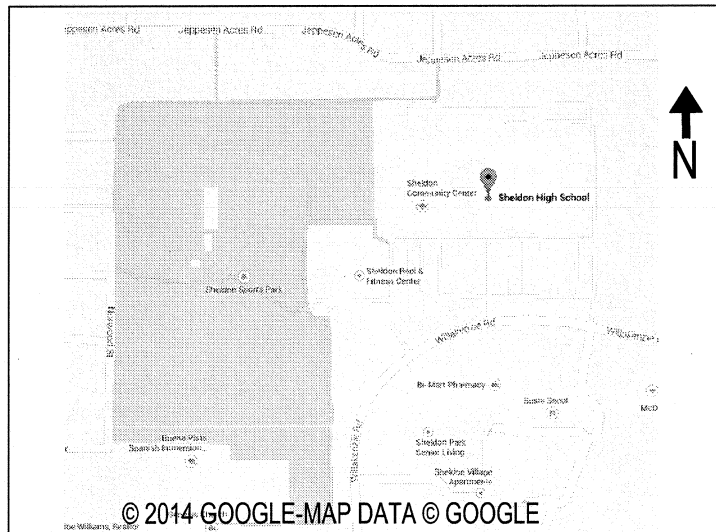


GENERAL

1. ALL CONSTRUCTION TESTING AND INSPECTIONS SHALL CONFORM TO THE 2012 IBC AND 2014 OSSC.
2. ALL DETAILS ARE TYPICAL. FOR CONDITIONS NOT SPECIFICALLY SHOWN, PROVIDE DETAILS SIMILAR TO THOSE SHOWN, SUBJECT TO REVIEW.
3. VERIFY ALL EXISTING FEATURES AND CONDITIONS (DIMENSIONS, ELEVATIONS, ETC.) UPON WHICH THESE DRAWINGS RELY.
4. OMISSIONS OR DISCREPANCIES BETWEEN THE VARIOUS ELEMENTS OF THE CONTRACT DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE WORK.
5. COORDINATE STRUCTURAL WORK WITH THE MECHANICAL, ELECTRICAL, PLUMBING, & FIRE WORK. OPENINGS IN FLOORS, BEAMS, OR JOIST LARGER THAN THOSE SHOWN ON TYPICAL DETAILS OF STRUCTURAL DRAWINGS SHALL BE REVIEWED BY THE ENGINEER OF RECORD BEFORE PROCEEDING WITH THE WORK.
6. DURING THE CONSTRUCTION PERIOD, THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE SAFETY OF THE BUILDING. THE CONTRACTOR SHALL PROVIDE ADEQUATE SHORING, BRACING, AND GUYS IN ACCORDANCE WITH ALL NATIONAL, STATE, AND LOCAL SAFETY ORDINANCES.
7. WHERE A CONFLICT OCCURS BETWEEN SPECIFICATIONS, NOTES ON THE DRAWINGS, GENERAL NOTES, AND SPECIFIC DETAILS, THE MORE RESTRICTIVE SHALL GOVERN.
8. DO NOT SCALE THE DRAWINGS.
9. VIBRATION EFFECTS OF MECHANICAL EQUIPMENT ARE NOT CONSIDERED TO BE DETRIMENTAL TO THE STRUCTURAL DESIGN AND HAVE NOT BEEN CONSIDERED BY THE STRUCTURAL ENGINEER.
10. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ALL EXISTING UTILITIES WHETHER SHOWN HEREIN OR NOT, AND TO PROTECT THEM FROM DAMAGE. THE CONTRACTOR SHALL BEAR ALL EXPENSE OF REPAIR OR REPLACEMENT IN CONJUNCTION WITH THE PERFORMANCE OF THIS WORK.
11. ITEMS IDENTIFIED BY TRADE NAME ARE INDICATIVE OF A LEVEL OF PERFORMANCE OR A GRADE OF MATERIAL. IN ALL SUCH CASES THE PHRASE "OR APPROVED EQUAL" SHALL APPLY. SUBSTITUTES SHALL BE SUBMITTED FOR APPROVAL PRIOR TO USE.
12. CONTRACTOR TO VERIFY ALL DIMENSIONS AND ELEVATIONS SHOWN ON STRUCTURAL, AND MECHANICAL DRAWINGS. NOTIFY ENGINEER OF ANY CONFLICTING INFORMATION PRIOR TO BEGINNING CONSTRUCTION.
13. THE BRACING AND SHORING SYSTEMS REQUIRED TO PROVIDE TEMPORARY SUPPORT OF THE STRUCTURE DURING CONSTRUCTION SHALL BE DESIGNED TO SUPPORT THE DEAD, LIVE, SOIL, EARTHQUAKE, AND WIND LOADS THAT MAY BE IMPOSED ON THE STRUCTURE DURING CONSTRUCTION, IN ACCORDANCE WITH INDUSTRY STANDARDS AND GENERALLY ACCEPTED ENGINEERING PRINCIPLES.

14. THE STABILITY AND INTEGRITY OF THE EXISTING STRUCTURES DURING CONSTRUCTION SHALL BE MAINTAINED AT LEVELS GENERALLY ACCEPTABLE WITHIN THE CONSTRUCTION INDUSTRY BY THE USE OF BRACING, SHORING AND UNDERPINNING UNTIL THE PROPOSED STRUCTURE MODIFICATIONS ARE COMPLETED. IN NO CASE SHALL THE EXISTING STRUCTURES BE ALLOWED TO BECOME UNSAFE DURING CONSTRUCTION.
15. NO CONSTRUCTION OR ORDERING MATERIALS SHALL TAKE PLACE UNTIL THE CONTRACTOR HAS RECEIVED REVIEWED SUBMITTALS BY THE ENGINEER.
16. CONSTRUCTION LIABILITY: CONSTRUCTION CONTRACTOR AND HIS/HER SUBCONTRACTORS AGREE THAT IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, CONSTRUCTION CONTRACTOR AND HIS/HER SUBCONTRACTORS WILL BE REQUIRED TO ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THE PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY; THAT THIS REQUIREMENT SHALL BE MADE TO APPLY CONTINUOUSLY AND NOT LIMITED TO NORMAL WORKING HOURS, AND CONSTRUCTION CONTRACTOR AND HIS/HER SUBCONTRACTORS FURTHER AGREE TO DEFEND, INDEMNIFY, AND HOLD DESIGN PROFESSIONAL HARMLESS FROM ANY AND ALL LIABILITY, REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OF WORK ON THE PROJECT EXCEPT LIABILITY FROM THE SOLE NEGLIGENCE OF THE DESIGN PROFESSIONAL.
17. THE SUBMITTALS SHALL SHOW LAYOUT, SIZE OF MEMBERS, CONNECTION DETAILS AND CONSTRUCTION SEQUENCE FOR ALL BRACING AND SHORING SYSTEMS. THE SUBMITTALS SHALL BE ACCOMPANIED BY STRUCTURAL CALCULATIONS SIGNED BY A REGISTERED STRUCTURAL ENGINEER. THE ENGINEER SHALL ALSO PROVIDE A LETTER STATING THAT HE/SHE HAS REVIEWED THE SUBMITTALS FOR COMPLETENESS AND SHALL PERFORM FIELD VISITS AS REQUIRED IN ORDER TO CHECK GENERAL CONFORMANCE OF THE CONSTRUCTION TO THE CALCULATIONS.
18. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS AS REQUIRED BY THESE DOCUMENTS AFTER THE CONTRACTOR HIMSELF HAS REVIEWED THE SUBMITTALS. ENGINEERING REVIEW OF ANY SUBMITTALS IS ONLY FOR COMPLIANCE WITH GENERAL STRUCTURAL REQUIREMENT AND IS SPECIFICALLY NOT FOR DIMENSIONAL OR QUANTITATIVE INFORMATION.



SCOPE OF WORK:

1. THE DESIGNS INCLUDED WITHIN THIS DRAWING SET ARE INTENDED FOR BUILDING A NEW BACKSTOP RUNNING DOWN THE FIRST BASE LINE OF THE SHELDON HIGH SCHOOL VARSITY BASEBALL FIELD.
2. NO PART OF THIS DRAWING SET MAY BE USED ON ANY STRUCTURE LOCATED NOT AT THE ADDRESS LISTED ABOVE WITHOUT WRITTEN CONSENT FROM THE ENGINEER OF RECORD.
3. PRIVATE LOCATES OF ALL UNDERGROUND SERVICES AND UTILITIES BY CONTRACTOR.

SITE ADDRESS : SHELDON HIGH SCHOOL
2455 WILLAKENZIE ROAD
EUGENE, OREGON 97401

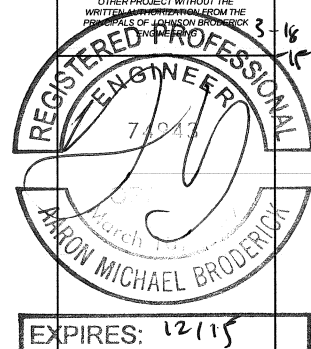
TAX MAP: 17032012045800
NW 1/4 NE 1/4 SEC 20 T 17 S R 3 W WM

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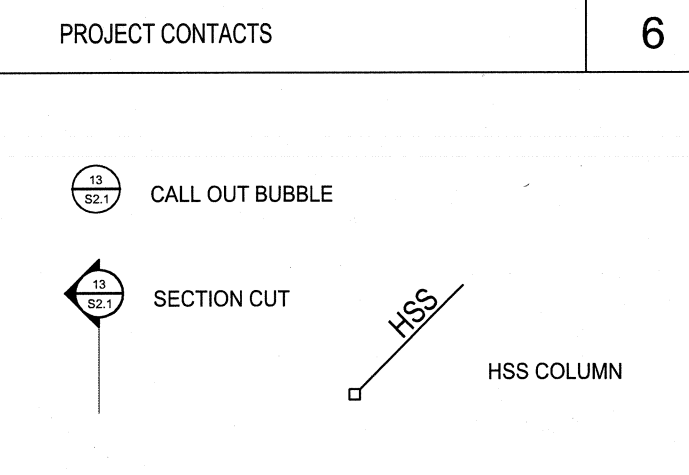
PROJECT SCOPE	8	LOT INFORMATION	4
LIST OF SUBMITTALS	7	STRUCTURAL ABBREVIATIONS	3
PROJECT CONTACTS	6	SHEET INDEX	2
STRUCTURAL SYMBOLS	5	SHEET NUMBERING	1
GENERAL NOTES	13	VICINITY MAP	9

@	AT
APA	AMERICAN PLYWOOD ASSOCIATION
BTW	BOTTOM
CONC	CONCRETE
EW	EACH WAY
FOC	FACE OF CHANNEL
FOB	FACE OF BUILDING
FTG	FOOTING
OC	ON CENTER
OSSC	OREGON STRUCTURAL SPECIALTY CODE
PLY	PLYWOOD
TOC	TOP OF CONCRETE
TYP	TYPICAL

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ENGINEER OF RECORD: AARON BRODERICK, PE
PROJECT CONTACT: AARON BRODERICK, IPE
EMAIL: AARON@JBE.US.COM

S0.0	GENERAL NOTES
S0.1	MATERIAL SPECIFICATIONS
S1.0	SITE PLAN
S2.0	ELEVATIONS
S3.0	STEEL ELEVATION
S4.0	CONNECTION DETAILS
S4.1	FOUNDATION DETAILS
S4.2	NET ATTACHMENT DETAILS



8	6	4	2
7	5	3	1

NOTES

DETAIL SPACE NUMBER FOR SHEETS S1.0 THROUGH S4.0 ALWAYS REMAIN THE SAME. IF A DETAIL OCCUPIES MORE THAN ONE SPACE IT IS IDENTIFIED BY THE NUMBER FOR THE LOWER RIGHT SPACE, A DETAIL OCCUPYING SPACES 1, 2, 3, AND 4 WOULD BE NUMBERED AS 1.

