	CULUMN AND FOOTING STRONG AXIS 3P	DL + 0.75W (MWFRS) + 0.75Lr	57.84	2.68
12'	COLUMN AND FOOTING STRONG AXIS 4P	DL + 0.75W (MWFRS) + 0.75Lr	106.04	3.65
CLR.	COLUMN AND FOOTING WEAK AXIS SP	(1 + .14 SBS) DL + 0.7pE	41.95	2.69
	COLUMN AND FOOTING WEAK AXIS 4P	(1 + .14 SDS) DL + 0.7pE	62.39	3 68

3P = 3 PANELS, 4P = 4 PANELS



IDENTIFY THE OPTIONS TO BE USED BY CROSSING OUT OPTIONS NOT USED IN ANY SPECIFIC PROJECT.



S 3325

PATENTS PENDING

IDENTIFICATION STAMP

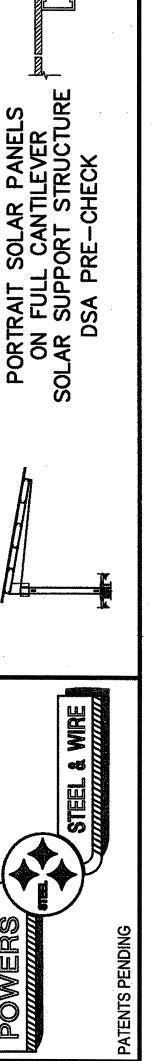
DIV OF THE STATE ARCHITECT

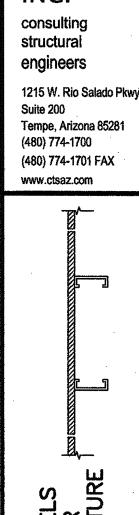
0 8 2012

DOCUMENT CODE: 2010 CBC A SEPARATE PROJECT APPLICATION FOR CONSTRUCTION IS REQUIRED

PRE-CHECK (PC)

DSA APP. NO 02-112000 IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT OFFICE OF REGULATION SERVICES 02/112000 4 FLENONESS KR 3.22.12





CARUSO

TURLEY

SCOTT



DRAWING EDITION/REF JOB # SITE PROJECT: **REVISIONS:** B NUMBER:

11-071 DRAWN: ENGINEER: CHECKE BLP PGS DS 3/15/12