Project: BASEBALL FIELD BACKSTOP SHELDON HIGH SCHOOL

Revisions:

Date: 3-18-2015

Project No: 15002.01

Drawn By: FAD

Checked By: AMB

STRUCTURAL NOTES

Sheet title: S0.0

STRUCTURAL STEEL

1. STEEL USAGE:

HSS COLUMNS: ASTM A500, GR. B, FY=46 KSI
 BASE PLATES: ASTM A36
 ANGLE, CHANNEL: ASTM A36

- ALL STRUCTURAL STEEL TO BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH AISC SPECIFICATIONS, LATEST APPROVED EDITION.
- ALL WELDING TO CONFORM TO AWS AND TO BE PERFORMED BY CERTIFIED WELDERS.
- ALL BUTT WELDS ARE TO BE COMPLETE PENETRATION UNLESS OTHERWISE NOTED. ALL FILLET WELDS SHOWN ARE MINIMUM REQUIRED BY STRESS. INCREASE WELDS TO AISC MINIMUM SIZES BASED ON THICKNESS OF MATERIAL JOINED UNLESS OTHERWISE NOTED.
- WELDING ELECTRODE TO BE E70XX UNLESS OTHERWISE NOTED, EXCEPT E70XX-T4 IS NOT TO BE USED. SEE FRAME CONNECTION DETAILS FOR REQUIREMENT OF WELD METAL.
- ALL DETAILS ARE TYPICAL. FOR CONDITIONS NOT SPECIFICALLY SHOWN, CONTRACTOR SHALL APPLY SIMILAR CONCEPT OR INTENT TO DETAIL THOSE CONDITIONS AND SUBMIT FOR REVIEW AND APPROVAL.
- BOLT HOLES FOR MACHINE BOLTS SHALL BE NO MORE THAN 1/16" OVERSIZE, UNLESS OTHERWISE NOTED. WHERE OVERSIZED HOLES ARE REQUIRED, PROVIDE 5"x3"x3" PLATE WASHER WELDED TO THE STRUCTURAL MEMBER.
- ALL STEEL MEMBERS CONNECTING TO OR SUPPORTING WOOD FRAMING SHALL HAVE 1/2" DIAMETER THREADED STUDS AT 24" ON CENTER UNLESS OTHERWISE NOTED.
- ALL FRAME MEMBERS BELOW FINISHED FLOOR SHOULD BE ENCASED IN MINIMUM 3" CONCRETE PROTECTION AGAINST SOIL, USE WIRE MESH AS REQUIRED.
- GALVANIZE STEEL TUBE PER ASTM A500.

MANUFACTURED PRODUCTS

- THE FOLLOWING MANUFACTURED PRODUCTS SHALL BE DESIGNED AND INSTALLED IN COMPLIANCE WITH THE ASSOCIATED ICC-ES EVALUATION REPORT LISTED BELOW:

A.	PRODUCT	REPORT
	SET	ESR-1772

SUBGRADE PREPARATION

- REMOVE ALL NON-NATIVE OR ORGANIC SOILS.
- PROVIDE MINIMUM 8" COMPACTED CRUSHED ROCK BASE AT FOOTINGS.

CONCRETE

- ALL CONCRETE SHALL BE DESIGNED, MIXED AND PLACED IN ACCORDANCE WITH ACI 318. USE MIXES WITH MAXIMUM AGGREGATE SIZE APPROPRIATE FOR FORM AND REBAR CLEARANCES TO BE ENCOUNTERED.
- THE PROPOSED MATERIALS AND MIX DESIGN SHALL BE FULLY DOCUMENTED. RESPONSIBILITY FOR OBTAINING THE REQUIRED DESIGN STRENGTH IS THE CONTRACTOR'S RESPONSIBILITY. SUBMIT TEST DATA ON EACH PROPOSED MIX FOR REVIEW IN ACCORDANCE WITH ACI REQUIREMENTS.

CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES:

ELEMENT:	AGGREGATE SIZE	28-DAY STRENGTH (PSI)
FOUNDATION	3/4"	3000
SLABS	3/4"	4000

EXTERIOR CONC. SLABS & FOOTING HAVE BEEN DESIGNED FOR $f_c = 2500$ PSI AND DO NOT REQUIRE SPECIAL INSPECTION UNLESS OTHERWISE NOTED ON THESE PLANS. PROVIDE 3000 PSI CONCRETE WHERE REQUIRED FOR WEATHERING, 5% AIR ENTRAINMENT.

- PRIOR TO PLACING CONCRETE, THE CONTRACTOR SHALL ENSURE THAT ALL EMBEDMENTS, ARE PROPERLY LOCATED AND SECURELY TIED IN PLACE. WET SETTING OF ANY APPURTENANCES IS NOT ALLOWED.

SOIL AND FOUNDATION

- THE FOUNDATION DESIGN IS BASED UPON THE 2014 OSSC.
- THE ALLOWABLE DESIGN VALUES ARE AS FOLLOWS:
 - A. VERTICAL BEARING PRESSURE = 1500 PSF
 - B. LATERAL BEARING PRESSURE = 150 PSF
- ALL FOOTINGS AND SLABS SHALL BE FOUNDED ON COMPACTED CRUSHED ROCK; ALL CLAY SOILS TO BE REMOVED.
- ALL WALL FOOTINGS SHALL BE CENTERED BELOW WALL/COLUMN ABOVE UNLESS OTHERWISE NOTED.

WELDING

- ALL WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS PER AWS "STANDARD QUALIFICATION PROCEDURE" TO PERFORM THE TYPE OF WORK REQUIRED. ALL WELDING SHALL BE IN ACCORDANCE WITH THE CURRENT AWS WELDING CODE.
- WELDING ELECTRODES SHALL BE E70 SERIES FOR A36, A572 AND A992 MATERIAL, AND E80 SERIES FOR A705 REINFORCING STEEL.
- WELD METAL TOUGHNESS SHALL BE REPORTED ON THE ELECTRODE MANUFACTURER'S CERTIFICATE OF COMPLIANCE. ALL ELECTRODES SHALL BE LOW HYDROGEN WITH A MINIMUM CVN VALUE OR 20 FT-LBS AT 0° F. EXCEPTIONS: METAL DECK WELDING, STAIR AND HANDRAIL WELDING, LIGHT GAGE STEEL WELDING.
- TACK WELDS, AIR-ARC GOUGING AND FLAME CUTTING SHALL NOT BE PERFORMED WITHOUT ADEQUATE PREHEAT OR INCORPORATION INTO THE FINAL WELD.
- THE FILLER METAL MANUFACTURER'S PUBLISHED RECOMMENDATIONS SHALL BE THE BASIS FOR DETERMINING THE ALLOWABLE RANGE OF ESSENTIAL VARIABLES FOR THE PREQUALIFIED WPS. UNLESS OTHERWISE NOTED ON THE PLANS, BACK-UP BARS FOR CJP WELDS SHALL BE REMOVED.

REINFORCING STEEL

- ALL REINFORCING STEEL SHALL BE PLACED IN CONFORMANCE TO "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318, LATEST APPROVED EDITION), AND THE "ACI DETAILING MANUAL" AS MODIFIED BY THE PROJECT DRAWINGS AND SPECIFICATIONS.
- REINFORCING STEEL TO BE ASTM A615, GRADE 60 DEFORMED BAR UNO.
- REINFORCING STEEL WITH WELDED CONNECTION TO BE ASTM A706, GRADE 60 DEFORMED BAR UNO.
- ALL LAP SPLICES SHALL BE CLASS B SPLICE AND 2'-0" MINIMUM UNLESS OTHERWISE NOTED ON SCHEDULE. MAINTAIN MINIMUM 1-1/2" CLEARANCE BETWEEN PARALLEL BARS OR WHERE NOT CONTACT LAPPED.
- ALL REINFORCING STEEL AND EMBEDMENTS TO BE HELD SECURELY IN PLACE PRIOR TO PLACING CONCRETE. PROVIDE SUFFICIENT SUPPORTS TO ALLOW WALKING ON REINFORCEMENT. PLACE REINFORCEMENT IN RELATIVE POSITION SHOWN ON THE DRAWINGS.
- PROVIDE CONTINUOUS REINFORCEMENT WHENEVER POSSIBLE.
- REINFORCEMENT BARS SHALL NOT BE RE-BENT WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER.

WIRE ROPE

- THE MAIN CABLE SHALL BE 1/2" DIAMETER, EXTRA-HIGH STRENGTH GRADE, GALVANIZED STEEL CABLE HAVING A MINIMUM TENSILE BREAKING STRENGTH OF 26,900 LBS OR GREATER.
- ALL CABLES ORIENTATED VERTICALLY SHALL BE 3/8" DIAMETER, HIGH STRENGTH GRADE, GALVANIZED STEEL CABLE HAVING A MINIMUM TENSILE BREAKING STRENGTH OF 15,400 LBS OR GREATER.
- ALL CABLES SHALL BE GALVANIZED IN ACCORDANCE ASTM A475
- ALL PULLEYS, WIRE CLIPS, QUICK CLIPS AND OTHER HARDWARE FOR CONNECTING THE NETTING SYSTEM SHALL BE CAPABLE OF WITHSTANDING AT LEAST TWO TIMES THE TENSILE LOADS INDICATED BELOW FOR THE THE RESPECTIVE CABLE TYPE.

TABLE 1 - STATIC LOADING

CABLE TYPE	STATIC FORCE	CABLE	CAPACITY	SAFETY FACTOR
MAIN	1,420 LBS	1/2"	26,900 LBS	18.9
VERTICAL	50 LBS	3/8"	15,400 LBS	308

TABLE 2 - CASE 1 (85 MPH WIND)

CABLE TYPE	TOTAL FORCE	CABLE	CAPACITY	SAFETY FACTOR
MAIN	13,400 LBS	1/2"	26,900 LBS	2.0
VERTICAL	7065 LBS	3/8"	15,400 LBS	2.2

TABLE 3 - CASE 2 (ICE AND 30 MPH WIND)

CABLE TYPE	STATIC FORCE	CABLE	CAPACITY	SAFETY FACTOR
MAIN	2770 LBS	1/2"	26,900 LBS	9.7
VERTICAL	1096 LBS	3/8"	15,400 LBS	14

BACKSTOP NETTING

- ALL NETTING SHALL BE #36 THREE-STRAND TWISTED TWINE - KNOTTED NETTING WITH A MESH SIZE OF 1 1/4" AND TWINE BREAKING STRENGTH OF 250LBS OR GREATER.

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REGISTERED PROFESSIONAL ENGINEER

3-15-15

74211

March 15, 2015

HAARON MICHAEL BRODERICK

EXPIRES: 12/15

Project:
 BASEBALL FIELD BACKSTOP
 SHELDON HIGH SCHOOL

Revisions:

Date: 3-18-2015

Project No: 15002.01

Drawn By: FAD

Checked By: AMB

MATERIAL SPECIFICATIONS

Sheet title:

S0.1

MATERIAL SPECIFICATIONS

13

MATERIAL SPECIFICATIONS

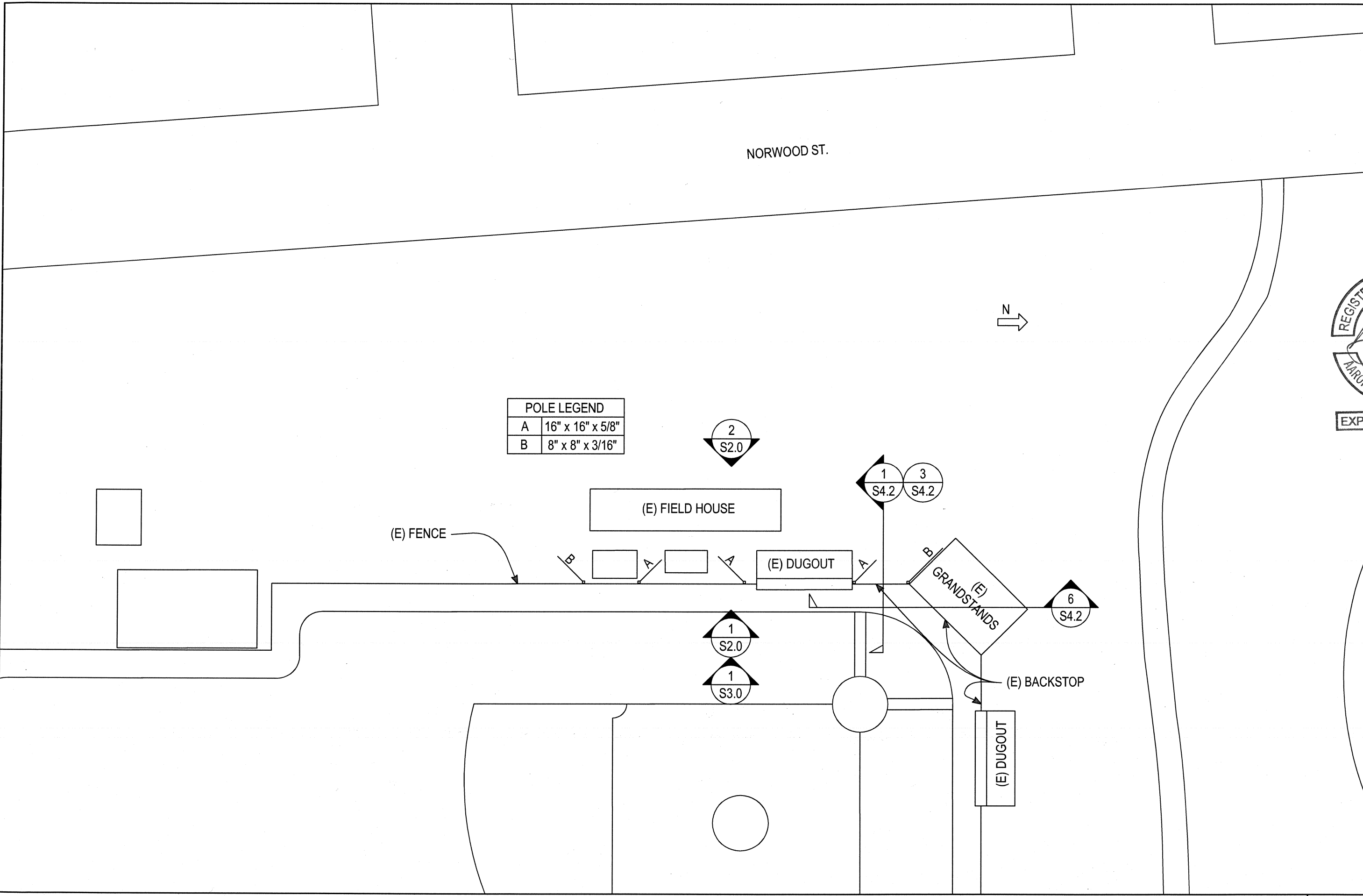
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MATERIAL SPECIFICATIONS

5

MATERIAL SPECIFICATIONS

1



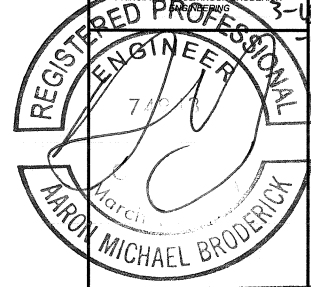
POLE LEGEND	
A	16" x 16" x 5/8"
B	8" x 8" x 3/16"

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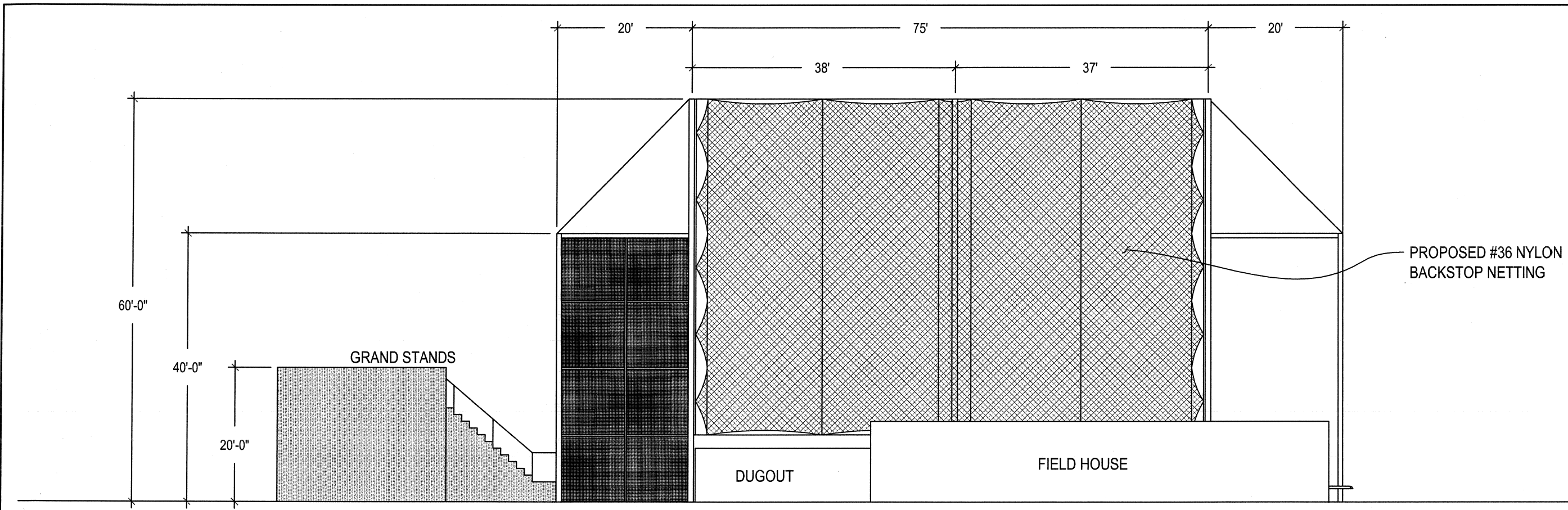


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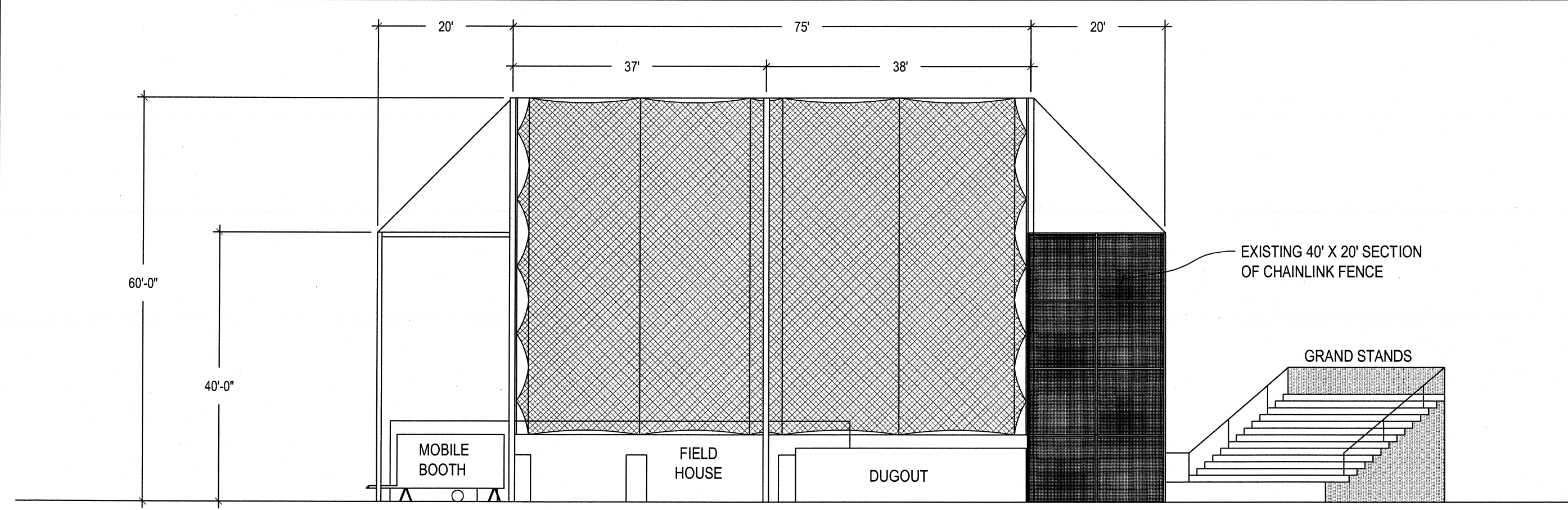
Project:
 BASEBALL FIELD BACKSTOP
 SHELDON HIGH SCHOOL

Revisions:
 Date: 3-18-2015
 Project No: 15002.01
 Drawn By: FAD
 Checked By: AMB

PLAN
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S1.0



WEST ELEVATION Scale: 1/16" = 1'-0" **2**



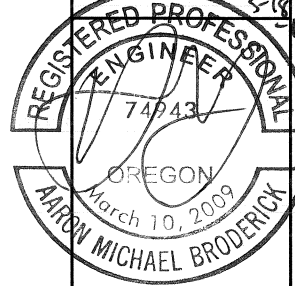
EAST ELEVATION Scale: 1/16" = 1'-0" **1**

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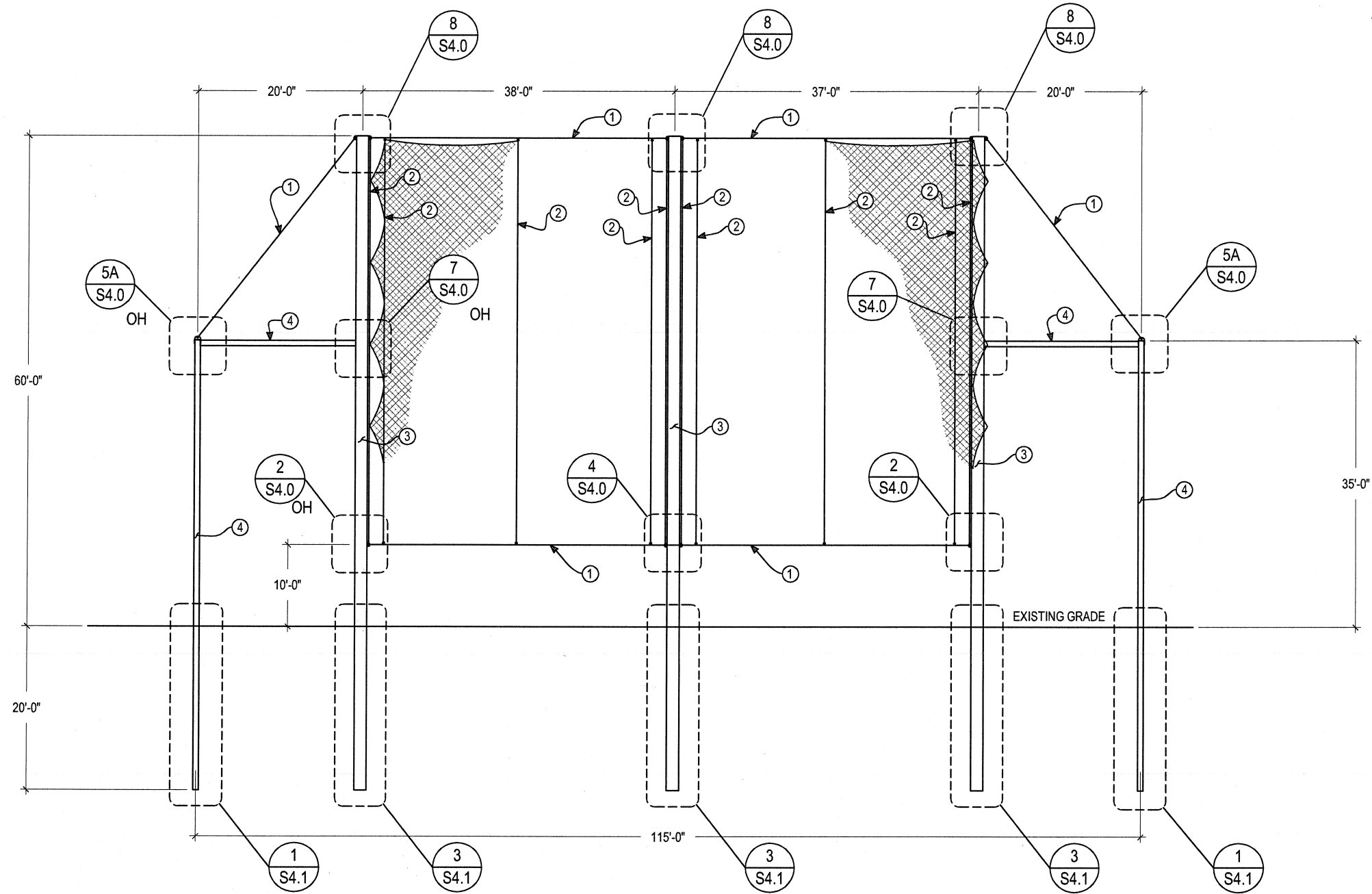


EXPIRES: 12/15

Project:
BASEBALL FIELD BACKSTOP
SHELDON HIGH SCHOOL

Revisions:
Date: 3-18-2015
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Checked By: AMB

ELEVATIONS
Sheet title:
S2.0



- ① = 1/2" EHS WIRE ROPE
- ② = 3/8" EHS WIRE ROPE
- ③ = 16" x 16" x 5/8" HSS
- ④ = 8" x 8" x 3/16" HSS

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BASEBALL FIELD BACKSTOP
SHELDON HIGH SCHOOL

Project:

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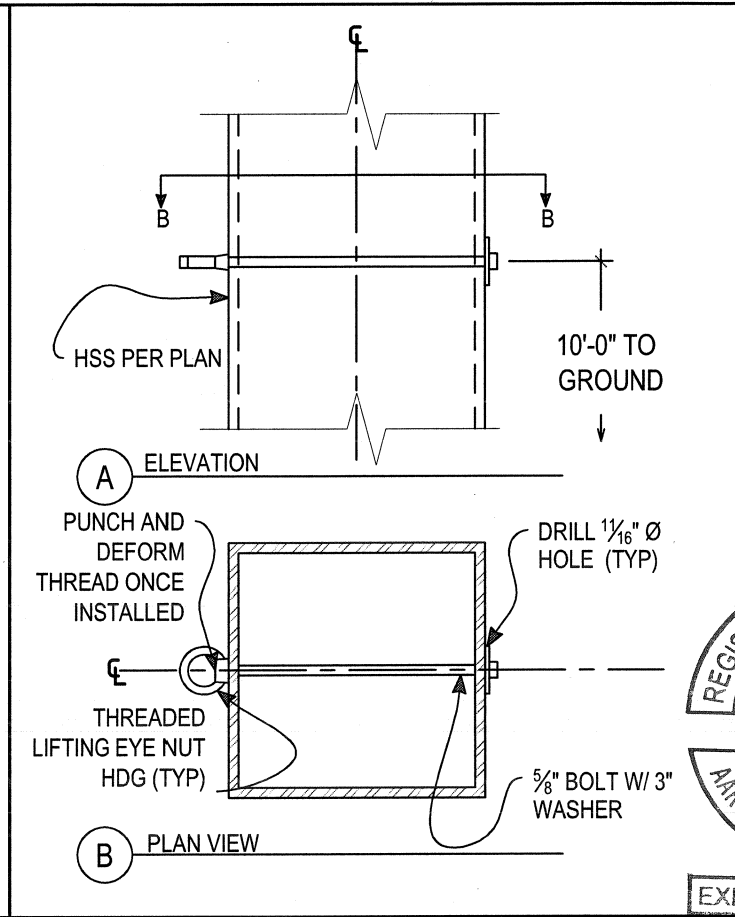
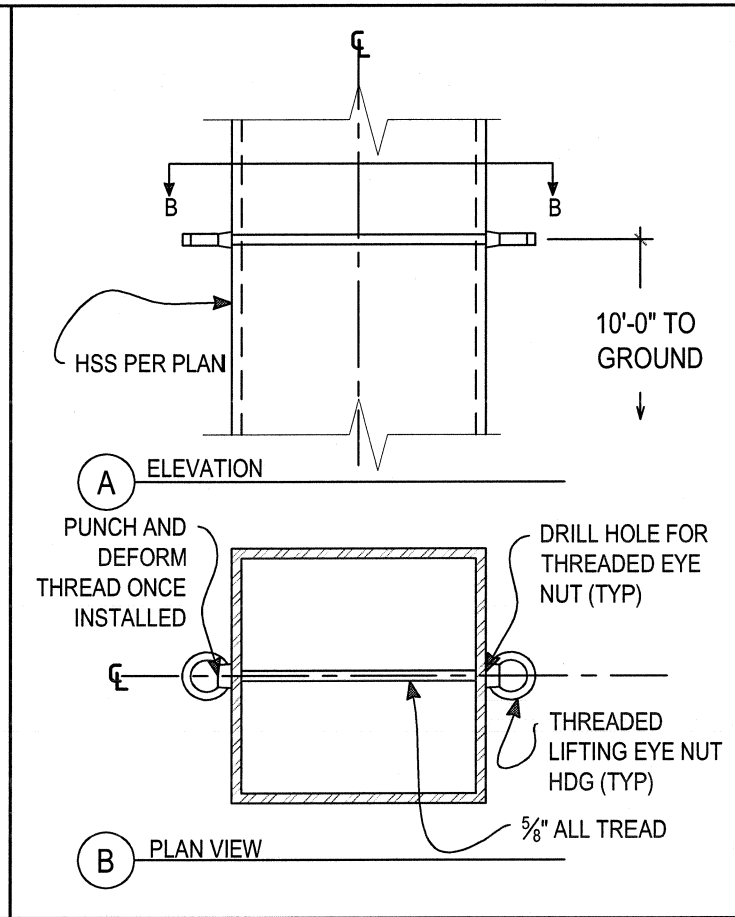
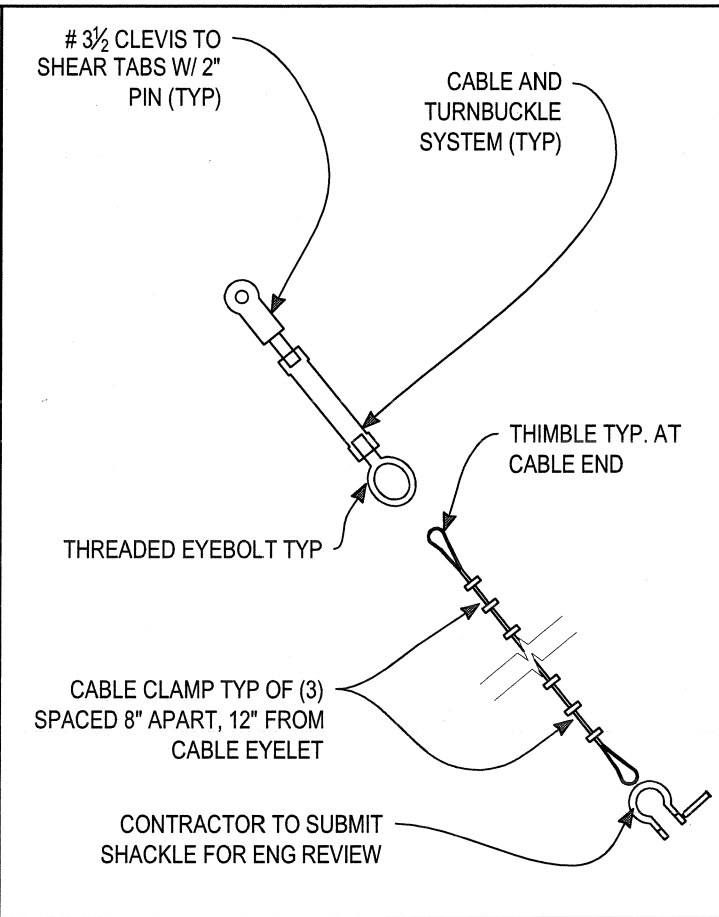
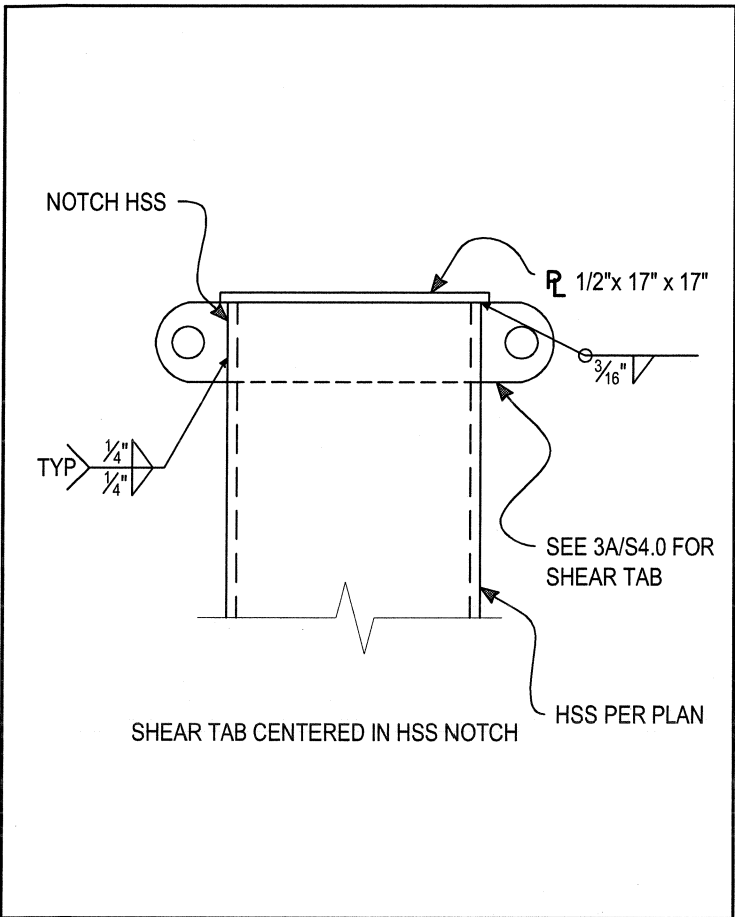
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STEEL ELEVATIONS

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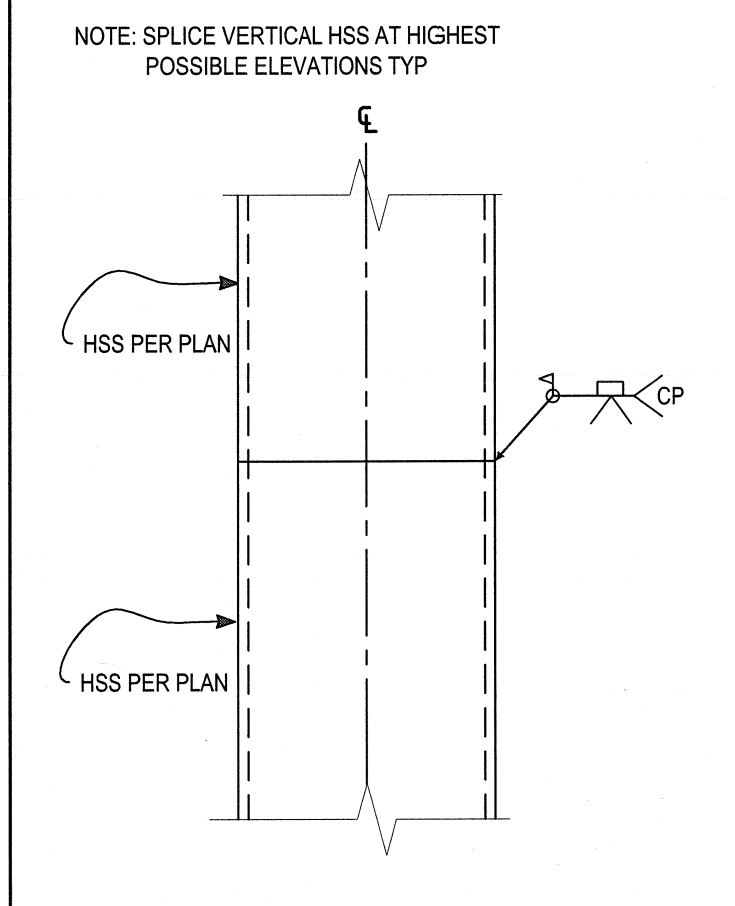
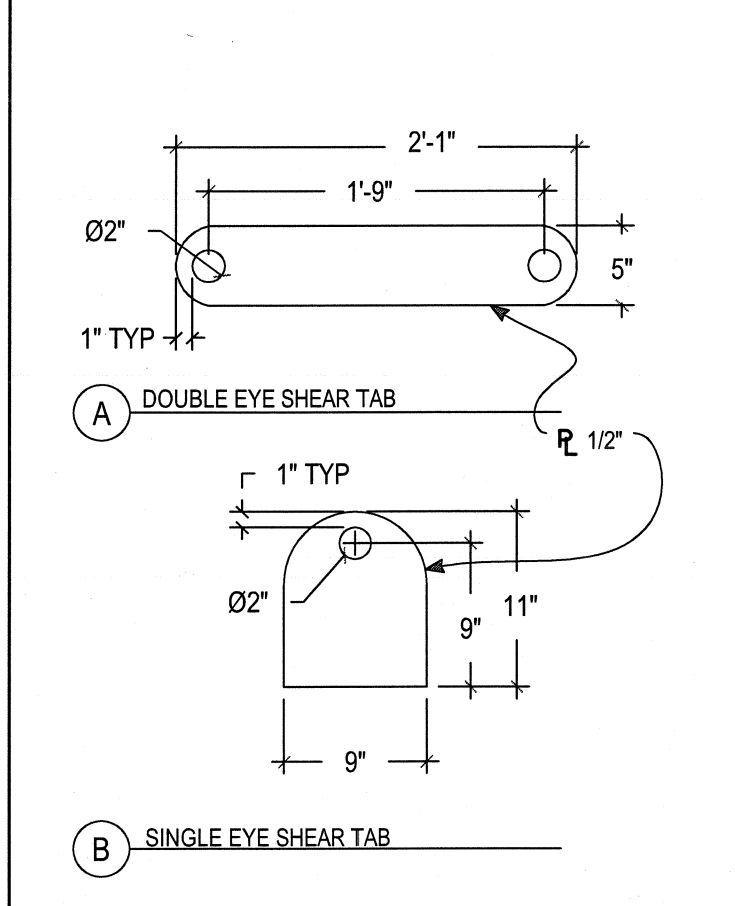
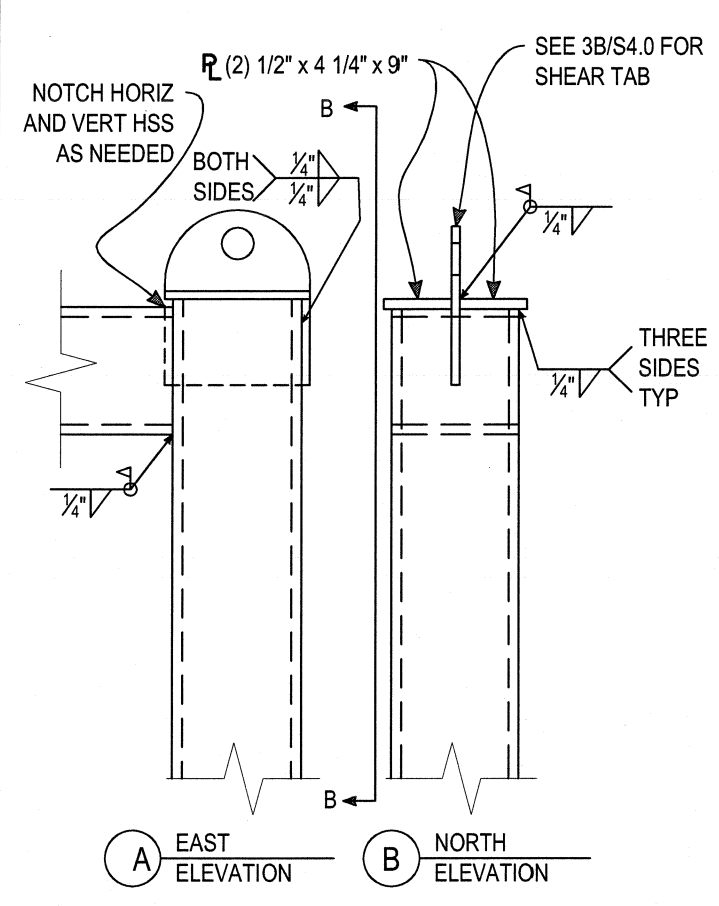
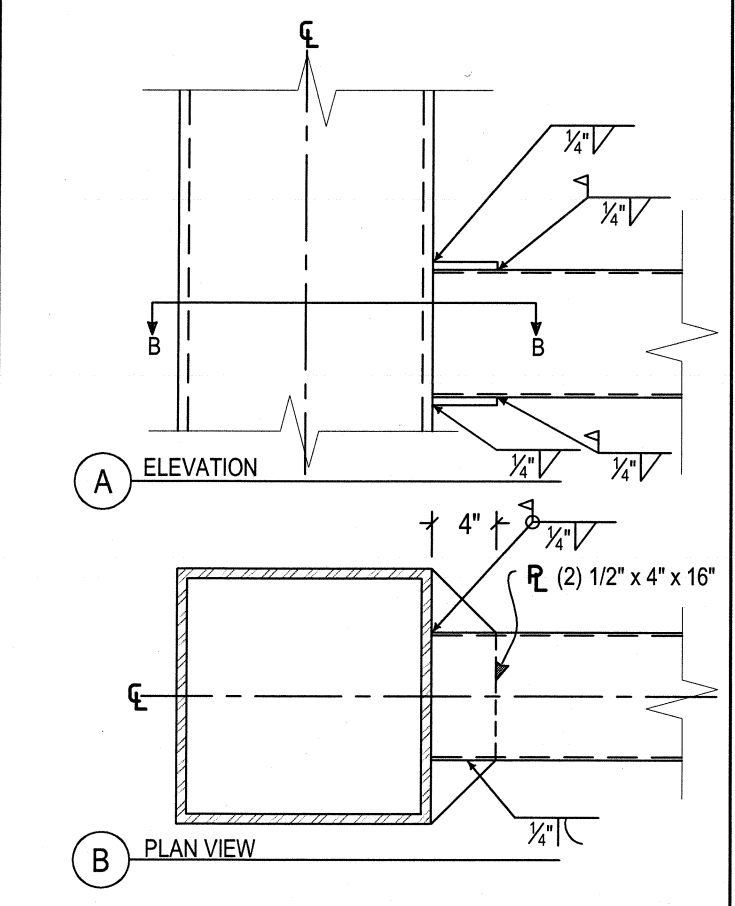


TYPICAL SHEAR TAB POST CAP
Scale: 1" = 1'-0" 8

TYPICAL CABLE CONNECTION
Scale: N/A 6

DOUBLE TENSION TIE
Scale: 1" = 1'-0" 4

SINGLE TENSION TIE
Scale: 1" = 1'-0" 2



HORIZONTAL REINFORCEMENT
Scale: 1" = 1'-0" 7

SHEAR TAB FOR ANGLED CABLE
Scale: 1" = 1'-0" 5

SHEAR TABS
Scale: 1" = 1'-0" 3

TYPICAL COLUMN SPLICE WHERE OCCURS
Scale: 1" = 1'-0" 1

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REGISTERED PROFESSIONAL ENGINEER
74943
MARCH 10, 2009
MICHAEL BRODERICK

EXPIRES: 12/15

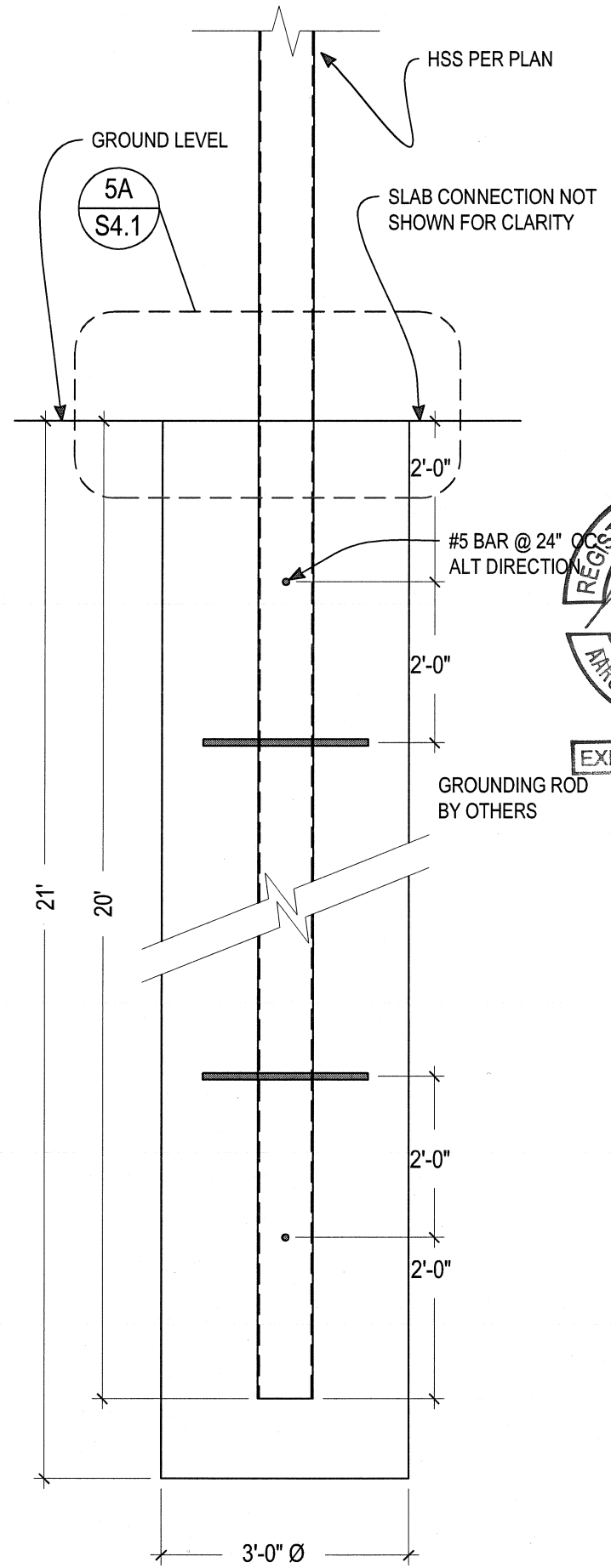
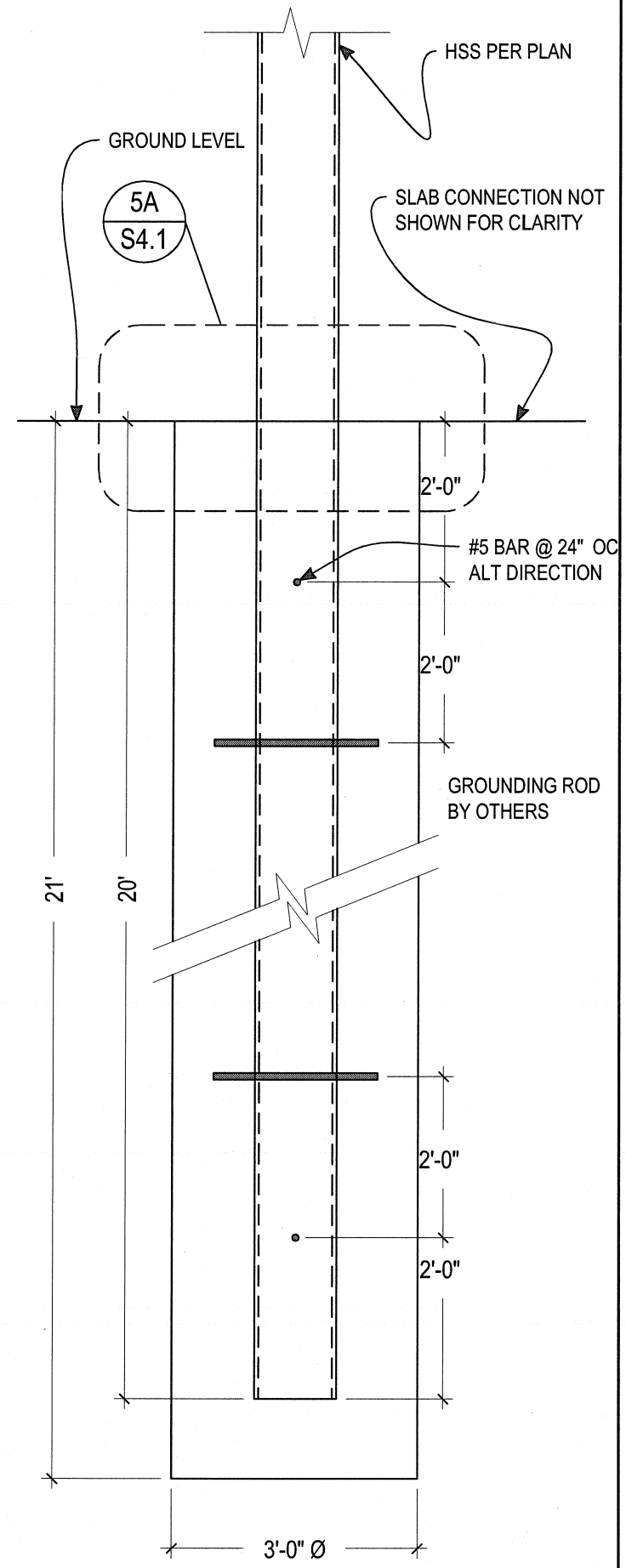
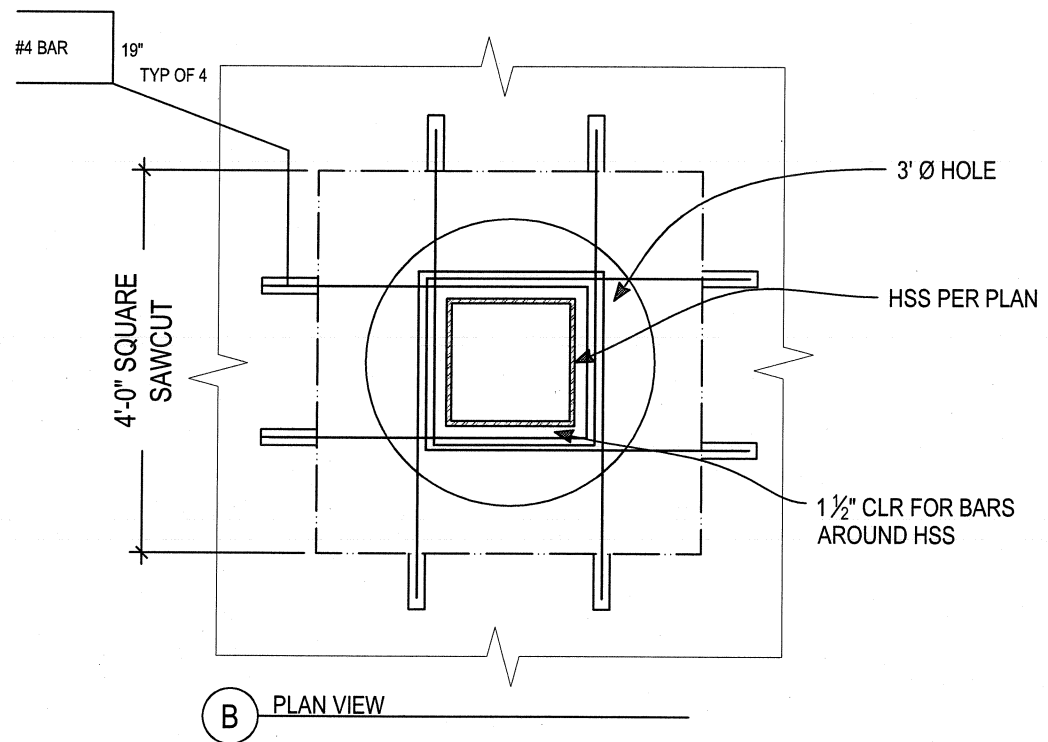
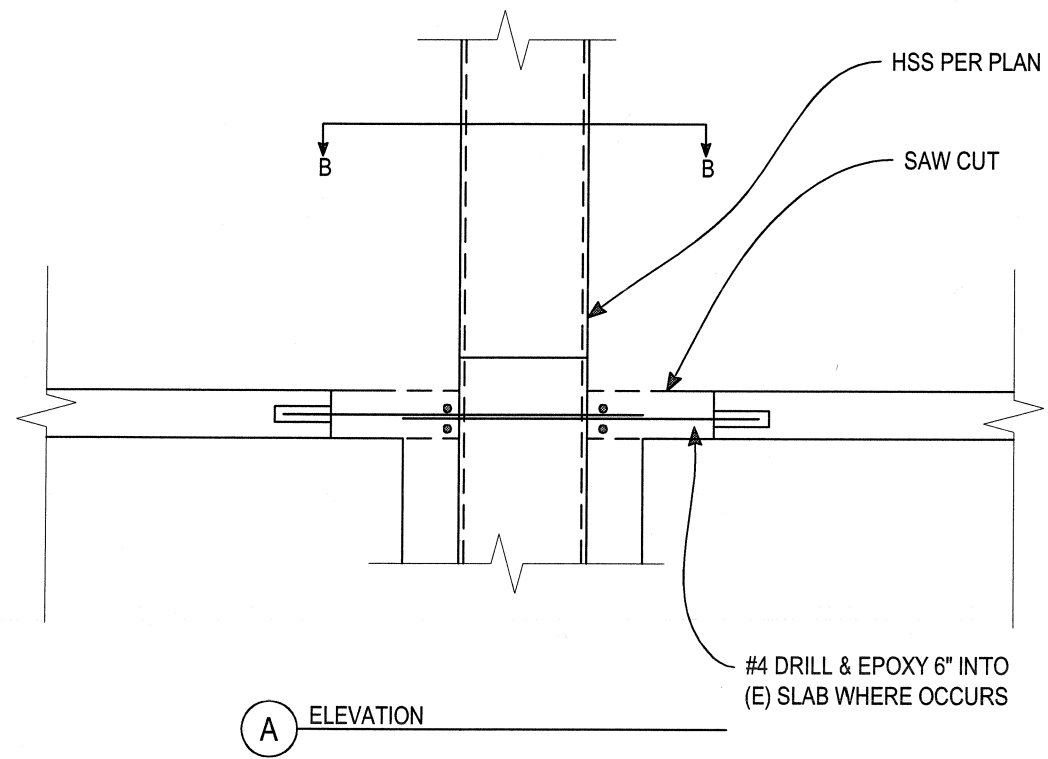
BASEBALL FIELD BACKSTOP
SHELDON HIGH SCHOOL

Project:

Revisions:
Date: 3-18-2015
Project No: 15002.01
Drawn By: FAD
Checked By: AMB

CONNECTION DETAILS

Sheet title:
S4.0

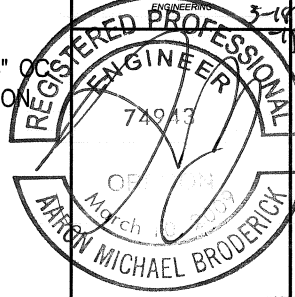


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EXPIRES: 12/15

Project:
BASEBALL FIELD BACKSTOP
SHELDON HIGH SCHOOL

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Checked By: AMB

FOUNDATION
DETAILS

Sheet title:
S4.1

REBAR PLACEMENT AROUND HSS

Scale: 1/2" = 1'-0"

5

16x16 HSS EMBEDMENT

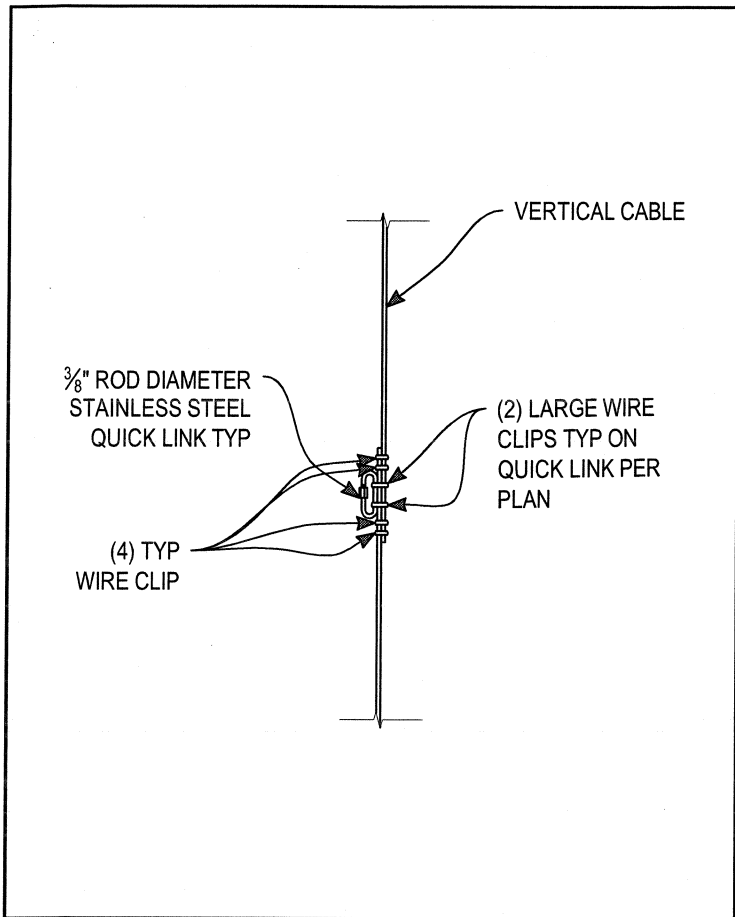
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3

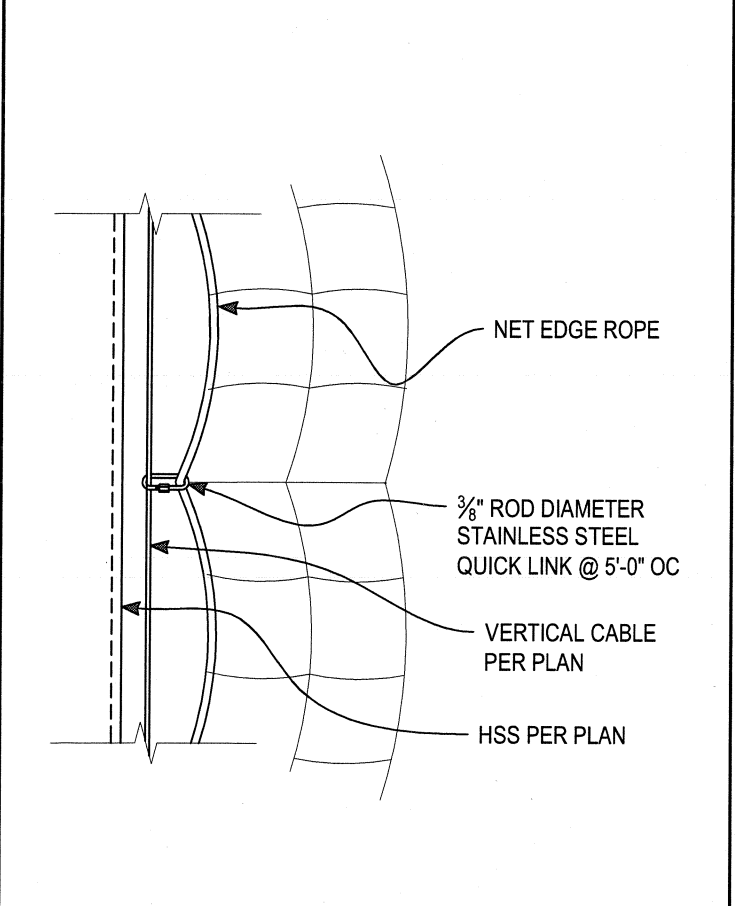
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Scale: 1/2" = 1'-0"

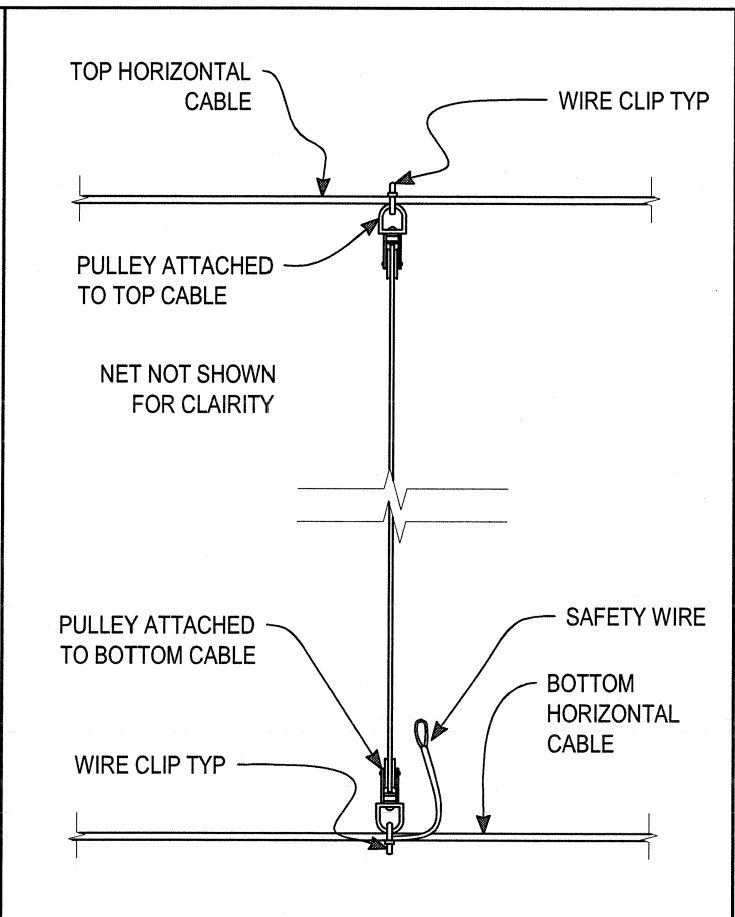
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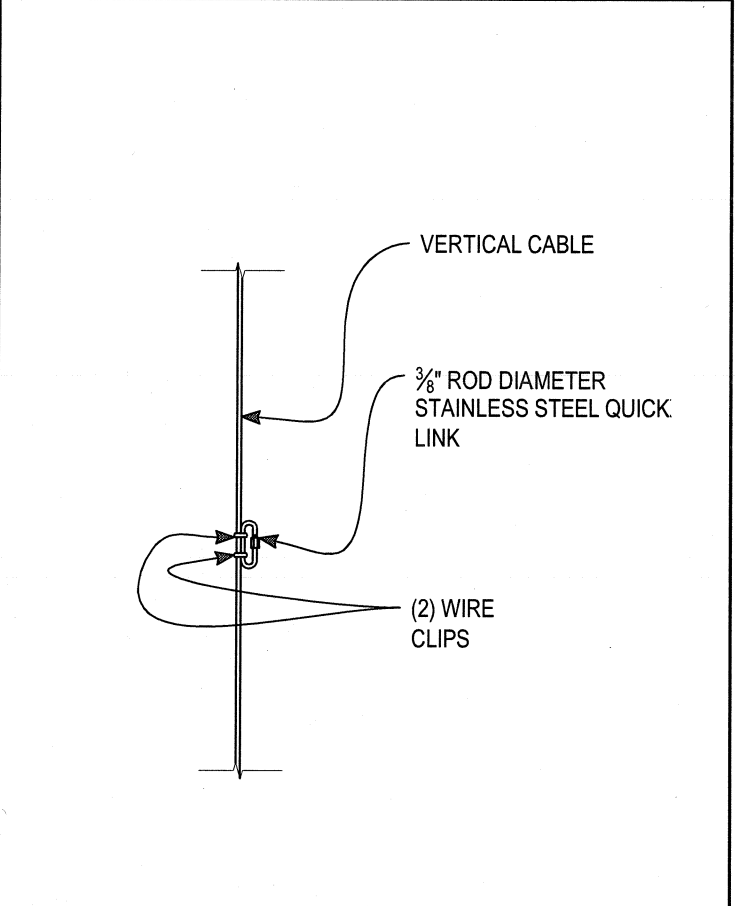
CABLE SPLICE AND QUICK LINK CONNECTION N/A
Scale: N/A



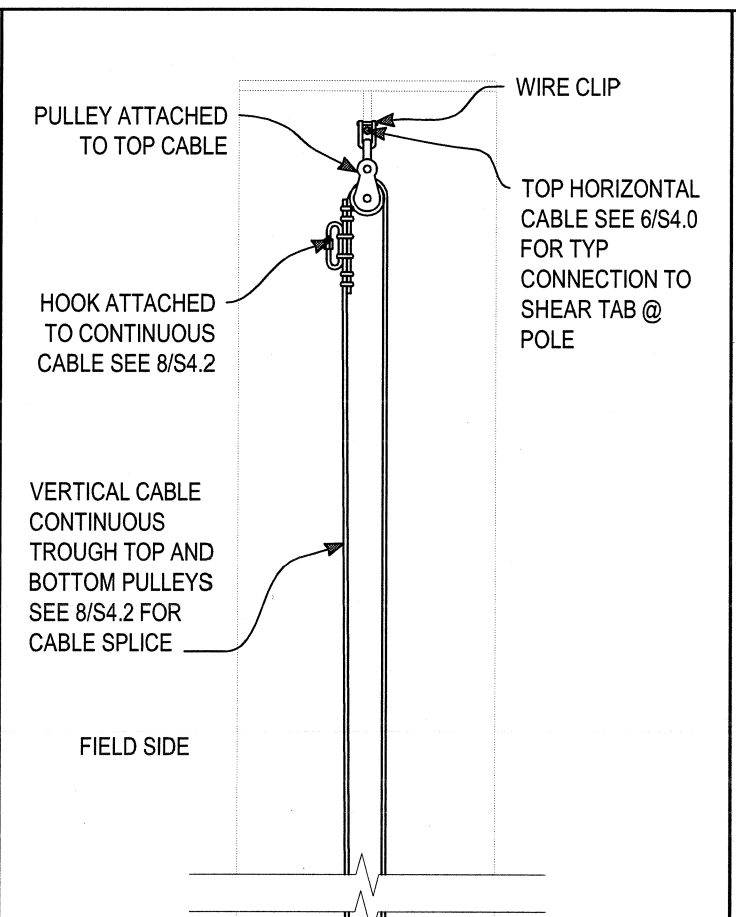
CONNECTION TO CABLE AT NET SIDE N/A
Scale: N/A



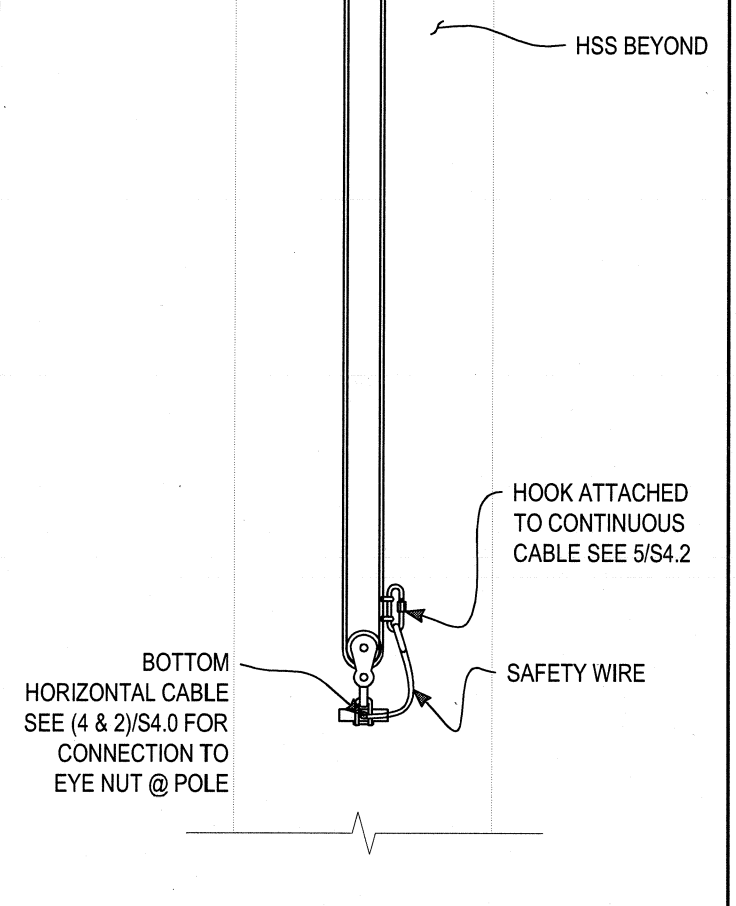
PULLEY SYSTEM N/A
Scale: N/A



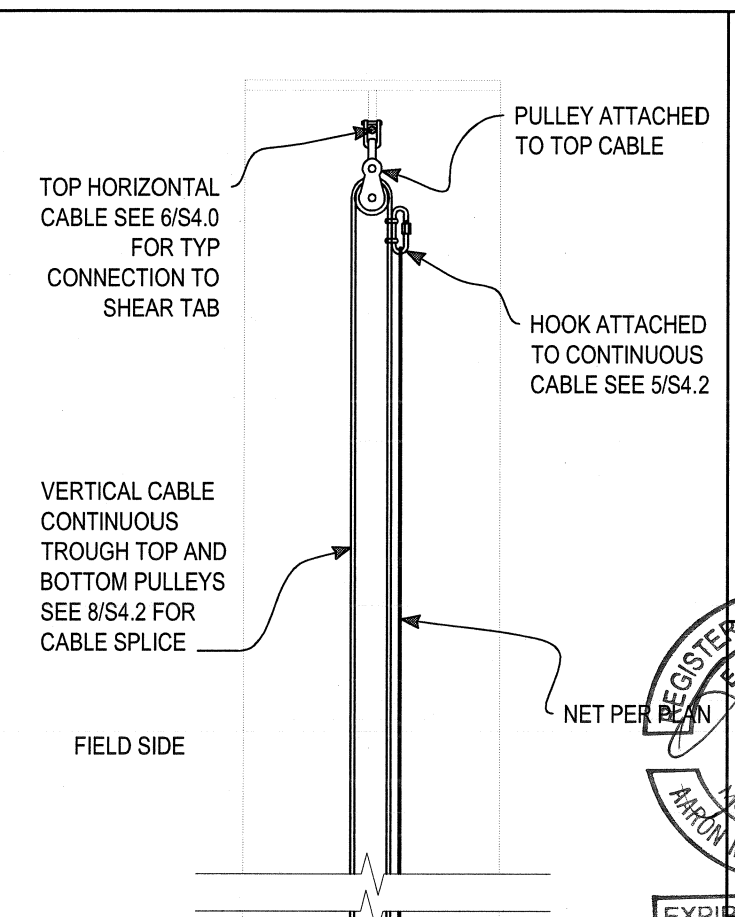
QUICK LINK FOR RAISING NET N/A
Scale: N/A



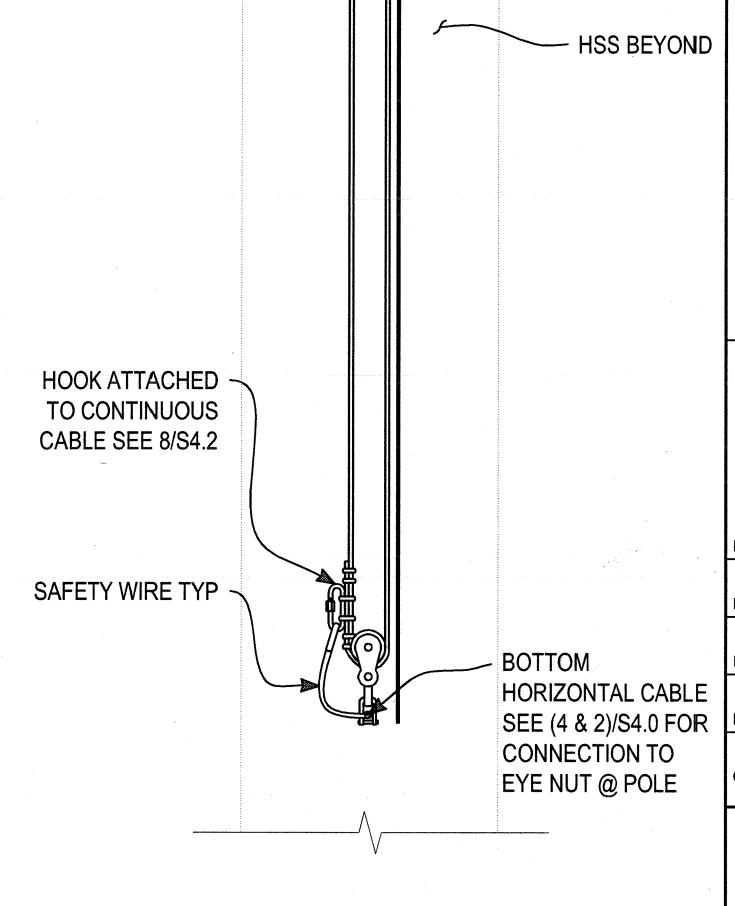
PULLEY SYSTEM NET DOWN N/A
Scale: N/A



PULLEY SYSTEM NET UP N/A
Scale: N/A



PULLEY SYSTEM NET UP N/A
Scale: N/A



PULLEY SYSTEM NET UP N/A
Scale: N/A

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REGISTERED PROFESSIONAL ENGINEER
74943
AARON MICHAEL BRODERICK
March 12, 2015
EXPIRES: 12/15

BASEBALL FIELD BACKSTOP
SHELDON HIGH SCHOOL

Project

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Date: 3-18-2015

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Checked By: AMB

NET ATTACHMENT DETAILS

Sheet title:

S4.2